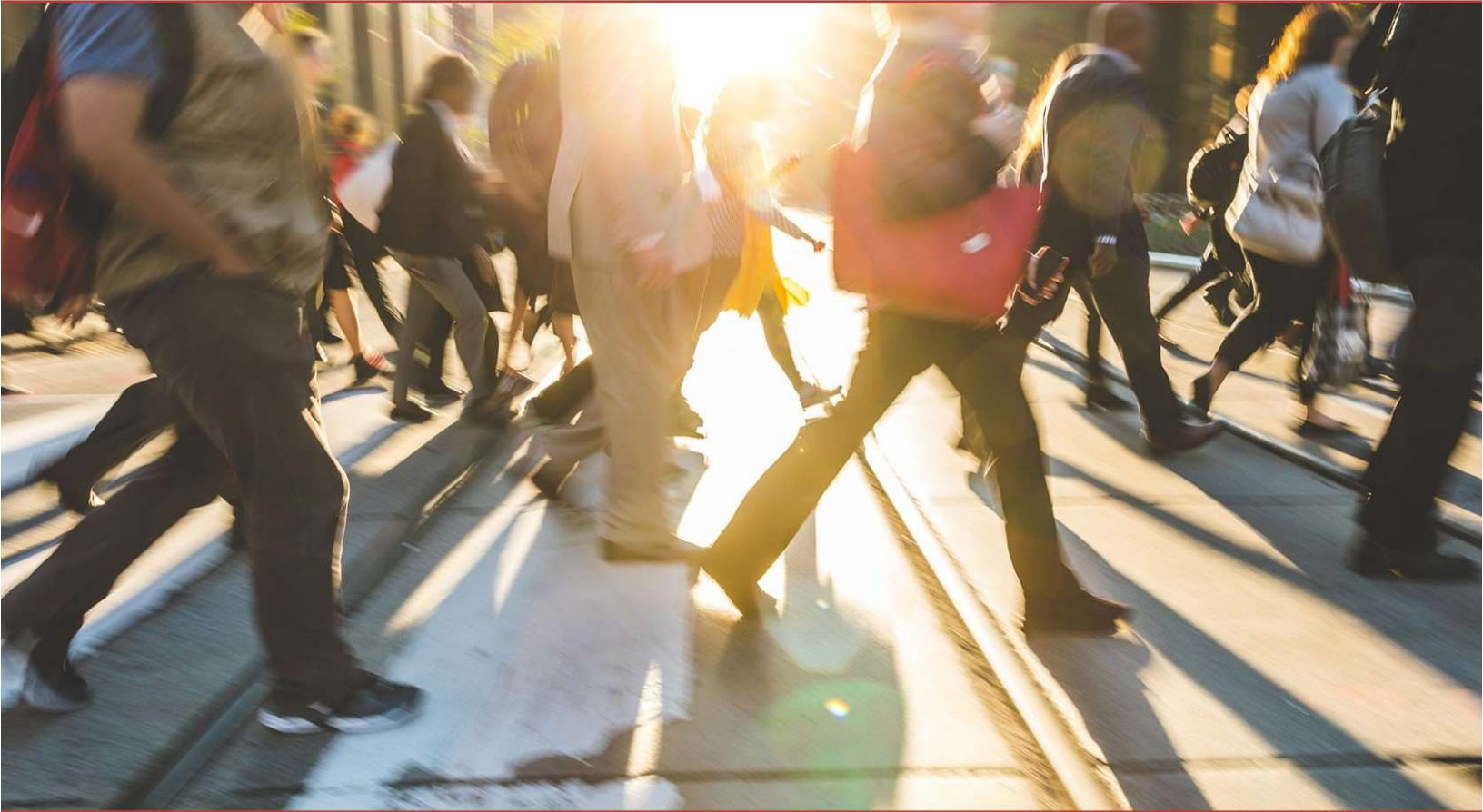


2451-2495 DANFORTH AVENUE PROPOSED MIXED-USE DEVELOPMENT

Urban Transportation Considerations
City of Toronto



Prepared For: First Capital Realty REIT.

November 8, 2024



BA Group

AUTHORSHIP

Date	Revision	Update
11/01/2024	Version 1	Draft Report
11/08/2024	Version 2	Final Report

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1.0 INTRODUCTION

BA Group is retained by First Capital Realty REIT, on behalf of FCHT Holdings (Ontario) Corporation, to provide transportation advisory services for the proposed mixed-use development (the “Proposed Development”) located at 2451-2495 Danforth Avenue (herein referred to as the “site”) in the City of Toronto. The site is located on the southeast corner of the Danforth Avenue and Westlake Avenue intersection, within the East Danforth neighbourhood of Toronto.

1.1 The Existing Site

The site consists of a rectangular shaped plot of land in the southeast corner of the Danforth Avenue and Westlake Avenue intersection. It is bounded by Danforth Avenue to the north, existing low-rise commercial developments to the east, existing 2-storey residential dwelling to the south, and Westlake Avenue to the west. The site is currently occupied by a grocery store that fronts onto Danforth Avenue abutted by surface parking lots on the east and west sides. A sidewalk is currently provided on the south side of Danforth Avenue and east side of Westlake Avenue along the property frontages.

Vehicular access to the site is currently provided via 3 driveways, including:

- An east-west driveway on the site’s western property boundary, connecting to Westlake Avenue; and
- two north-south driveway accesses along the northern property boundary connecting to Danforth Avenue.

1.2 Area Context

The site is located in the Main Street and Danforth GO Protected Major Transit Station Areas (PMTSAs), as defined within the Province of Ontario Growth Plan. PMTSAs are subject to a density target, given their adjacency to higher order transit.

The site is well-located with respect to higher-order transit services and within proximity of a variety of amenities and services. The site is located approximately 260 metres west of the Danforth GO Station, 190 metres west of the Main Street Subway Station, and 650 metres from the Woodbine Subway Station, at the confluence of several major transit services, as well as frequent bus services routing to / from the Main Street Station and Woodbine Station bus terminals.

Active transportation within the site vicinity is supported via cycle lanes on Danforth Avenue, bikeshare stations, sidewalks on both sides of streets, and zebra pavement marking at intersections to improve pedestrian visibility.

With respect to the site’s location adjacent to higher order transit and active transportation infrastructure, the site is well positioned to supported reduced auto mode share.

The site location and plan are illustrated in Figure 1 and Figure 2, respectively.



1.3 Proposed Development

An Official Plan Amended (OPA) and Zoning By-law Amendment (ZBA) application is being made to the City of Toronto to permit a mixed-use redevelopment including residential units as well as grocery and retail uses at-grade. Specifically, the proposed development programme includes the construction of two new buildings providing a total of 620 residential units, 2,258m² of grocery and 939m² of retail gross floor area (GFA). It is proposed to maintain the general location of the existing site driveway on Westlake Avenue which will provide access to the underground parking, pick-up/drop-off spaces and loading facilities.

The two existing vehicle access from Danforth Avenue are proposed to be removed which will provide an unobstructed pedestrian and cyclist connections along the Danforth Avenue site frontage by eliminating turning movements conflicts.

The following sections provide a description of the development proposal as currently conceived and the transportation-related aspects of the site plan.

The proposed development programme is summarized in Table 1. Relevant excerpt plans from the current architectural drawing set are provided in Appendix A.



Table 1 Development Programme Summary

Use	Development Proposal	
Residential	Studio units	139 units
	1-bedroom units	266 units
	2-bedroom units	152 units
	3+ bedroom units	63 units
	Total units	620 units
Non-Residential	Grocery	2,258 m ²
	Commercial	939 m ²
	Total GFA	3,197 m²
Vehicular/Site Plan Elements		
Vehicle Parking Spaces	Residential	190 spaces
	Retail / Visitor	86 spaces
	Car-share	2 spaces
	Total	278 spaces
Bicycle Parking Spaces	Short-term	146 spaces
	Long-term	568 spaces
	Total	714 spaces
Loading Facilities	1 Type 'A', 1 Type 'B', Type 'C', and 1 Type 'G'	

Notes:

1. Based upon site statistics provided by Superkül Architects, dated November 7th, 2024.



1.4 Site Plan Elements

Site Access

The development proposal includes the removal of two existing accesses connecting to Danforth Avenue with the Westlake access to be retained to accommodate site traffic. Removal of the Danforth Avenue site accesses will better align the site with complete street principals and will contribute to an uninterrupted public realm eliminating vehicular movements across the pedestrian clearway and cycle lane on Danforth Avenue east.

The current development concept plan illustrates 1 site driveway connection to Westlake Avenue, approximately 35 metres south of Danforth Avenue (measured from the centreline of Danforth Avenue to the north curb of the driveway). The driveway will accommodate access to pick-up / drop-off facilities, access to the underground parking garage, and loading areas.

Vehicular Parking

A total of 278 parking spaces are provided within a two level below grade parking facility. This includes 190 spaces for residents and 88 spaces for retail and visitor use. The parking facility includes 11 accessible parking spaces located on the P1 and P2 levels adjacent to elevator cores as per City of Toronto Zoning By-law 569-2013.

Bicycle Parking

A total of 714 bicycle parking spaces (136 short-term, 568 long-term and 10 publicly accessible bicycle parking spaces) are provided on site. Long-term and short-term bicycle parking spaces are provided on the P1 level.

There are 15 existing Toronto Bike Share spaces located on the west side of the site. These spaces are proposed to be relocated along the Danforth Avenue frontage, in proximity to the grocery store entrance.

Loading and Servicing

To support the proposed residential development, 1 Type C and 1 Type G are proposed at-grade. The east building's loading area includes 1 Type G to accommodate waste and refuse pick-up and residential moving activity. The west building will include 1 Type C loading space to support residential moving activity. An additional Type A and Type B are proposed to support the grocery loading demands and are located separate from the residential loading areas.

Short Term Parking

Short term parking demand is accommodated on-site. A total of 4 spaces are dedicated for pick-up drop-off (PUDO) activity, 2 for each building at-grade adjacent to residential accesses.

1.5 Study Scope

BA Group has undertaken a review of the key transportation-related aspects of the Zoning By-law Amendment (ZBA) being submitted to the City of Toronto to permit the proposed development. Key aspects of the concept development that has been reviewed are as follows.

Existing Site and Proposed Development

- A review of the existing site uses and overview of the Proposed Development programme.
- A review of the parking supply provisions of the Proposed Development plans.
- A review of the bicycle parking supply provisions for the Proposed Development plans.
- A review of the loading space provisions for the Proposed Development plans.



- A review of the functionality and appropriateness of the proposed vehicle circulation facilities incorporated into the site plan including loading / garbage collection facility arrangements.

Transportation Context

- A review of existing and future transportation context including key municipal road, transit, pedestrian and cycling changes and other non-automobile dependent travel options in the area.

Transportation Demand Management (TDM)

- A review of TDM measures to encourage, facilitate and support non-automobile travel to / from the site for residents, visitors, and employees of the site.

Travel Demand Forecasting

- An outline of travel characteristics and travel demand projections for pedestrians, cyclists, transit users, and automobile users of the Proposed Development.
- An assessment of the existing vehicle traffic activity patterns and volumes in the study area during the key weekday morning and afternoon peak periods.
- A comprehensive review of the vehicle traffic changes that may occur in the area in the future with the development of several other area development projects.
- A review of existing vehicle site -related traffic activity levels that will be eliminated as a result of the Proposed Development.
- An assessment of the vehicular traffic and other trip generation characteristics of the Proposed Development.

Traffic Operations Review

- A detailed review of the traffic operations at intersections in the area under existing and future traffic conditions including an assessment of the operational impacts of the Proposed Development.

The findings of our report are summarized in the following sections.



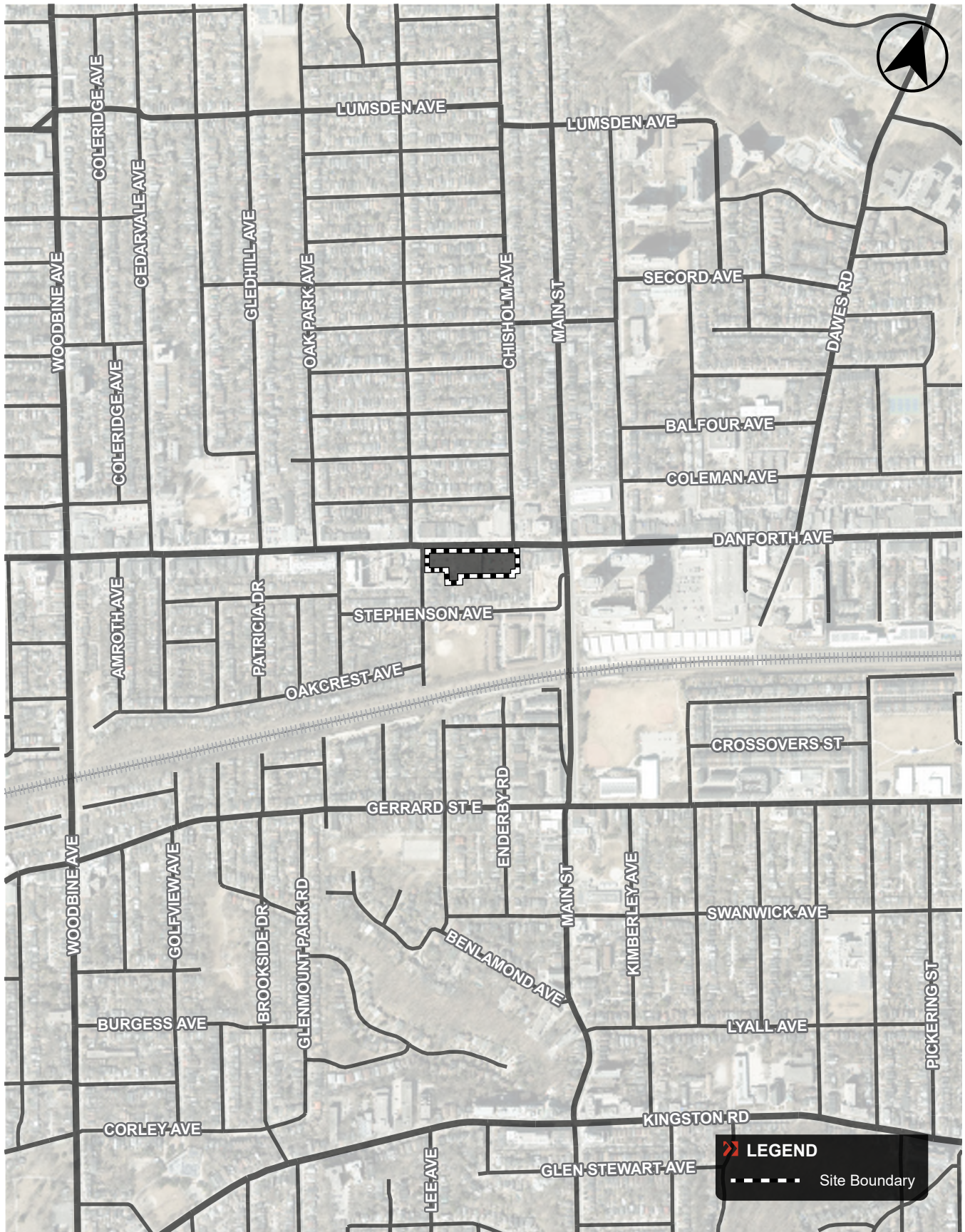


FIGURE 1 SITE LOCATION

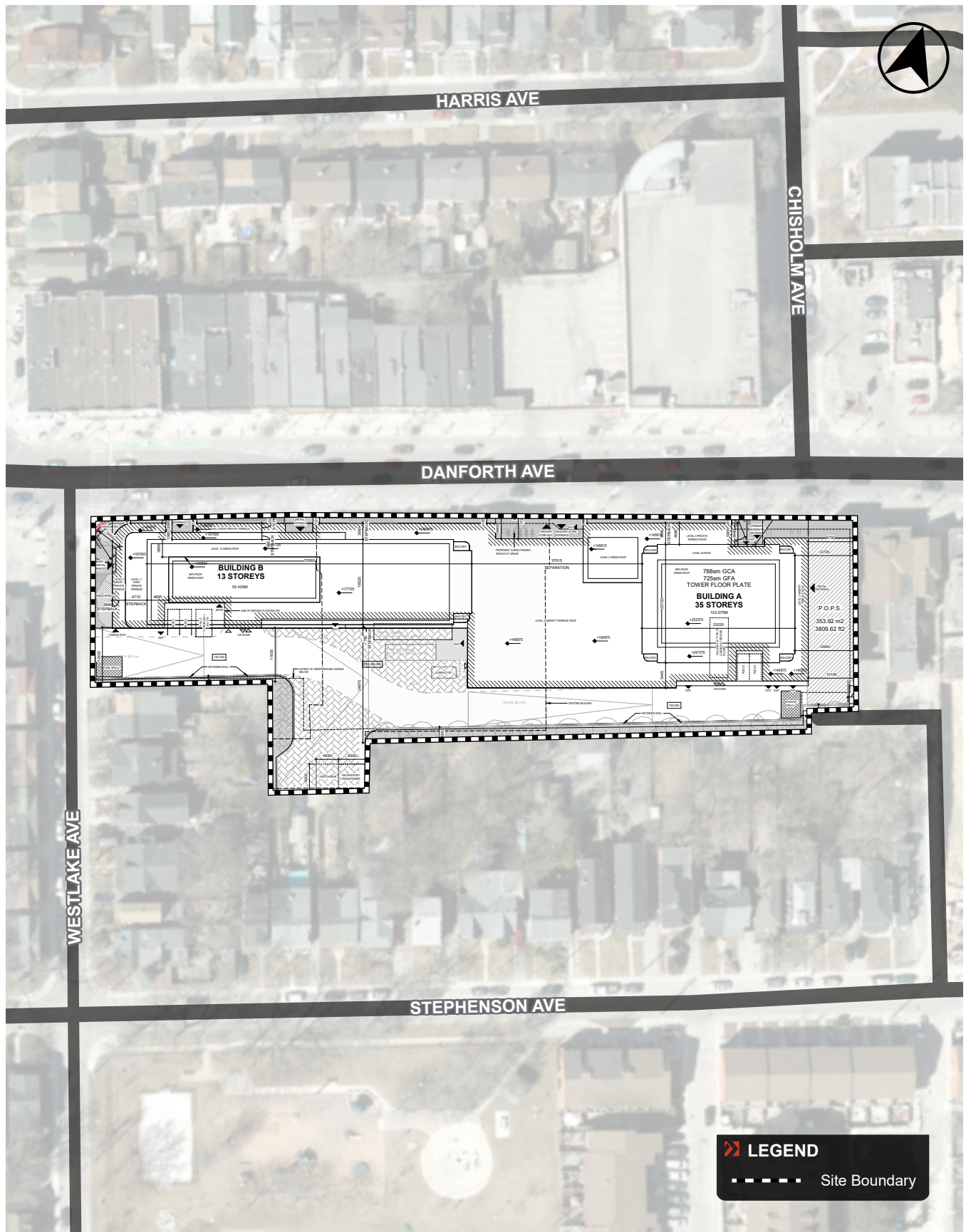


FIGURE 2 SITE PLAN

2.0 PRE-APPLICATION CONSULTATION RESPONSE TO COMMENTS

As part of the Pre-Application Consultation (PAC) (*Pre-App No.: 24 175202 STE 19 PAC*), City of Toronto staff provided comments dated August 27, 2024, related to the proposed development. The current development programme is generally consistent with what was presented to the city during the Pre-Application Consultation (PAC) process (a modest increase in unit count (41 units) and same commercial GFA). This section outlines the comments received from the City of Toronto and responses to those comments.

City Comments – Transportation Services

Comment 1:

Delineate and identify, on all applicable plans, a 6.0 metre corner rounding at the northwest corner of the site (the southeast corner of Danforth Ave and Westlake Ave), to be conveyed as a Pedestrian Clearway Easement.

Response: Noted. See updated architectural plans provided in Appendix A.

Comment 2:

Revise the applicable plans to provide/construct a raised cycle track along the Danforth Avenue frontage of the site. The cycle track should be built as per City standard T-603.051, with a cycle track width of 2.4 metres and buffers of 0.6 metres to the north (separation from traffic) and 0.3 metres to the south of the cycle track (separation from the sidewalk).

Response: Following the PAC meeting, the project team engaged with City of Toronto staff to coordinate the design of a raised cycle track along the Danforth Avenue site frontage. Given site constraints, it was agreed to provide a 1.8m wide cycle track with a 0.9 metre buffer to the north (separation from traffic) and 0.3 metre buffer to the south. No changes are proposed to the eastbound travel lane.

The proposed cycle track plan is provided in Appendix B.

Comment 3:

Provide and maintain electric vehicle (EV) infrastructure in accordance with the requirements of Zoning By-law No. 569-2013, Chapter 200.5.1.10(14). As such, a minimum of 100 percent of resident parking and 25 percent of visitor parking spaces must include an energized outlet capable of providing Level 2 charging or higher.

Response: EV infrastructure is provided in accordance with the Zoning By-law requirements. See **Section 4.4** for further details and architectural plans provided in Appendix A.

Comment 4:

Identify how pick-up and drop-off (PUDO) activity including ride-share and deliveries will be managed on-site/off-street in the Transportation Impact Study (TIS).

Response: Designated pick-up / drop-off spaces are proposed on site, accessed off the site driveway. A total of 4 PUDO spaces are proposed, 2 for each building, located at-grade adjacent to residential accesses. Refer to **Section 7.0** for further information.

Comment 5:

Provide and maintain loading spaces in accordance with Site Specific Zoning By-law Chapter 220 of Zoning By-law 569-2013. Provide Vehicle Manoeuvring Diagrams (VMDs), separately in the TIS and/or on the ground floor plan, for the proposed loading spaces, demonstrating vehicles entering and exiting the site in a forward motion. As per the in-force Zoning By-law, the site requires one (1) Type A, one (1) Type B, one (1) Type C and one (1) Type G loading space. As such, the proposal includes one (1) Type A, one (1) Type B, two (2) Type C and one (1) Type G loading space, which satisfies the loading space requirements of the Zoning By-law. Label the dimensions of the proposed loading spaces.



Response: The proposed loading supply is consistent with the minimum Zoning By-law requirements for the site and includes 1 Type 'A', 1 Type 'B', 1 Type 'C' and 1 Type 'G' loading space. VMDs are provided in Appendix C and further details on the proposed loading supply is provided in **Section 6.0**

Comment 6:

Provide TDM plan in accordance with the policies in the City's Official Plan, Toronto Green Standard (TGS) – Version 4, and Guidelines for the Preparation of Transportation Impact Studies (2013). The applicant shall retain qualified professionals to identify the appropriate travel demand management programs/measures to be implemented on/for the subject site to reduce the single occupancy auto vehicle trips generated by the proposed development. Be advised, that TDM measures exclude parking management strategies, the City's policy/Zoning By-law/TGS Tier 1 requirements and promotional/educational strategies.

Response: Noted. A TDM plan is provided in accordance with the City's Official Plan, and Toronto Green Standards (TGS) – Version 4 (see **Section 7.0**).

Comment 7:

Demonstrate how the project complies with all transportation-related requirements of the TGS Version 4, Sections AQ 1.1 to 1.2, 2.1 to 2.6, and 3.2. All relevant plans must be clearly labelled with the number, type, dimensions and location of the required infrastructure.

Response: **Sections 4.4** and **9.6** demonstrate the satisfaction of the relevant TGS version 4.0 sections. Refer to updated architectural plans provided in Appendix A.

Comment 8:

Parking ramps should be designed in accordance with the following criteria:

- a. The maximum slope of a covered or heated ramp is 15 percent;
- b. The maximum slope of an outdoor unheated ramp is 10 percent;
- c. The minimum width of a clear straight one-way driveway is 3.5 metres;
- d. The minimum width of a clear straight two-way driveway is 5.5 metres;
- e. The maximum sloped floor for direct access to parking areas is 5 percent;
- f. For curved ramp sections, the ramp must be wide enough for two standard passenger cars (P-CAR) to pass each other, as demonstrated using VMDs;
- g. For ramp slope changes of 7.5 percent or greater, a transition area with a minimum length of 3.0 metres (measured parallel to the direction of travel on the ramp) must be provided. The slope of the transition area must be half the sum of the first slope of the ramp or driveway and the second slope of the ramp or driveway.

Response: Noted. Refer to updated architectural plans provided in Appendix A. Vehicle Manoeuvring Diagrams are provided in Appendix C.

Comment 9:

A Transportation Demand Management Strategy is required to demonstrate how the application complies with all transportation-related requirements and the applicable Toronto Green Standard.

Response: Noted. A TDM plan in accordance with Toronto Green Standards (TGS) – Version 4 is provided in **Section 7.0**.



3.0 TRANSPORTATION CONTEXT

The site is located within the East Danforth neighbourhood of the City of Toronto and is well-connected relative to higher-order transit services, with convenient access to Main Street Subway Station, Woodbine Subway Station and Danforth GO Station.

The site's location provides a multitude of work, recreational, amenity and shopping opportunities within walking distance of the site. The level of convenient pedestrian access and transit accessibility provided within the area offers acceptable non-automobile travel opportunities for residents and residential visitors of the area. This serves to reduce the need for area residents to travel on a regular day-to-day basis using a car and to own a vehicle.

An overview of the transportation context in the vicinity of the site is provided in the following sections.

3.1 Area Road Network

3.1.1 Existing Area Road Network

The site is well located relative to the roadway connections provided across the City and the Greater Toronto Area (GTA). A detailed description of the area road network and existing on-street parking regulations surrounding the site are provided in detail in Table 2.

The existing lane configurations and traffic control are illustrated in Figure 3 and the area road network is illustrated in Figure 5.

Table 2 Existing Area Road Context

Road Type / Name	Speed Limit	On-street Parking & Regulations	Description
Major Arterial			
Danforth Avenue	50 km/ hr	East of Westlake Avenue, parking is permitted from 8:00am to 4:00pm, Monday to Saturday (eastbound), and 8:00am to 6:00pm east of Chisholm Avenue. Between Main Street and Chisholm Avenue, parking is permitted from 8:00am to 4:00pm, Monday to Saturday (westbound), and 8:00am to 6:00pm west of Chisholm Avenue.	Danforth Avenue is an east-west City of Toronto road that extends from the Don Valley Parkway in the west (where it becomes Bloor Street) to Cliffside Drive to the east (where it merges with Kingston Road). In the vicinity of the site, the roadway has a two-lane cross section with one vehicular lane operating in each direction and auxiliary right and left turn lanes at key intersections. Cycle tracks are provided along both sides of the roadway. The Danforth Avenue and Westlake Avenue intersection is signalized.
Minor Arterial			
Main Street	50 km/ hr	No parking or standing permitted at any time.	Main Street is a north-south City of Toronto road that extends from Kingston Road in the south (where it becomes Southwood Drive) to 250 metres north of Lumsden Avenue (where it becomes Hamstead Avenue). In the vicinity of the site, the roadway has a four-lane cross section with two lanes operating in each direction with auxiliary left turn lanes at key intersections and streetcar tracks running down the centre lanes. The Danforth Avenue / Main Street intersection is signalized.



Road Type / Name		Speed Limit	On-street Parking & Regulations	Description
Local	Westlake Avenue	40 km/hr	In the vicinity of the site, no parking is permitted on the east side of Westlake Avenue (northbound). South of Danforth Avenue, parking is not permitted on the west side of the roadway between 12:01am and 7:00am, except by permit.	Westlake Avenue is a north-south local road that extends from Oakcrest Avenue in the south and transitions to just 90 metres north of Hamstead Avenue (where it becomes Cosburn Avenue). In the vicinity of the site, the roadway has a two-lane cross-section, with one travel lane in each direction.
	Stephenson Avenue	30 km/hr	On the north side, parking is permitted for an hour anytime between 8:00am to 6:00pm. No Parking is permitted on the south side of the roadway.	Stephenson Avenue is an east-west local road extending from Morton Road in the west to Main Street in the east. The roadway has a two-lane cross-section, with one travel lane in each direction.
	Chisholm Avenue	30 km/hr	On the east side, parking is permitted for an hour anytime between 8:00am to 6:00pm. However, parking is not permitted on the east side between 12:01am and 7:00am, except by permit. No Parking is permitted on the west side of the roadway.	Chisholm Avenue is a north-south local road extending from Danforth Avenue in the south to Hamstead Avenue in the north. The roadway has a two-lane cross-section, with one travel lane in each direction.
	Morton Road	30 km/hr	On either side of the road, parking is permitted for an hour anytime between 8:00am to 6:00pm. Parking is not permitted on the either side of the roadway between 12:01am and 7:00am, except by permit.	Morton Road is a north-south local road extending from Oakcrest Avenue in the south to Danforth Avenue in the north. The roadway has a two-lane cross-section, with one travel lane in each direction.

3.1.2 Future Area Road Network

As part of the proposed development two existing vehicle access connecting to Danforth Avenue will be closed as the site accesses are consolidated to Westlake Avenue. Removal of the Danforth Avenue site accesses will better align the site with complete street principals and will contribute to an uninterrupted public realm eliminating vehicular movements across the pedestrian clearway and cycle lane on Danforth Avenue east. The future lane configuration and traffic controls are depicted in Figure 4.



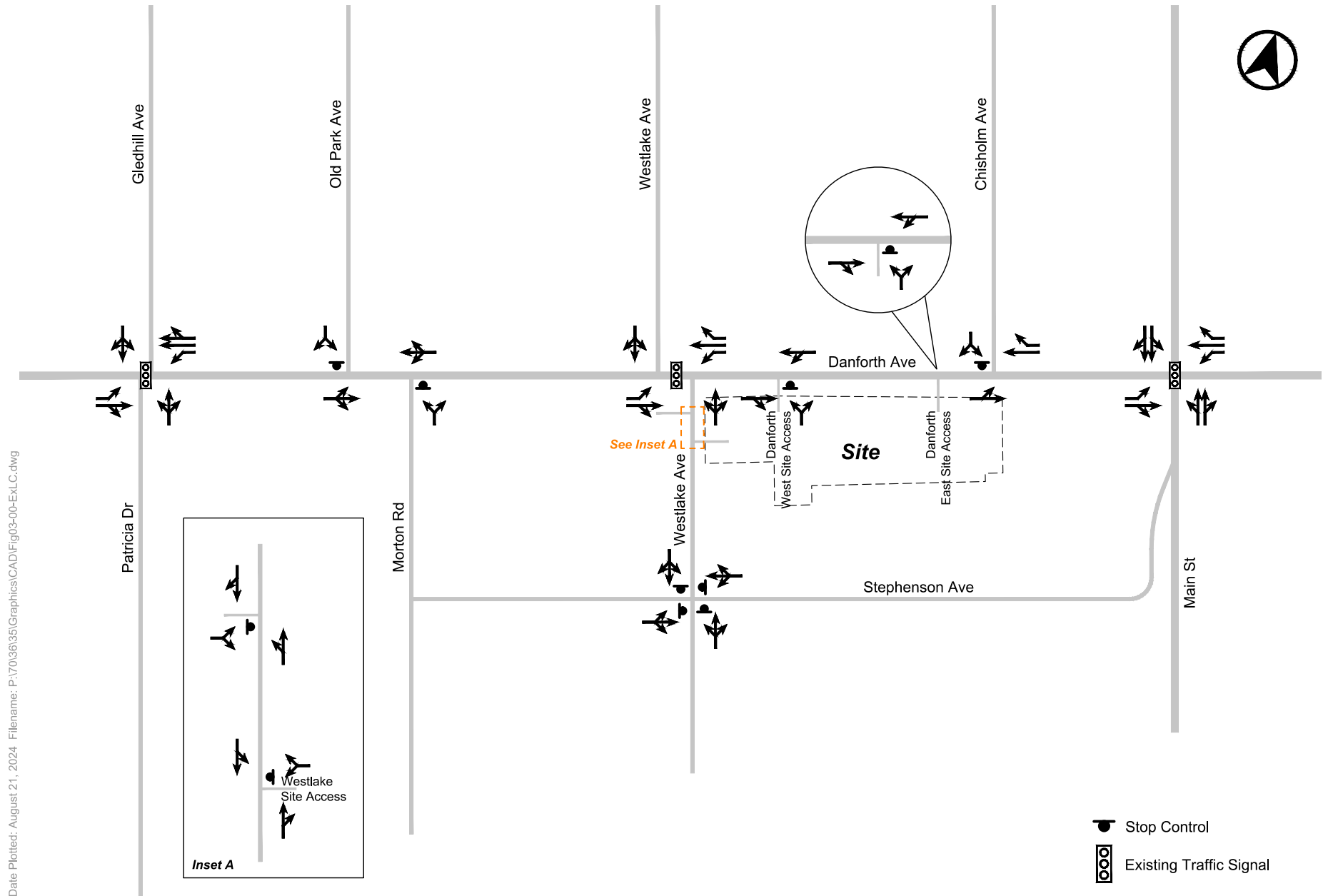


FIGURE 3 EXISTING LANE CONFIGURATION AND TRAFFIC CONTROL

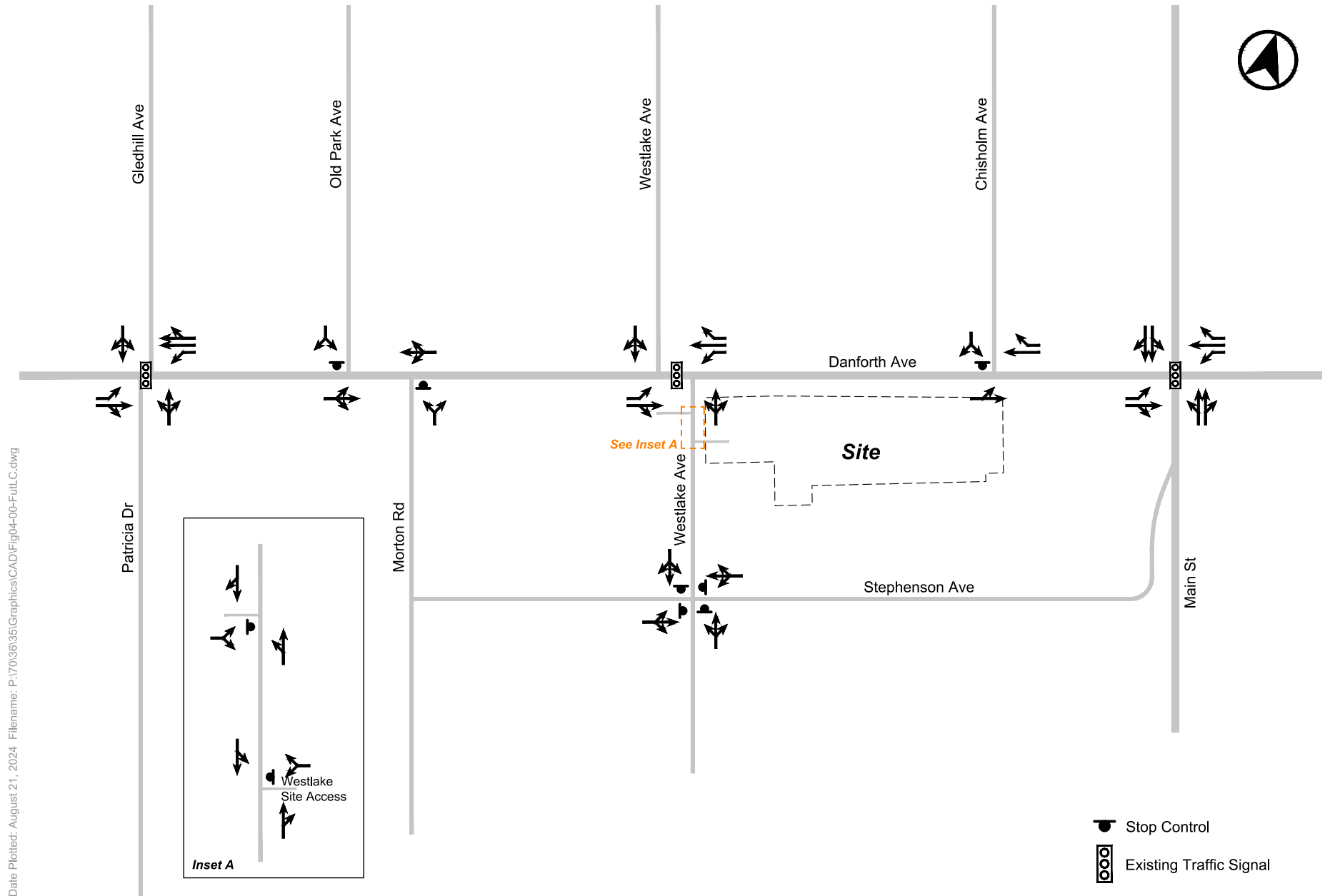


FIGURE 4 FUTURE LANE CONFIGURATION AND TRAFFIC CONTROL

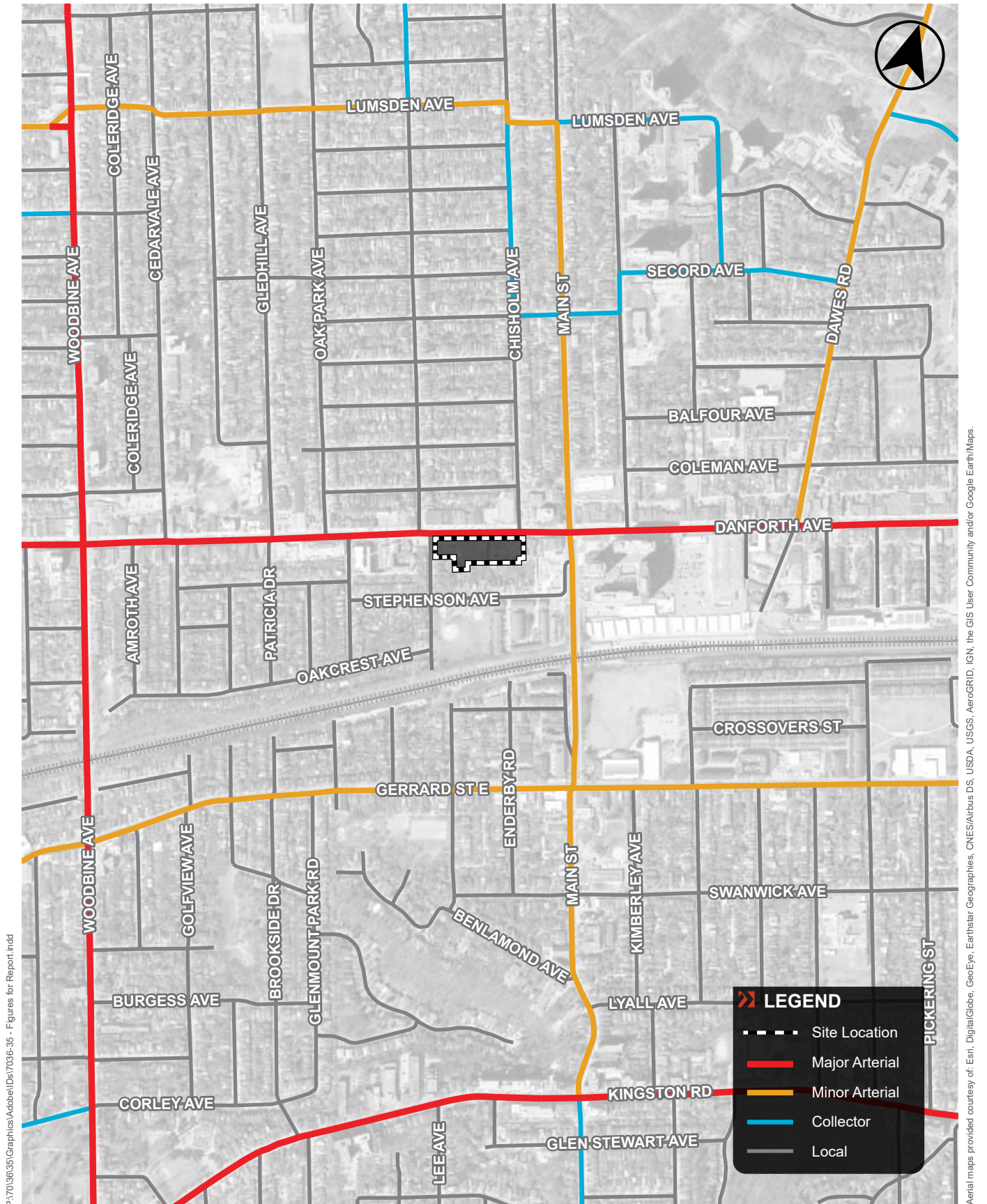


FIGURE 5 EXISTING ROAD NETWORK

3.2 Area Transit Network

The site is well served by transit operated by GO Transit and the Toronto Transit Commission (TTC). Most notably, the site is directly adjacent to the Main-Danforth Mobility Hub (located east of the site), which includes the Main Street Subway Station and bus terminal, and the Danforth GO Station.

The nearest TTC bus routes are also conveniently accessible from the site's location, including several routes operating directly adjacent to the site along the Danforth Avenue and Main Street corridors. The nearest bus stops on Main Street are within 120 metres east of the site.

3.2.1 Existing Transit Facilities

An overview of the subway and bus services in proximity to the site is summarized in Table 3. The existing transit network is further illustrated in Figure 6.

Table 3 Existing Area Transit Context

	Route	Headways	Closest Stop	Route Description
TTC Subway	Line 2 – Bloor / Danforth	3 min. weekday peak periods (4 – 5 min. off peak)	Main Street Station (~ 190 metres / 3 min walk)	The Line 2 subway has a total of 31 stations, generally running in an east west direction. The line operates from Kennedy Road at Eglinton Avenue East, westwards along Danforth Avenue / Bloor Street West to Kipling Avenue. Line 2 also connects with the Yonge-University-Spadina Subway (Line 1) at two different stops (Bloor-Yonge and St. George / Spadina).
			Woodbine Station (~ 650 metres / 9 min walk)	
TTC Streetcar	506 - Carlton	9 – 10 min. weekday peak periods (10 min. off peak)	Main Street / Danforth Avenue (~ 95 metres / 1 min walk)	The 506 Carlton Streetcar route operates generally in an east-west direction between Main Street Station on the Bloor-Danforth Subway and High Park Loop. This route also serves the College and Queen's Park Stations on the Yonge-University-Spadina Subway. This route is part of the 10-Minute Network: route operates every ten minutes or better at all times.
TTC Bus	20 - Cliffside	15 – 16 min. weekday peak periods (20 – 23 min. off peak)	Main Street Station (~ 190 metres / 3 min walk)	The 20 Cliffside bus route operates generally in an east-west direction, between Main Street Station and Kennedy Station on Line 2 Bloor-Danforth.
	23 - Dawes	9 – 10 min. weekday peak periods (11 – 16 min. off peak)	Main Street Station (~ 190 metres / 3 min walk)	The 23 Dawes bus route operates generally in a north-south direction, between Main Street Station on the Line 2 Bloor-Danforth and the area of Victoria Park Avenue and St. Clair Avenue East.



Route		Headways	Closest Stop	Route Description
TTC Bus	62 - Mortimer	17 – 20 min. weekday peak periods (20 min. off peak)	Main Street Station (~ 190 metres / 3 min walk)	The 62 Mortimer bus route operates generally in an east-west direction, between Broadview Station and Main Street Station on Line 2 Bloor-Danforth.
	64 – Main	9 – 10 min. weekday peak periods (11 – 20 min. off-peak)	Main Street / Danforth Avenue (~ 95 metres / 1 min walk)	The 64 Main bus route operates generally in a north-south direction, between the area of Queen Street East and Wineva Avenue and Main Street Station on Line 2 Bloor-Danforth.
	87 – Cosburn	9 – 10 min. weekday peak periods (11 – 20 min. off-peak)	Main Street Station (~ 190 metres / 3 min walk)	The 87 Cosburn bus route operates generally in an east-west direction, between Broadview Station and Main Street Station on Line 2 Bloor-Danforth.
	91 – Woodbine	15 – 18 min. weekday peak periods (23 – 30 min. off-peak)	Woodbine Station (~ 650 metres / 9 min walk)	The 91 Woodbine bus route operates generally in a north-south direction, between Woodbine Station on Line 2 Bloor-Danforth and the area of York Mills Road and Don Valley Parkway. Two service branches are operated: The 91C (Woodbine Stn-York Mills) operates at all times, seven days a week. The 91D (Woodbine Stn-York Mills via Railside) operates northbound during the morning peak period, and southbound during the afternoon peak period, from Monday to Friday only.
	92 – Woodbine South	10 min. weekday peak periods (14 – 22 min. off-peak)	Woodbine Station (~ 650 metres / 9 min walk)	The 92 Woodbine South bus route operates generally in a north-south direction, between Woodbine Station on the Line 2 Bloor-Danforth Subway and the area of Woodbine Avenue and Lake Shore Boulevard East.
	93 – Parkview Hills	30 min. weekday peak periods (25 – 30 min. off-peak)	Woodbine Station (~ 650 metres / 9 min walk)	The 93 Parkview Hills bus route operates generally in a north-south direction, between Woodbine Station on Line 2 Bloor-Danforth and the Parkview Hills area.
	113 – Danforth Road	15 – 16 min. weekday peak periods (20 – 23 min. off-peak)	Main Street Station (~ 190 metres / 3 min walk)	The 113 Danforth bus route operates generally in an east-west direction, between Main Street Station and Kennedy Station on the Line 2 Bloor-Danforth Subway.
	135 - Gerrard	20 – 23 min. weekday peak periods (20 min. off-peak)	Main Street / Danforth Avenue (~ 95 metres / 1 min walk)	The 135 Gerrard bus route operates generally in an east-west direction, between Main Street Station and Warden Station on the Line 2 Bloor-Danforth Subway.



	Route	Headways	Closest Stop	Route Description
GO Transit	Lakeshore East GO Line – Danforth GO	30 min (commuter directions)	Danforth GO Station	Lakeshore East GO line operates between Oshawa GO in Durham Region and Union Station in Toronto, generally in an east-west direction. Service along the Lakeshore East Line is provided 7 days a week, typically between morning peak times to late evening times.

Notes:

1. Headways based on TTC Service Summary from May 12, 2024 to June 22, 2024.



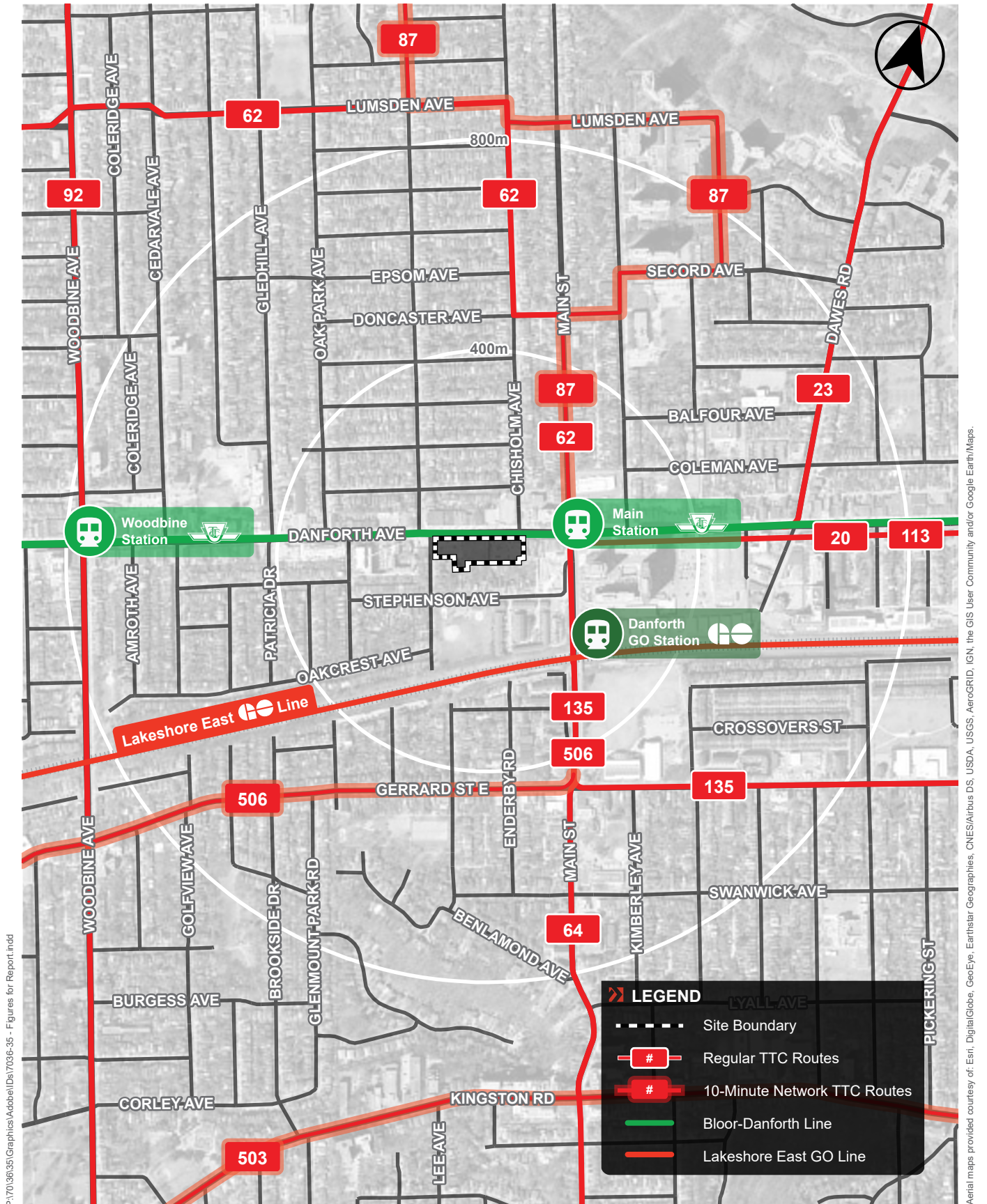


FIGURE 6 EXISTING TRANSIT NETWORK

3.2.2 Planned Transit Improvements

While the existing level of transit accessibility afforded to the site today provides convenient connectivity across the City, planned investments in public transit services will further improve the transit options for future residents and visitors of the site and surrounding area.

These include a range of large scale service improvements planned by Metrolinx as part of the GO Expansion Program. A summary of the various area transit improvements is provided in the following sections.

3.3 Area Pedestrian Connections

The site is well-situated from a pedestrian perspective, located within walking distance of numerous commercial and retail land uses along the Danforth Avenue corridor.

The intersection of Danforth Avenue / Westlake Avenue, adjacent to the site, is signalized and equipped with pedestrian signal heads, crosswalks with curb ramps, zebra pavement marking and tactile walking surfaces. Within the vicinity of the site, sidewalks are provided along both sides of all major area roads and signalized pedestrian crossings at all major intersections.

Several improvements to the pedestrian realm are proposed as part of the proposed development including widening sidewalks and eliminating curb cuts on Danforth Avenue (all vehicular access is consolidated onto Westlake Avenue).

The area pedestrian network is illustrated in Figure 7.



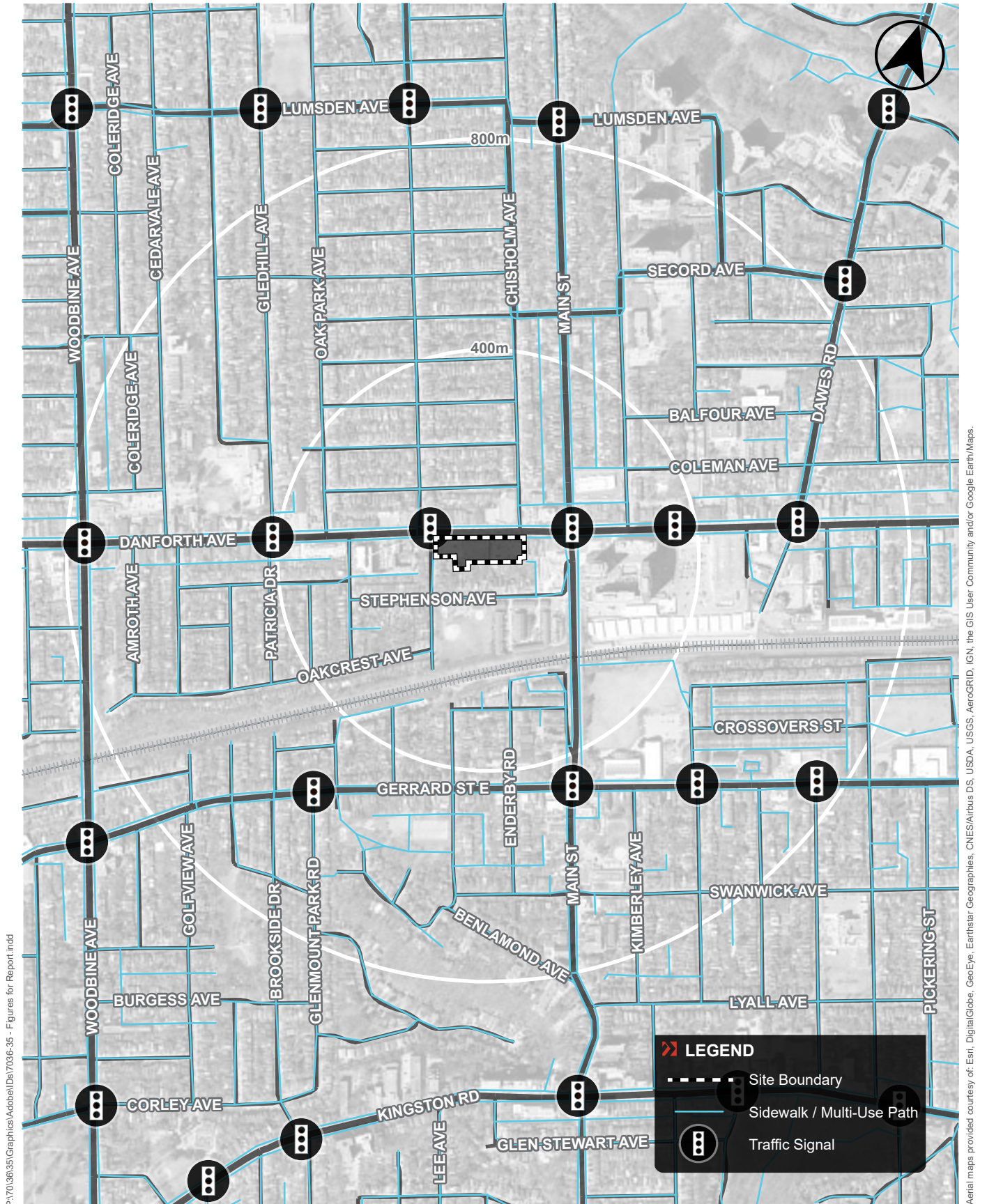


FIGURE 7 EXISTING PEDESTRIAN CONTEXT

3.4 Area Cycling Network

The site is located conveniently within one of the most connected areas of the City of Toronto's cycling infrastructure network.

3.4.1 Existing Area Cycling Network

Table 4 Existing Area Cycling Infrastructure Network

Route/ Direction		Type of Cycling Infrastructure	Description	Image (Source: Google)
East-West Connections	Danforth Avenue	Cycle Tracks	The route runs in an east-west direction generally along Bloor Street and Danforth Avenue, extending from Runnymede in the west to Dawes Road in the east. The Danforth cycle tracks were extended from Dawes Road to Victoria Park Avenue in 2022. Cycle tracks are physically separated from vehicular traffic for the majority of the route.	 Danforth Avenue (adjacent to the site) facing east
	Taylor Creek Trail	Major Multi-Use Trail	Taylor Creek Trail generally runs in an east-west direction through Taylor Creek Park and Coxwell Ravine Park. The route links to numerous bicycle networks including the Lower Don Trail, the Dawes bike lanes, and numerous signed shared roadways and quiet residential streets. The trail is separated from vehicular traffic.	
North-South Connections	Main Street	On-Street Shared Cycling Connections	South of Hamstead Avenue, Main Street is a shared roadway running in a general north-south direction along Main Street from Hamstead Avenue in the north to Alfresco Lawn in the south. The route links to the Cosburn and Dawes bike lanes to the north via additional signed shared roadway routes, and the Waterfront Trail to the south. There is no designated cycling infrastructure on the road.	
	Woodbine Avenue	Cycle Tracks	The route runs in a north-south direction along Woodbine Avenue, extending from O'Connor Drive in the north to Kingston Road in the south.	



Route/ Direction		Type of Cycling Infrastructure	Description	Image (Source: Google)
	Dawes Road	Bike Lanes	The Dawes bike lane extends from Danforth Avenue in the south to Victoria Park Avenue in the north. The lane is indicated by lines marked on the pavement. The route links to the Taylor Creek Trail, the Crescent Town Road bike lane and the local area signed shared roadways that generally run in an east-west direction. There are bike lanes on both sides of the road.	 Dawes Road at Danforth Avenue facing north

3.4.2 Planned Cycling Network

3.4.2.1 CITY OF TORONTO CYCLING NETWORK PLAN

In 2016, Toronto City Council adopted a “Ten Year Cycling Network Plan” that outlines the City’s planned and proposed investments in cycling infrastructure over the next ten years (2016-2025). The Ten-Year Plan intends to connect gaps within the existing cycling network, expand the network to new areas of the City, and renew existing routes by improving their quality. The ten-year cycling plan is guided by a Near-Term Implementation program which identifies priority projects on a triannual basis. In summer of 2024, City Council adopted the 2024 Cycling Network Plan Update Report (2025 – 2027).

Near Term Implementation Program

As part of the 2024 Cycling Network Plan Update Report (2025 – 2027), the City of Toronto has not identified any cycle infrastructure projects within the site vicinity.

Ten Year Cycling Network Implementation Plan

The Ten Year Cycling Plan, proposed cycling infrastructure on the north and south sides of Danforth Avenue. Danforth Avenue cycle lanes were completed as a part of the 2019 Cycling Network Plan Update Report. No additional cycle infrastructure projects are proposed within the Ten Year Cycling Network Implementation Plan.

The existing area cycling network in the immediate site vicinity is illustrated in Figure 8.

3.4.3 Planned Cycling Improvements

As discussed in **Section 2.0**, raised cycle tracks are proposed to replace the existing bike lane along the Danforth Avenue site frontage. This elevated design will provide better separation from traffic and improve the cycling experience.

Functional design plans are depicted in Appendix B.





FIGURE 8 EXISTING AND PROPOSED CYCLING CONTEXT

3.5 Area Share Mobility Services

3.5.1 Car-Share Facilities

Car sharing across central Toronto provides a low-commitment transportation alternative for automobile use. The success and influence of car-share programs, which were only in their infancy a decade ago, now provide convenient, non-private automobile travel opportunities for thousands of residents, employees, and visitors of the City of Toronto.

There are three primary car sharing companies operating in Toronto – ZipCar, Enterprise CarShare, and Communauto – and each offers their members access to vehicles conveniently located around the City.

There are six car-share spaces on the site today. In addition, there are 11 car-share spaces within an 800-metre radius of the site. It is proposed to maintain 2 car-share spaces on site and relocate them within the below grade parking garage.

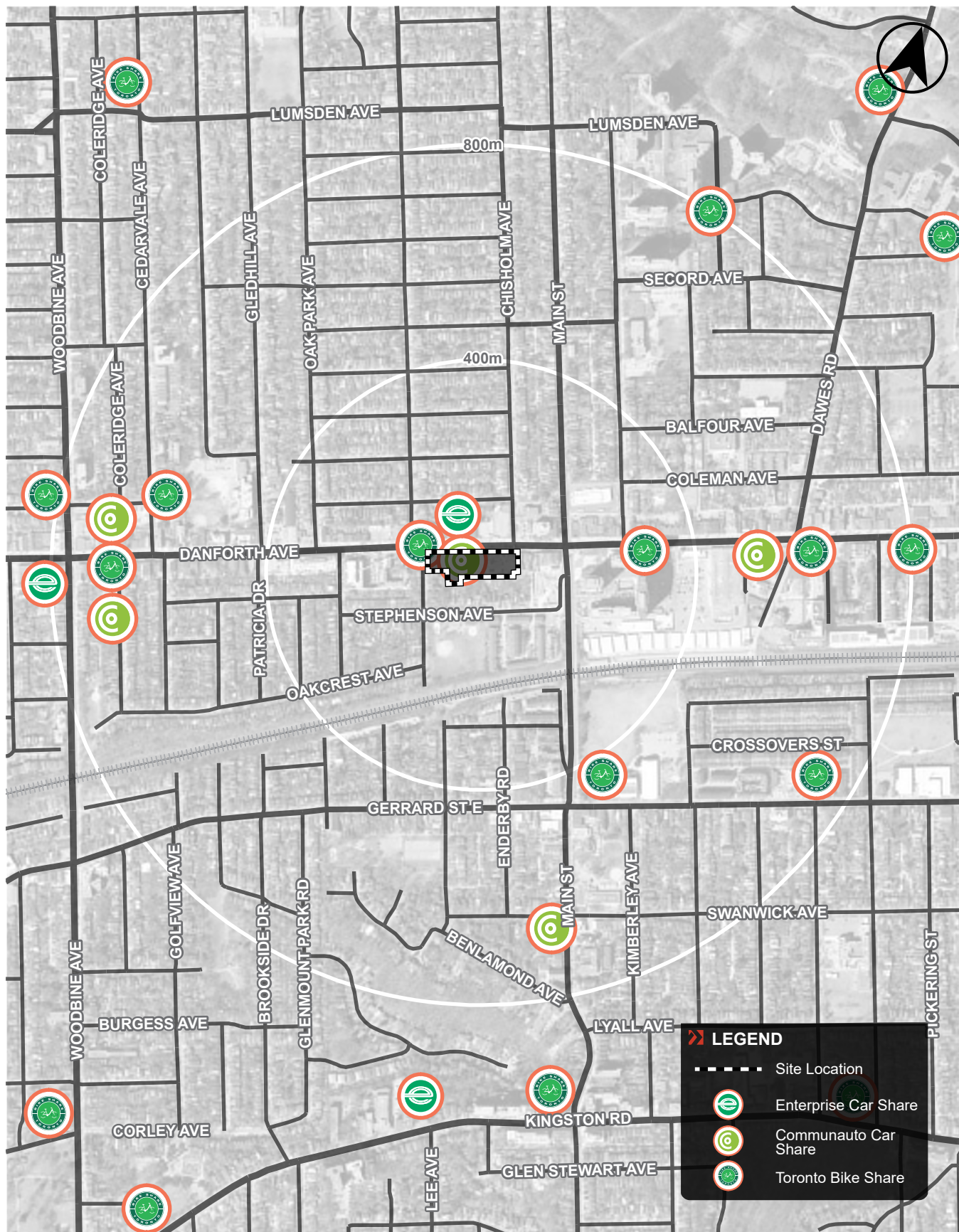
The availability of car-share facilities in the site's vicinity will provide an alternative travel resource which residents and visitors of the site can rely on when traveling to and from the site.

3.5.2 Bike Share Facilities

Within an 800-metre radius of the site, there are 8 Bike Share Toronto stations which collectively hold up to approximately 136 bicycles. The nearest existing Bike Share Toronto station is located on the western boundary of the site. The existing on-site bike share station will be relocated on-site, along the Danforth Avenue frontage in proximity to the grocery store entrance.

The locations of existing car share vehicles and Toronto Bicycle Share docking stations are illustrated in Figure 9.





Aerial maps provided courtesy of: Esri, DigitalGlobe, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, the GIS User Community and/or Google Earth/Maps.

FIGURE 9 AREA CAR-SHARE AND BIKE-SHARE FACILITIES

4.0 VEHICULAR PARKING CONSIDERATIONS

4.1 Zoning By-law Requirements

The site is subject to the parking supply requirements for developments specified in City of Toronto Zoning By-law 569-2013, as amended by By-law 89-2022. The site is located in 'Parking Zone A', as defined in By-law 89-2022.

Application of the City of Toronto Zoning By-law 569-2013 (as amended) - Parking Zone A parking standards to the Proposed Development is summarized in Table 5.

Table 5 Zoning By-law 569-2013 'Parking Zone A' Parking Requirements

Use / Type	Units / GFA ¹	Minimum Rate		Maximum Parking Permitted	
		Rate (Minimum)	Requirement (Minimum)	Rate (Maximum)	Requirement (Maximum)
Studio	139 units	None	0 spaces	0.3 spaces / unit	41 spaces
1-bedroom	266 units		0 spaces	0.5 spaces / unit	133 spaces
2-bedroom	152 units		0 spaces	0.8 spaces / unit	121 spaces
3-bedroom	63 units		0 spaces	1.0 spaces / unit	63 spaces
<i>Residential Subtotal</i>	620 units	0 spaces		358 spaces	
Residential Visitor	620 units	2 + 0.01 spaces / unit	8 spaces	1.0 space / unit for the first 5 units; and 0.1 spaces / unit for the sixth and subsequent dwelling units	66 spaces
Non-Res. Commercial	3,197m ²	None	0 spaces	3.50 sps / 100 m ² GFA	111 spaces
<i>Non-Residential Subtotal</i>		8 spaces		177 spaces	
Total				535 spaces	

Notes:

1. Based upon site statistics provided by Superkül Architects dated November 7th, 2024.
2. Zoning By-law 569-2013 [200.5.1.10 (9)] specifies that if the calculation of the number of required parking spaces results in a number with a fraction, the number is rounded down to the nearest whole number, but there may not be less than one parking space.

Application of the City of Toronto Zoning By-law 569-2013 'Parking Zone A', parking standards, results in a minimum requirement of 8 residential visitor parking spaces, and a maximum supply of 535 parking spaces (358 resident, 66 visitor, and 111 retail).



4.2 Accessible Parking Requirements

The City of Toronto Zoning By-law 569-2013 (as amended) requires accessible parking to be provided based on the effective parking space calculations in Zoning By-law 569-2013 (as amended) for Parking Zone A, are summarized in Table 6.

Table 6 Zoning By-law 569-2013 'Parking Zone A' Accessible Parking Requirements

Land Use	Units / GFA ¹	Effective Parking Rate ²	Effective Parking Spaces	Accessible Parking Spaces Required ³
Studio	139 units	0.3 spaces / unit	41 spaces	5 spaces + 1 accessible space / 50 effective parking spaces
1-bedroom	266 units	0.5 spaces / unit	133 spaces	
2-bedroom	152 units	0.8 spaces / unit	121 spaces	
3+ bedroom	63 units	1.0 spaces / unit	63 spaces	
Resident Subtotal			358 spaces	
Residential Visitor	620 units	0.10 spaces / unit	62 spaces	
Retail	3,197m ²	1.00 spaces / 100 m ² GFA	31 spaces	
Non-Resident Subtotal			93 spaces	
Total Effective Parking Spaces			451 spaces	
Total Accessible Requirement				13 spaces

Notes:

1. Based upon site statistics provided by Superkül Architects, dated August 29, 2024.
2. Application of "Effective" Parking Rate and Requirement is a procedural requirement, stipulated by By-law 569-2013, intended to calculate the required quantity of parking spaces (see Section 200.15.10.5)
3. As per Section 200.15.10.5 (1) (C) "it states: if the number of effective parking spaces is more than 100 spaces, a minimum of 5 parking spaces plus 1 parking space for every 50 effective parking spaces or part thereof in excess of 100 parking spaces must comply with all regulations for an accessible parking space in Section 200.15"

Application of the effective parking requirement would result in a minimum requirement of 13 accessible parking spaces.

4.3 Proposed Parking Supply

Vehicular parking will be provided within a two-level below-grade. A total of 278 parking spaces are proposed across the P1 and P2 level. The 86 retail spaces and visitor spaces will be located on the P1 level along with 2 car-share spaces. The remaining 190 resident parking spaces will be located across the P1 and P2 level. The proposed parking supply includes 13 accessible parking spaces located adjacent to elevator cores.

It is also proposed to relocate 2 of the existing car-share spaces on the site into the below grade parking garage.

The proposed total parking supply of 278 spaces (190 resident, 86 retail / visitor spaces and 2 car-share spaces) are within the parking minimum and maximum rates as prescribed by Zoning By-law 569-2013 and are considered appropriate.



4.4 Toronto Green Standard Version 4 Parking Requirements

As of May 1, 2022, all new developments are required to meet Toronto Green Standard (TGS) Version 4 (“TGS Version 4. The Tier 1 performance measures are required throughout the approvals process of the development. The site is subject to the “Mid to High Rise Residential and Non-Residential Version 4” standards as outlined below.

4.4.1 AQ 1.1 – Single-Occupant Vehicle Trips

This standard requires that single-occupancy auto vehicle trips generated by the site be reduced by 25 percent through various multi-modal infrastructure strategies and Transportation Demand Management (TDM) measures. This reduction is supported by the provision of a comprehensive list of infrastructure and investments in facilitating and encouraging non-auto transportation options. These measures are identified in Table 11 and **Section 8.3**, herein.

The effectiveness of individual TDM measures can vary upon a number of factors, including the site location, site context and which other TDM measures are being implemented. In the context of the latter, TDM measures should be considered to work together as a cohesive unit. For example, in some cases, the effectiveness of a TDM measure on its own may be limited, but in tandem with additional measures, their effectiveness can be increased. In this respect, it is difficult and inappropriate to assign a projected reduction to single-occupancy vehicles to each TDM measure. Rather, it is more appropriate to estimate how the TDM suite as a whole may affect change to specific transportation modes to and from the site.

The TDM measures adopted for the Proposed Development (as discussed in **Section 8.3**) have been categorized as incentives for alternative transportation modes, such as carpool, transit, bicycle, and walk to achieve the 25% reduction in single-occupancy auto vehicle trips. These measures are expected to work together to encourage positive shifts to alternative transportation modes by providing viable trip alternatives.

4.4.2 AQ 1.2 – Electric Vehicle Infrastructure

This standard requires parking spaces to be “EV ready” which refers to provision of an energized outlet capable of supplying level 2 charging or higher in accordance with Zoning By-law 569-2013. AQ 1.2 states that:

- All resident parking spaces provided for dwelling units, excluding visitor parking spaces; and
- 25 percent of resident visitor and non-resident parking spaces.

All resident parking spaces (190 spaces) will be equipped with EV infrastructure. The proposed non-residential parking supply of 88 spaces, requires that a minimum of 22 parking spaces are required to be equipped with energized outlets capable of providing Level 2 charging or higher. The Proposed Development currently illustrates 212 “energized” parking spaces, meeting the TGS Version 4, AQ 1.2 requirements.



5.0 BICYCLE PARKING CONSIDERATIONS

5.1 Bicycle Parking Requirements

The site is subject to the bicycle parking requirements for Zone 1 as per City of Toronto Zoning By-law 569-2013 as amended by 839-2022 and Toronto Green Standards Version 4 - Tier 1.

Application of the minimum bicycle requirements for Bicycle Zone 1 to the site is summarized in Table 7.

Table 7 Bicycle Parking Requirements (Zoning By-law 569-2013, Bicycle Zone 1)

Use	Number of Units / GFA ¹	Type of Space	Bicycle Space Requirements	
			Minimum Bicycle Parking Rate	Spaces Required
Resident	620 units	Short-term space	0.20 spaces / unit	124 spaces
		Long-term space	0.90 spaces / unit	558 spaces
Non-Residential, Retail	3,197 m ²	Short-term space	3 + 0.3 spaces / 100 m ² IFA	13 spaces
		Long-term space	0.2 spaces / 100 m ² IFA	7 spaces
Toronto Green Standard V.4 – AQ 2.6 Uses within 500m of Transit, Publicly Accessible spaces				10 spaces
Total				712 spaces
Residential Long-term Spaces with Energized Outlet (120 V) ³				85 spaces

Notes:

1. Based on site statistics provided by Superkül Architects, dated November 7th, 2024.
2. As per Section 230.5.1.10(2) of Zoning By-law 569-2013, if “the calculation of the minimum bicycle parking spaces for all uses results in a fraction of a bicycle parking space being required, the number of required bicycle parking spaces must be rounded up to the next whole number.”
3. AQ 2.4 of the TGS (Version 4) states to provide at least 15% of the required long-term bicycle parking spaces with an Energized Outlet (120 V) adjacent to the bicycle rack or parking space.

Application of Zoning By-law 569-2013 (as amended by 839-2022) standards to the current proposal would require the provision of a minimum of 712 bicycle parking spaces (including 137 short-term, 565 long-term spaces and 10 publicly accessible spaces). The TGS Version 4, Tier 1 minimum requirements are consistent with the Zoning By-law requirements.

5.2 Proposed Bicycle Parking Supply

A total of 714 bicycle parking spaces are provided within the P1 level. The total bicycle parking supply includes 568 long-term parking spaces and 136 short term parking spaces. As per Toronto Green Standards Version 4, an additional 10 publicly accessible short-term bicycle spaces will be provided at-grade. The proposed supply meets and exceeds the minimum bicycle parking requirement, as per Zoning By-law 569-2013.

It is also proposed to relocate the existing Toronto Bike Share spaces (located on the west side of the site) along the Danforth Avenue frontage, in proximity to the grocery store entrance.

A bicycle repair station is proposed within the short-term and long-term bicycle parking facilities.



5.3 Toronto Green Standard Version 4 Bicycle Parking Requirements

5.3.1 AQ 2.4 – Electric Bicycle Infrastructure

Residential: At least 15% of the required long-term bicycle parking spaces, or one parking space, whichever is greater, shall include an Energized Outlet (120 V) adjacent to the bicycle rack or parking space.

The architectural plans note that 84 long-term bicycle parking spaces (15% of 558 spaces) will be provided as electric bicycle parking spaces.

5.3.2 AQ 2.6 – Publicly Accessible Bicycle Parking

For all uses within 500m of transit station entrance, provide at least 10 additional publicly accessible, short-term bicycle parking spaces, at-grade on the site or within the public boulevard in addition to bicycle parking required under AQ 2.1.

The site is located within approximately 190 metres of the Main Street Subway Station and 260 metres of the Danforth GO Station entrance. As such, the provision of 10 publicly accessible short-term bicycle parking spaces will be required.

The required 10 publicly accessible bicycle parking spaces will be provided along the Danforth Avenue frontage, in proximity to the grocery store entrance.



6.0 LOADING CONSIDERATIONS

6.1 City of Toronto Zoning By-law 569-2013 Requirements

The City of Toronto Zoning By-law 569-2013 specifies loading requirements is outlined in Table 8.

Table 8 Minimum Loading Requirements – Zoning By-law 569-2013

Use	Units / GFA ¹	Range	Type of Loading Spaces			
			Type A	Type B	Type C	Type G
Residential	620 units	400 dwelling units or more	--	--	1 space	1 space
Grocery	2,258 m ²	2,000 to 4,999 m ²	1 space	1 space	--	--
Retail ²	939 m ²	500 to 1,999 m ²	--	1 space	--	--
Total			1 space	2 spaces	1 space	1 space
Total After Sharing ^{3,4}			1 space	1 space	1 space	1 space

Notes:

1. Based on site statistics provided by Superkül Architects, dated November 7th, 2024.
2. For the purposes of calculating shared loading requirement, it is assumed that the site is zoned as a commercial residential zone (i.e. CR Zone).
3. Section 40.10.90.1 (1): "In the CR zone, if a mixed use building has a minimum of 30 dwelling units, the requirement for a Type "A" loading space or a Type "B" loading space is satisfied by the provision of a Type "G" loading space".

Application of the City of Toronto Zoning By-law 569-2013 loading standards to the development programme requires the provision of 1 Type 'A', 1 Type 'B', 1 Type 'C' and 1 Type 'G'.

6.2 Proposed Loading Supply

It is proposed to meet the minimum Zoning By-law requirement for loading.

Residential and retail loading facilities (one Type 'C' and one Type 'G' space) are proposed at-grade, accessed off the site driveway. One Type C space will be provided in the western building to support the retail loading and residential moving activities. One Type G space will be provided in the eastern building and is proposed to accommodate waste and refuse pick-up for the entire site as well as retail loading and residential moving activities for the eastern building.

Dedicated loading (one Type 'A' and one Type 'B' space) will be provided to serve the proposed grocery store on site with access off the site driveway.

6.2.1 Operations and Manoeuvring

Vehicle Manoeuvring Diagrams (VMDs) have been developed which demonstrate the ability of service and delivery vehicles to manoeuvre within the site when entering / leaving the site in a forward motion. The design vehicles used to assess the proposed loading configuration are as follows:

The design arrangements of the proposed loading area and related access facilities incorporated into the current proposal are appropriate and will meet the functional manoeuvring needs of City of Toronto garbage and recycling collection and other vehicles that may wish to service the site. VMDs are provided in Appendix C and illustrate the turning movements for the design vehicles entering / exiting the proposed loading spaces.



7.0 SHORT TERM PARKING CONSIDERATIONS

7.1 Overview of Short-Term Parking and Pick-Up / Drop-Off Activities

Short-term parking facilities are spaces designated for temporary use to accommodate activities such as picking up or dropping off a passenger (e.g. Uber, Lyft, taxi, personal drop-off, etc.) or making short-term small-vehicle deliveries (e.g. scheduled couriers, food deliveries, on-demand deliveries, etc.) to a site. The emergence of auto-based shared mobility services (e.g. Uber, Lyft, ridesharing, etc.) and on-demand delivery services (e.g. Uber Eats, Amazon Flex, etc.), particularly in urban contexts, increasingly necessitates the consideration of short-term vehicular activity throughout the site planning and design process.

While not required from a Zoning By-law standpoint, there are functional requirements that guide the need to consider the provision and location of adequate space to accommodate short-term activities associated with a given site. Practically speaking, these activities can be accommodated by a variety of facility types and are in part driven by the following:

- Development size, type, and geographic location.
- Site constraints (space limitations, conflicting development needs).
- Site location within the area transportation network, for example:
 - Adjacency to Arterial, Collector, or Local Roads.
 - Presence of adjacent cycling and pedestrian infrastructure; and
 - Adjacent on-street parking regulations (e.g. parking permitted, no parking, no stopping, no standing, etc.).
- Urban design considerations (pedestrian facilities, landscaping, public realm, etc.).

Each of these factors contribute to the potential facility type(s) and location which may be suitable to accommodate short-term activities on a site, and facilities can vary to maintain compatibility with these factors while continuing to meet the site's functional needs. Examples of potentially suitable short-term vehicle facilities may include, but are not limited to:

- Designated parking spaces
- Turnaround loops
- On- or off-street layby areas
- Designated short-term activity zones
- Informal on-street parking areas (where applicable restrictions permit)
- Small loading spaces

It should be noted that short-term parking and pick-up / drop-off activities are distinct from loading and longer duration service / maintenance / moving activities in that they are conducted using relatively small vehicles (e.g. Type C1 and C2) and occur over short periods of time (typically only a couple of minutes, and not more and 30 minutes). Conversely, loading and service / maintenance activities typically occur with larger vehicles sizes (e.g. Type B and larger), over longer periods of time and are intended to be accommodated within designated spaces on-site (loading spaces). On this basis, short-term parking and pick-up / drop-off activities represent a distinct site-related activity with unique site planning and accommodation considerations relative to loading activities.



7.2 Approach

Short-term parking supply needs for the site have been established based on a review of demands at proxy sites, with similar transportation contexts to the proposed development. Accumulation rates (85th percentile, 95th percentile and peak) were derived from the surveys on a per unit basis at a range of buildings with comparable contexts, and have been applied to the proposed site to determine short-term parking needs. Given the similarity of “front-door” activities (e.g., passenger pick-up / drop-off, quick and small food or parcel deliveries, couriers, etc.) at developments across urban areas of the City, the selected proxy sites are considered to be indicative in determining “front-door” demands that can be expected at the site.

7.3 Forecasted Short-Term and Pick-up / Drop-off Accumulation

BA Group has undertaken short-term parking studies at several developments with similar transportation contexts to the site. These studies captured short-term pick-up/drop-off and parking activity within designated spaces on-site as well as on adjacent streets (including on-street parking spaces). Recorded activities were considered as short-term if they had an observed interaction with the site (e.g. passenger pick-up / drop-off, delivery, etc.), if the vehicle was small (i.e. Type C and smaller), and if the interaction with the site occurred over a short period of time (i.e. 30 minutes or less).

Summary information related to each proxy site is provided in Table 9.

Table 9 PUDO Proxy Site Survey Information

Site	Major Intersection	Survey Date	Building Type	Number of Residential Units	PUDO Facility Type
15-33 Singer Court	Sheppard Ave E / Leslie St	Thurs. April 20th, 2023	Condominium	1,131 units	Designated PUDO Area
161 Roehampton Avenue	Yonge Street / Eglinton Avenue E	Thurs., January 25, 2024	Condominium	573 units	On-Street
		Sat., January 27, 2024			
575 & 585 Bloor Street	Bloor St E / Parliament St	Thurs. June 6 th , 2024	Condominium	772 units	Designated Layby Areas & On-Street Parking
99 Broadway Avenue & 195 Redpath Avenue	Yonge Street / Eglinton Avenue E	Tues., March 28, 2023	Condominium	888 units	On-Street, Laneway Parking
39 Roehampton Avenue	Yonge Street / Eglinton Avenue E	Tues., March 28, 2023	Condominium	439 units	On-Street, UG Driveway

Surveys were conducted to derive forecast accumulation rates on a per-unit basis for the site. The forecast accumulation rate was applied to the site to determine the expected 85th percentile, 95th percentile and peak accumulation for the site. Results of the proxy surveys and application of the corresponding 85th percentile 95th percentile and peak accumulation rates to each phase of the proposed development are provided in Table 10.



Table 10 Short-Term Parking and Pick-Up / Drop-Off Accumulation Summary and Forecast

Location	Number of Residential Units	85 th Percentile Rate	95 th Percentile Rate	Peak Accumulation
Observed Accumulation per Unit				
15-33 Singer Court	1,131 unit	0.004	0.005	0.007
575 & 585 Bloor Street	772 unit	0.001	0.003	0.003
161 Roehampton Avenue	573 units	0.0017	0.0017	0.0052
		0.0017	0.0035	0.0052
99 Broadway Avenue & 195 Redpath Avenue	888 units	0.0011	0.0023	0.0045
39 Roehampton Avenue	439 units	0.0023	0.0046	0.0068
Average Accumulation Rate (Veh. / Unit)		0.002	0.003	0.005
Forecast Site Accumulation				
2451 – 2495 Danforth	620 units	1	2	3

Application of the average percentile accumulation rates to the proposed developments results in an expected demand of 1, 2 and 3 vehicles at the 85th, 95th and peak accumulation respectively.

7.4 Proposed Pick-up / Drop-Off Supply

The intent of the proposed pick-up / drop-off (PUDO) arrangements will facilitate day-to-day activities associated with the residential uses on-site, accommodating a range of “front door” activity in a convenient and reasonable manner while maintaining a strong public realm at-grade.

The PUDO demands of the site will be accommodated on-site via two designated PUDO areas on-site, outlined below:

- Tower A – PUDO activity will be accommodated in two (2) at-grade parking spaces adjacent to the lobby; and
- Tower B – PUDO activity will be accommodated in two (2) at-grade parking spaces adjacent to the lobby;

Based on the foregoing, the proposed on-site short-term parking facilities are considered appropriate to meet the short-term parking and pick-up / drop-off demands associated with the site.



8.0 TRANSPORTATION DEMAND MANAGEMENT

8.1 Mobility Choice Travel Plan

The Mobility Choice Travel Plan is proposed to guide the provision of viable alternative personal transportation options beyond the single-occupant, private automobile. This Plan intends to support the site by outlining TDM measures and the suite of strategies under consideration to promote the use of more active and sustainable transportation modes, respond to the mobility needs of residents and visitors to the site, and reduce dependence on the private automobile.

Four specific objectives define the policy framework for the Mobility Choice Travel Plan:

- Encourage the use of alternate travel modes (transit, cycling, walking);
- Increase vehicle occupancy;
- Shift travel to off-peak periods; and
- Reduce vehicle kilometres travelled.

A comprehensive framework has been developed that will serve as a guideline for the implementation of effective TDM strategies during the site design stage, as well as in its operations following the full redevelopment of the property.

8.2 Organizational Framework

The four broader objectives can be organized within the following categories:

- Facilitation of Reduced Car Ownership and Usage;
- Vehicular Parking Supply and Management;
- Encourage Transit Use;
- Encourage and Facilitate Bicycle Use;
- Enhance Pedestrian Access and Walkability;
- Land Use and Building Infrastructure; and
- Coordination, Communication, and Promotion

8.3 Mobility Plan Strategies

While strong opportunities exist in the area's infrastructure to accommodate sustainable transportation practices, the ability to fully leverage these opportunities, ensuring the success of the Mobility Plan strategies is important. To this end, Mobility Plan strategies are presented with targeted "intentions" (i.e. what it is trying to achieve and for whom), accompanied by methods of implementation. Potential strategies are then framed in the context of the site and the strategies most appropriate for the application are proposed. A summary of the mobility strategy is outlined below.

Based on the site context and proposed land uses, the following TDM strategies will be explored (as summarized in Table 11).



Table 11 Recommended Site TDM Measures

TDM Measure	Overview	Impact
Cycling Related		
Bicycle Parking	Bicycle parking to meet TGS standards.	Improved cycling convenience.
Bicycle Repair Station	Bike repair station(s) will be provided within the secure long-term bicycle parking rooms of the underground parking garage. This allows residents of the proposed building to change tires, inflate tires, adjust seat, etc.	Improved cycling convenience.
Bike-Share	A Bike Share station may be relocated on site, subject to confirmation with Bike Share Toronto.	Accommodates cycling and <i>last-mile</i> connection. Allows site residents and visitors access to a bicycle on an infrequent as-needed basis.
Transit-Related		
Provision of Transit pass	Provide pre-loaded Presto cards (1 per unit upon initial occupancy for a duration of one month with access to TTC).	Promotes transit use.
Travel Information Brochures	Provide a travel information brochure to residents providing an overview of transportation (walk, cycle, car-share, transit) in the area.	Identifies mobility choices in the area.
Pedestrian-Related		
Pedestrian connections	Enhance existing public sidewalks along site frontages as part of development program.	Improved pedestrian safety and connectivity.
Automobile Infrastructure		
Unbundling of Parking	Unbundling of unit and parking space sales will benefit potential tenants who do not need or want a parking space.	Tenants only purchase / rent parking spaces as required. Parking costs are unbundled for purchasers or renters.
EVSE Spaces	100% of the resident parking supply will be EV spaces and 25% of commercial parking supply will be EV spaces (consistent with TGS v4).	Reduces greenhouse gas emissions.
Car-Share	Maintain two existing car-share spaces and relocate them within the garage.	Reduces automobile reliance and ownership. Accommodates automobile-related needs of residents who don't own a car or require a car on an infrequent as-needed basis.



9.0 MULTI-MODAL TRAVEL DEMAND FORECASTING

The site is located within the Danforth and Main area, in proximity to a transportation network that provides significant opportunities for sustainable modes of travel (i.e. transit, walking, and cycling). As part of this study, BA Group has established travel demand forecasts for auto-based and non-auto based trips for the site. Person trip generation for the site was based on a review of trip generation rates observed at existing developments with comparable contexts and characteristics to the proposed development, mode split information is obtained from the Transportation Tomorrow Survey (TTS). Further details are provided in the following sections.

9.1 Approach and Baseline Parameters

Travel demand forecasts for the site have been developed to reflect pedestrian, cycling, and transit usage that are reflective of the existing travel characteristics of the area. The site is located adjacent to high order transit services, a site plan that supports active transportation and a mix of uses that are supportive of non-auto based travel to and from the site, particularly during the weekday peak periods of travel.

For the purposes of this analysis, travel demand to and from the site has been developed for the residential use proposed on the site with a person-trip generation methodology by applying person occupancy, modal split, direction of travel, and time of travel assumptions obtained from the 2016 Transportation Tomorrow Survey (TTS) and data collection studies conducted by BA Group. The TTS data is provided in Appendix D.

9.2 Existing Area Travel Characteristics

The existing residential travel characteristics for the site vicinity were assessed using the 2016 TTS, as summarized in Table 12.

Table 12 Area Residential Mode Share

Mode	Inbound	Outbound
Auto Driver	22%	29%
Auto Passenger	5%	3%
Rideshare/ Taxi Passenger	3%	3%
Transit	52%	50%
Walk	12%	9%
Cycle	6%	6%
Total	100%	100%

Notes:

1. Inbound travel mode shares based on afternoon peak period residential inbound trips. Outbound travel mode shares based on morning peak period residential outbound trips.
2. Data shown for 2006 TTS zones 251 and 258.



9.3 Site Residential Person Trip Generation

A person trip rate for the site was derived from the proxy sites surveyed below. As detailed above, these proxy sites were selected based on their similar transportation context. A review of the person trip generation rates for the proxy sites, provided as trips per unit, is summarized in Table 13.

Table 13 Site Residential Person Trip Rates

Proxy Site	Survey Date	Units	AM Peak Hour			PM Peak Hour		
			In	Out	2-Way	In	Out	2-Way
2055 Danforth Avenue	Tues, June 11, 2024	147	0.10	0.36	0.46	0.4	0.23	0.63
575 & 585 Bloor Street East	Thurs, June 6, 2024	772	0.07	0.27	0.34	0.29	0.2	0.49
1638 Bloor Street West	Wed, March 29, 2023	108	0.15	0.34	0.49	0.26	0.25	0.52
Average Trip Rate			0.11	0.32	0.43	0.32	0.23	0.55
Site Residential Person Trips			70	200	270	200	140	340

Notes:

1. Site statistics provided by Superkül Architects, within architectural plans dated November 7th, 2024.

Application of the adopted site person trip rate to the proposed development will result in **270** and **340** two-way residential trips during the weekday morning and afternoon peak hours, respectively.



9.4 Site Residential Trips by Travel Mode

The site residential trips by travel mode are summarized in Table 14 below.

Table 14 Residential Site Trips by Mode

Parameter						
Residential Units		620 units ¹				
		Peak Hour Travel Characteristics				
		Weekday AM Peak		Weekday PM Peak		
		Inbound	Outbound	Inbound	Outbound	
Person Trips / Unit		0.11	0.32	0.32	0.23	
Person Trips ²		70	200	200	140	
Trips by Travel Mode						
Mode	Inbound	Outbound	Weekday AM Peak		Weekday PM Peak	
Auto	22%	29%	15	60	45	40
Auto Passenger	5%	3%	5	5	10	5
Rideshare / Taxi	3%	3%	0	5	5	5
Transit	52%	50%	35	100	105	70
Walk	12%	9%	10	20	25	10
Cycle	6%	6%	5	10	10	10
Total	100%	100%	70	200	200	140
Peak Hour Traffic Demand						
			AM Peak Hour		PM Peak Hour	
			Trips	Rate	Trips	Rate
Inbound			15	0.02	45	0.07
Outbound			60	0.10	40	0.06
2-way			75	0.12	85	0.13

Notes:

1. Site statistics provided by Superkül Architects within architectural plans dated November 7th, 2024.
2. Person trips based on Table 13.

Application of the mode split results in **75** and **85** two-way auto trips during the weekday morning and afternoon peak hours, respectively.

The site is expected to generate **135** and **175** two-way transit trips during the weekday morning and afternoon respectively, **5** and **10** two-way combined trips will be accommodated as PUDO rideshare/taxi trips and **45** and **55** two-way combined walking and cycling trips during the weekday morning and afternoon peak hours, respectively.



9.5 Site Grocery Store Trips

The existing grocery store use has been included within the proposed new development with approximately the same GFA. As such, grocery store vehicle trips for the development have been assumed to remain consistent with the existing site vehicle trips. Existing vehicular trips using the Danforth Avenue driveways have been reassigned in the analysis to the proposed site driveway off Westlake Avenue. Existing site vehicle trips are summarized in Table 15 below.

Trips to and from the grocery store made using non-auto modes (walking, cycling and transit) are also assumed to remain the same.

Table 15 Site Grocery Store Vehicular Trips

	AM Peak Hour			PM Peak Hour		
	In	Out	2-Way	In	Out	2-Way
Grocery Store Trips	45	40	85	45	55	100

- Notes:
1. Determined from traffic counts conducted by Spectrum Traffic Data Inc. on behalf of BA Group on Thursday, May 16, 2024.



9.6 Toronto Green Standards (TGS) Version 4.0 – Reduction in Single-Occupant Vehicle Trips

The site is subject to the 'Mid to High Rise Residential and Non-Residential Version 4' standards as outlined in Toronto Green Standard Version 4.0, which applies to applications submitted on or after May 1, 2022. This standard requires that single-occupancy auto vehicle trips generated by the site be reduced by 25 percent through various multi-modal infrastructure strategies and Transportation Demand Management (TDM) measures.

A number of TDM measures are proposed on-site (as discussed in **Section 7.0**) to further reduce single occupancy vehicle trips and encourage other alternative, non-motorized travel through a number of strategies. Such strategies include, but not limited to, the provision of bicycle parking and repair stations, pedestrian connections, car-share spaces, and strategic proximity to an array of transit services.

A reduction is evident when comparing the vehicular trip generation forecasts for the site (outlined in Table 14) report and those calculated using the ITE Trip Generation Manual 10th Edition, outlined in Table 16.

Table 16 Trip Generation Reduction Summary

	AM Peak Hour			PM Peak Hour		
	In	Out	2-Way	In	Out	2-way
ITE Trip Rates (LUC 222 – Multifamily Housing) ¹	0.02	0.20	0.22	0.13	0.06	0.19
Number of Trips ² (620 units)	10	125	135	80	40	120
Site Trip Generation per BA Group Rates (Section 9.4)	0.024	0.01	0.12	0.07	0.06	0.13
Number of Trips ² (620 units)	15	60	75	45	40	85
Reduction	-5	65	60	35	--	35
% Reduction	--	--	57%	--	--	35%

Notes:

1. ITE Trip Rate is representative of the average peak hour vehicle trip rate for sites within dense multi-use urban centres located close to rail transit.
2. Trips rounded to the nearest 5 vehicles.

As summarized in Table 16, compared to the ITE trip generation rates for a 'multifamily high-rise' land use located within a dense multi-use urban setting, close to rail transit, the site is anticipated to generate ~ 35 - 55% less two-way vehicle trips during the critical weekday peak hours. This reduction, reflects the overall transportation context of the site that encourages non-auto trips



10.0 MULTI-MODAL TRAVEL ASSESSMENT

10.1 Transit Assessment

This section provides an assessment on the impact of the proposed residential components of the development on local transit.

10.1.1 Transit Trip Generation

The site generated transit trips, as determined by the methodology in **Section 9.0**, are summarized in Table 17 below.

Table 17 Site Transit Trips

	AM Peak Hour			PM Peak Hour		
	In	Out	2-Way	In	Out	2-Way
Site Transit Trips	35	100	135	105	70	175

As stated, the site is expected to generate **135** and **175** two-way transit trips during the weekday morning and afternoon, respectively.

10.1.2 Transit Trip Distribution

The distribution of transit trips across local routes within proximity to the site has been evaluated using data from the 2016 Transportation Tomorrow Survey (TTS) for destinations and origins of transit-trips made by residents who reside in the site area. Subsequently, these trips have been assigned to routes within the local transit network, as summarized in Table 18.

Table 18 Transit Trip Distribution

Route	Direction (To/From)	Inbound	Outbound
135 Gerrard	South	5%	0%
91 Woodbine	North	5%	5%
64 Main	South	5%	0%
506 Carleton	South	15%	10%
Danforth GO	East	0%	0%
	West	5%	10%
Line 2 – Bloor-Danforth	East	10%	10%
	West	55%	65%
Total		100%	100%

Notes:

1. Inbound travel mode shares based on afternoon peak period residential inbound trips taken by transit. Outbound travel mode shares based on morning peak period residential outbound trips taken by transit.
2. Data shown for 2006 TTS zones 251 and 258.



10.1.3 Forecast Site Transit Trips

Site transit riders to and from the site, as generated in **Section 9.4**, have been distributed to transit routes person transit trips summarized in Table 17. The number of site-generated trips assigned to local transit routes is summarized in Table 19.

Table 19 Site Transit Trip Volumes

Route	Direction (To/From)	AM Peak Hour ²	PM Peak Hour ²
92 Woodbine South	South	0(2)	0(5)
91 Woodbine	North	5(2)	4(5)
64 Main	South	0(2)	0(5)
506 Carleton	South	10(5)	7(16)
Danforth GO	East	10(5)	7(16)
	West	0(0)	0(0)
Line 2 – Bloor-Danforth	East	10(2)	7(5)
	West	10(4)	7(11)
Total		100(35)	70(105)

Notes:

1. XX(XX) – Boarding (Alighting)
2. Boarding and alighting are not rounded to account for low trip volumes on some route.

From the above, the site is expected to add a maximum of 5 riders to any surface bus route during any peak hour. The majority of transit riders are expected to utilize the Line 2 – Bloor-Danforth subway.

10.2 Pedestrian Travel Assessment

Pedestrian trips for the proposed development can be divided into the following two categories:

- Primary Pedestrian Trips – these are pedestrian trips where the primary mode of travel to their destination is walking; and
- Transit-based Pedestrian Trips – these are pedestrian trips where the primary mode of travel to their destination is transit, with walking to and from transit stops or stations.

The total number of pedestrian trips expected to be generated by the proposed development is summarized in Table 20.

Table 20 Total Site Pedestrian Volumes

	AM Peak Hour			PM Peak Hour		
	In	Out	2-Way	In	Out	2-Way
Primary Pedestrian Trips	10	20	30	25	10	35
Transit-based Pedestrian Trips	35	100	135	105	70	175
Total Pedestrian Trips	45	120	165	130	80	210



The proposed development will generate **165** and **210** total two-way pedestrian trips during the weekday morning, and weekday afternoon peak hours, respectively. A majority of these pedestrians will be travelling to Main Station to access bus and subway services, which is a less than 250 metre walk from either of the residential tower lobbies. As described in **Section 9.4**, the street network surrounding the site is made up of continuous and connected sidewalks which make up a well-connected network, providing a high level of area coverage without “gaps” or disconnected links for pedestrian travel.

10.3 Cycling Assessment

The cycling trips expected to be generated by the site as determined by the methodology in **Section 9.0** are summarized in Table 21 below.

Table 21 Site Cycling Volumes

	AM Peak Hour			PM Peak Hour		
	In	Out	2-Way	In	Out	2-Way
Cycling Trips	5	10	15	10	10	20

As described in **Section 9.4**, the site is in proximity to several cycling routes that will encourage cycling as a viable mode of travel to and from the site for experienced and non-experienced riders alike. In particular, separated cycling lanes along Danforth Avenue provide cyclists with many destinations and connections to other cycling routes throughout the city.



11.0 TRAFFIC VOLUMES

11.1 Analysis Scenarios and Horizons

The travel demand forecast and traffic analysis have been completed for the following weekday morning and afternoon peak hour scenarios:

- **Existing Traffic** – volumes on the road network under existing conditions.
- **5-year Future Background Traffic** – volumes in the future prior to build-out of the site which considers area growth.
- **5-year Future Total Traffic** – volumes in the future after build-out of the site, inclusive of area background growth.

11.2 Existing Traffic Volumes

Base existing traffic volumes were established for the weekday morning and afternoon hours, the busiest hour of traffic between 7:30 a.m. – 9:30 a.m., and 4:00 p.m. – 6:00 p.m. respectively for intersections within the study area, based upon recent traffic count information collected by Spectrum Traffic Inc. on behalf of BA Group which are summarized in Appendix E.

Traffic count information adopted as the basis for the analysis of operational impacts of the proposed development is summarized in Table 22.

Table 22 Traffic Count Information

Intersection	Count Date	Source
Danforth Avenue / Chisholm Avenue	Thursday, May 16 th , 2024	Spectrum Traffic Inc.
Danforth Avenue / West Site Access		
Danforth Avenue / East Site Access		
Westlake Avenue / Stephenson Avenue		
Westlake Avenue / Site Access		
Westlake Avenue / Morton Road		
Danforth Avenue / Oak Park Avenue		
Danforth Avenue / Main Street		
Danforth Avenue / Westlake Avenue		
Danforth Avenue / Gledhill Avenue & Patricia Drive		

Weekday morning and afternoon peak hour existing traffic volumes are illustrated in Figure 10.



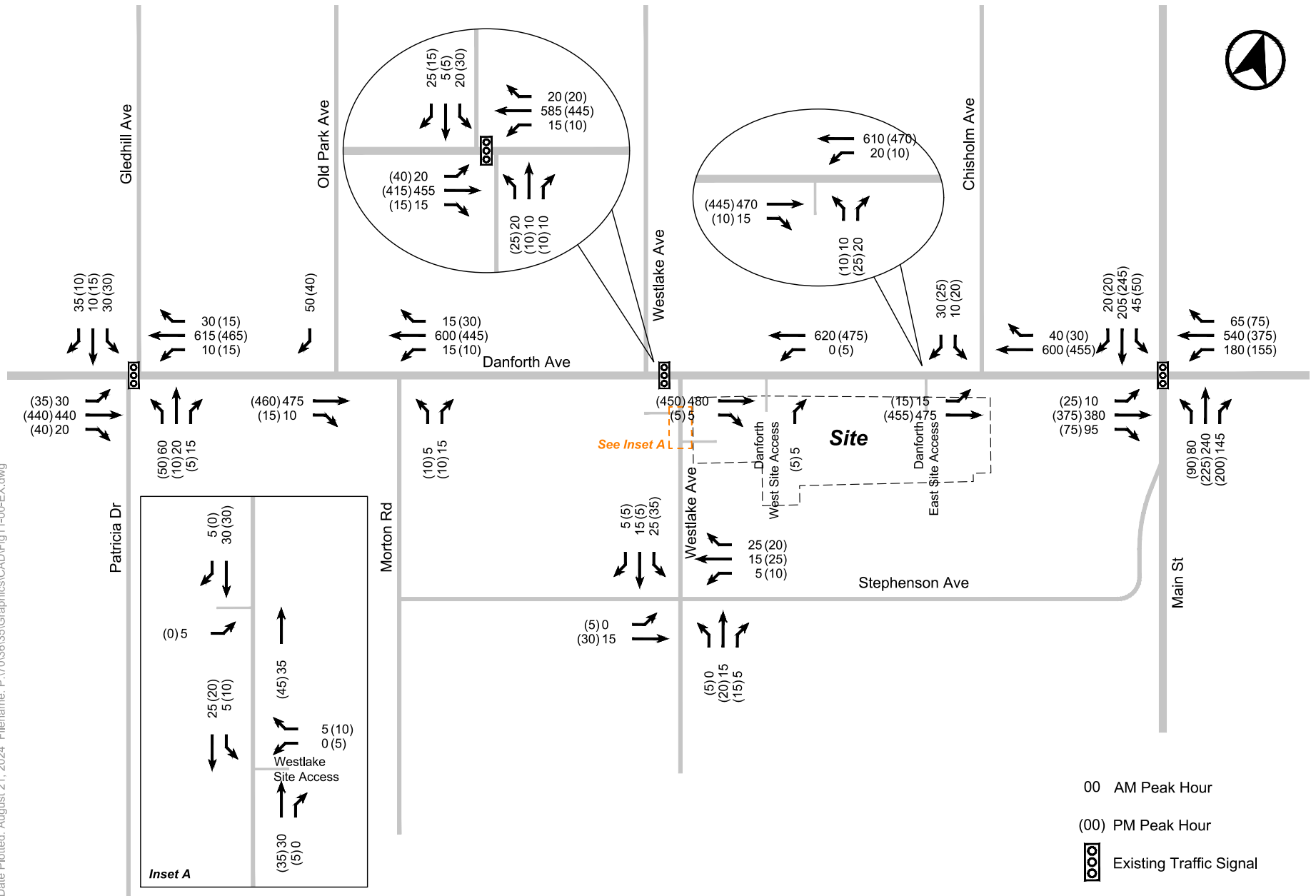


FIGURE 10 EXISTING TRAFFIC VOLUMES

11.3 Background Traffic Volumes

11.3.1 Corridor Growth

Historical traffic counts at the Main Street and Danforth Avenue intersection from 2022 to 2024 were reviewed to determine if there have been any changes in traffic volumes. A review of the trends indicates 0.5% corridor growth on Danforth Avenue during the weekday morning and afternoon peak periods. Corridor growth calculations are attached in Appendix F.

11.3.2 Background Developments

Traffic volume allowances have been made to account for area background developments that are either proposed, approved, or currently under construction in the area of the Site. A total of 11 development proposals have been considered, which represent in the order of 7,188 residential units and 10,801 m² of non-residential GFA.

Trip generation rates and traffic distributions for each background development are based on information provided in the respective traffic impact studies (TIS) that have been prepared for each project. Background developments considered in this analysis are summarized in Table 23. Background development locations are shown in Figure 11.

Table 23 Background Developments

Development	Development Statistics	Source
6 Dawes Road	1,033 res. units	LEA
8 Dawes Road	399 res. units, 451 m ² non-res GFA	BA Group
9 & 25 Dawes Road	646 res. units, 1,518 m ² non-res GFA	BA Group
10 - 30 Dawes Road	636 res. units, 2,686 m ² non-res GFA	BA Group
2494 Danforth Avenue	146 long-term care beds, 887 m ² non-res GFA	BA Group
2681 Danforth Avenue	609 res. units	IBI
2720 – 2734 Danforth Avenue	81 res. units, 256 m ² non-res GFA	LEA
2721 Danforth Avenue	1,139 res. units, 390 m ² non-res GFA	LEA
2575 - 2625 Danforth Ave	1,552 res. units, 1094 m ² non-res GFA	BA Group
276 – 294 Main St & 144 Stephenson Avenue	301 res. units, 1371 m ² non-res GFA	BA Group
985 Woodbine Avenue	646 res. units, 2,148 m ² non-res GFA	BA Group

11.3.3 Future Background Traffic

Future background traffic volumes representing the sum of existing site traffic volumes, corridor growth and background development traffic volumes are illustrated in Figure 12.





FIGURE 11 BACKGROUND AREA DEVELOPMENTS

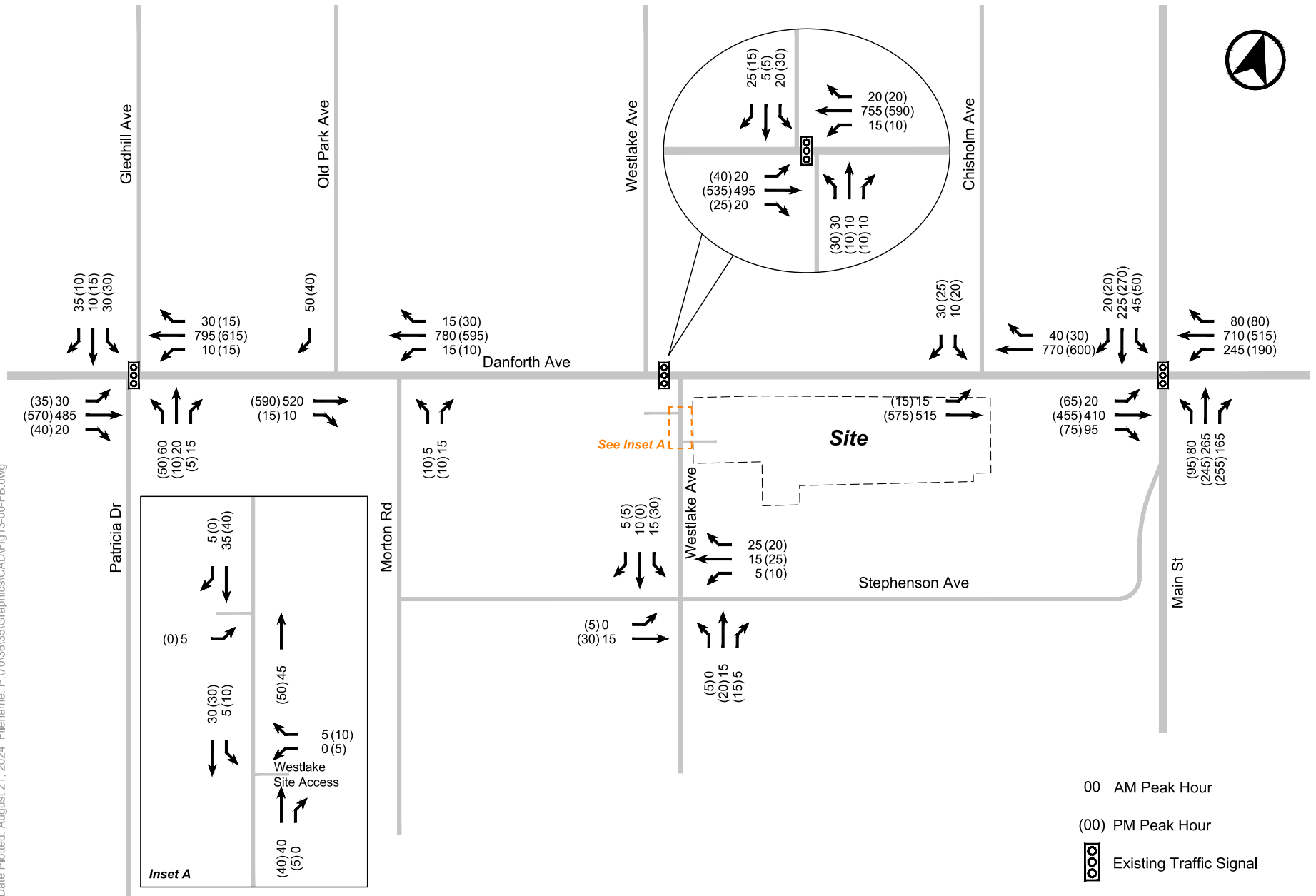


FIGURE 12 FUTURE BACKGROUND TRAFFIC VOLUMES

11.4 Site Traffic Volumes

11.4.1 Site Traffic Generation

11.4.1.1 RESIDENTIAL TRIP GENERATION

Private Vehicle Trip Generation

Residential vehicular trip generation rates for the proposed development are based on the results of the multi-modal forecasting approach described in **Section 9.0**. A review of vehicular trip rates from developments with similar unit counts and similar proximities to subway stations was undertaken to evaluate the appropriateness of results from the multi-modal forecasting approach. Proxy residential trip generation rates are summarized Table 13.

Based on **Section 9.0** and the foregoing, the site traffic generation is summarized in Table 24. Residential site uses are expected to generate **75** and **80** two-way trips in the weekday morning and afternoon peak hours, respectively.

Table 24 Residential Site Traffic Volumes

	AM Peak Hour			PM Peak Hour		
	In	Out	2-Way	In	Out	2-Way
Residential Trip Rates (per unit)	0.024	0.10	0.12	0.07	0.06	0.13
Residential Traffic Generation (620 units)	15	60	75	45	40	85

11.4.1.2 NON-RESIDENTIAL TRIP GENERATION

As stated in **Section 9.5**, the grocery store vehicle trips are expected to remain the same upon the opening of the new grocery store in the development, which has a similar planned GFA to the existing grocery store. As such, existing vehicular trips using the Danforth Avenue driveways have been reassigned in the analysis to the consolidated site driveway off Westlake Avenue. Figure 13 depicts the redistribution of grocery store site traffic.



11.4.2 Total Site Vehicle Trips

A summary of vehicle trips to and from the site is provided in Table 25.

Table 25 Total Site Vehicle Trips

	AM Peak Hour			PM Peak Hour		
	In	Out	2-Way	In	Out	2-Way
Residential Vehicle Trips	15	60	75	45	40	85
Grocery Store Trips	45	40	85	45	55	100
Total	60	100	160	90	95	185

The site will generate **160** and **180** two-way vehicle trips during the weekday morning and afternoon peak hours, respectively. It is noted that the grocery store vehicle trips are existing trips that will remain upon completion of the development. The existing grocery store trips have been retained but are consolidated to the Westlake access. Site Traffic Distribution and Assignment

Residential Trip Distribution

Residential site traffic has been assigned onto the area road network based upon a review of information queried from the 2016 Transportation Tomorrow Survey (TTS) for the site vicinity, as summarized in Table 26. Detailed TTS queries and data are attached in Appendix D.

Table 26 Site Traffic Distribution

Route	To / From	Inbound ¹	Outbound ²
Woodbine Avenue	North	45%	45%
	South	20%	20%
Danforth Avenue	East	15%	15%
	West	20%	10%
Main Street	South	0%	10%
Total		100%	100%

Notes:

1. Inbound distribution based on afternoon peak period residential inbound trips ending in TTS zones 250 and 251.

2. Outbound distribution based on morning peak period residential outbound trips originating in TTS zones 250 and 251.

Residential site traffic distributed onto the area road network is illustrated in Figure 14. As discussed in **Section 11.4**, future non-residential site traffic is conservatively assumed to remain the same as today.

11.5 Future Total Traffic Volumes

Future total traffic volumes, which represent the sum of existing traffic volumes, corridor growth background development traffic, redistributed grocery store traffic and new site traffic volumes are illustrated in Figure 15.



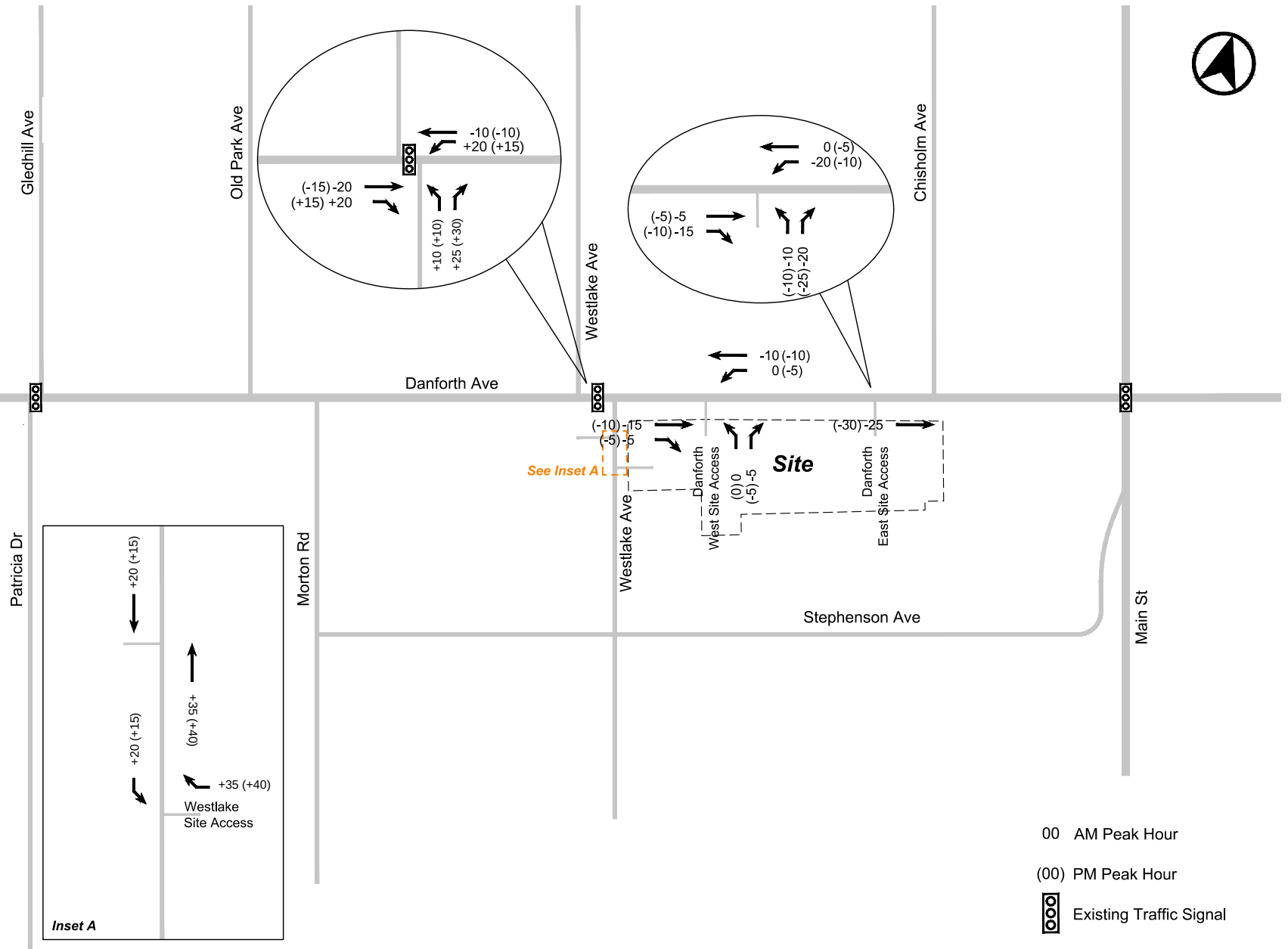


FIGURE 13 EXISTING SITE TRAFFIC VOLUMES TO BE REDISTRIBUTED TO WESTLAKE SITE ACCESS

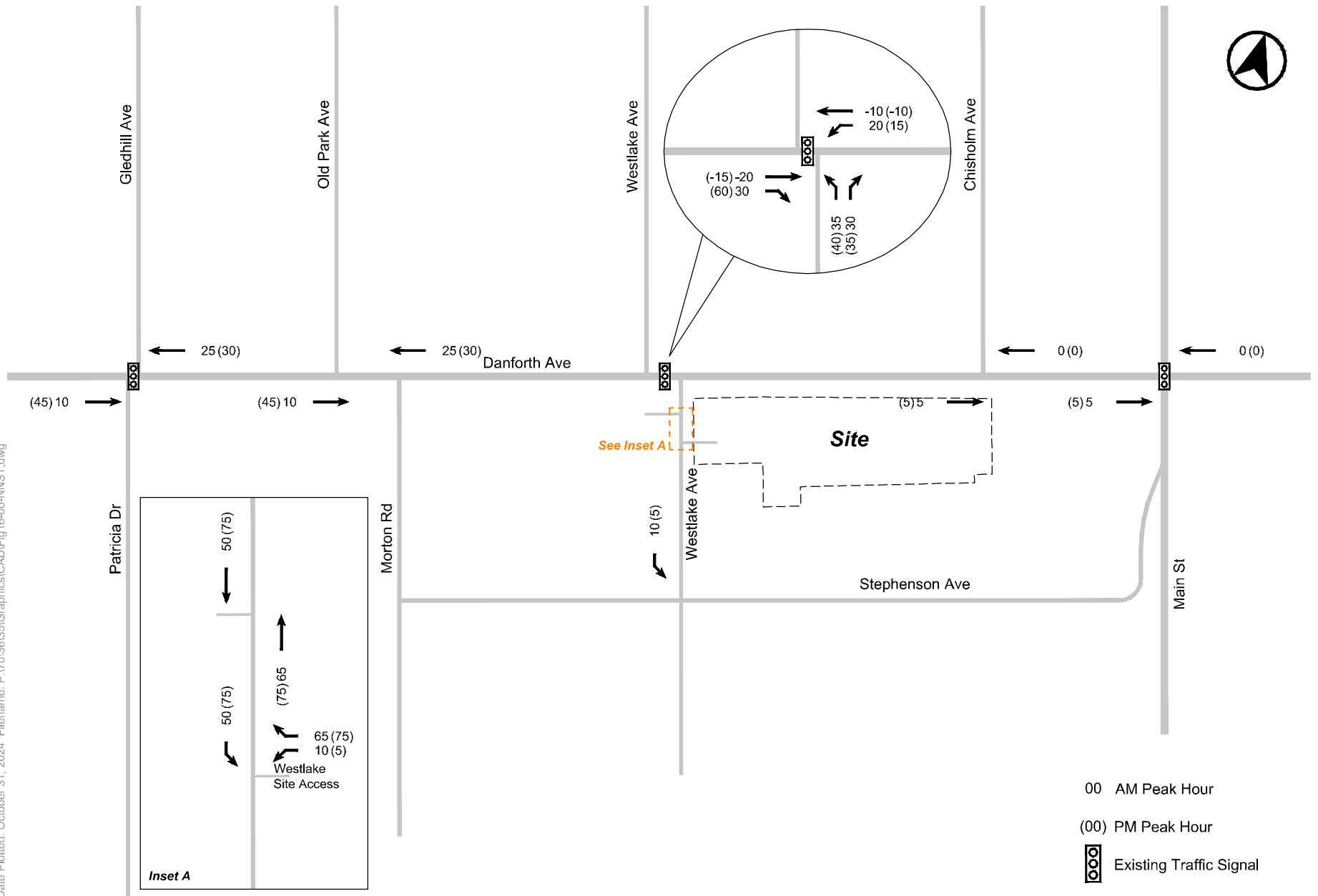


FIGURE 14 NET NEW SITE TRAFFIC VOLUMES

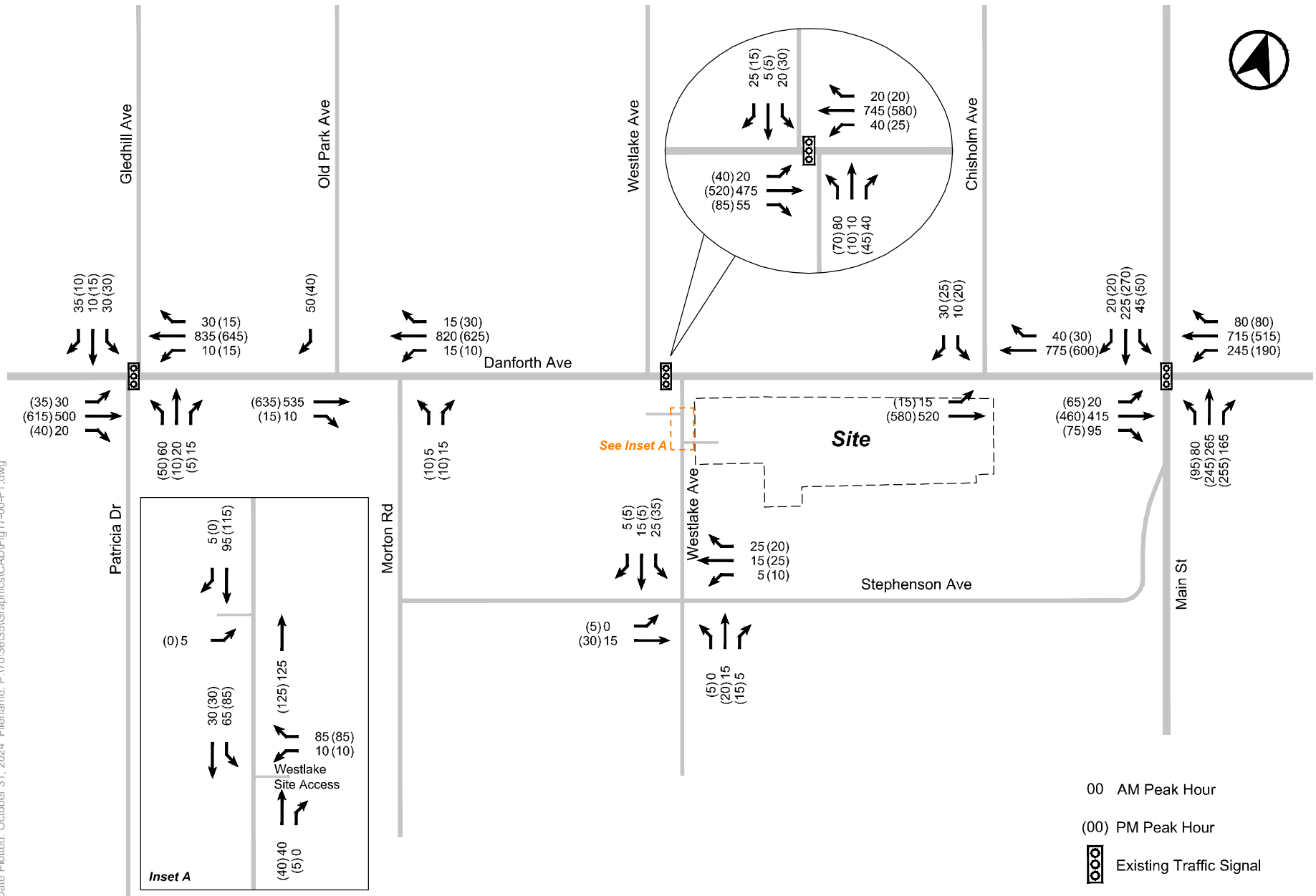


FIGURE 15 FUTURE TOTAL TRAFFIC VOLUMES

12.0 TRAFFIC OPERATIONS ANALYSIS

12.1 Analysis Methodology

The traffic operations analysis has been undertaken at the area intersections using standard capacity analysis procedures as follows:

Signalized Intersections:

The analysis undertaken at intersections operating under traffic signal control has been undertaken using the methodologies and procedures outlined in the Highway Capacity Manual (HCM) 2000, and in accordance with the City of Toronto's guidelines for analysis undertaken using Synchro 11.0 software. The product of the signalized intersection evaluation is an intersection performance index (volume to capacity ratio or v/c), where a v/c index of 1.00 indicates 'at or near capacity' conditions.

HCM level of service (LOS) criteria for unsignalized intersections is as follow:

- LOS A: Control Delay ≤ 10 s
- LOS B: $10\text{s} < \text{Control Delay} \leq 20$ s
- LOS C: $20\text{s} < \text{Control Delay} \leq 35$ s
- LOS D: $35\text{s} < \text{Control Delay} \leq 55$ s
- LOS E: $55\text{s} < \text{Control Delay} \leq 80$ s
- LOS F: Control Delay > 80 s

Unsignalized Intersections:

The unsignalized intersection analysis was completed using standard capacity procedures for intersections operating under "two-way" and "all-way" stop control and in accordance with the methodologies outlined in the Highway Capacity Manual 2000 (HCM2000).

The product of this analysis is a level of service (LOS) designation, ranging from LOS of A to F; which provides a relative indication of the level of delay experienced by motorists completing a turning manoeuvre at an intersection. LOS A represents conditions under which motorists would experience little delay and LOS F reflects conditions where more extended delays can be expected.

HCM level of service (LOS) criteria for unsignalized intersections is as follows:

- LOS A: Control Delay ≤ 10 s
- LOS B: $10\text{s} < \text{Control Delay} \leq 15$ s
- LOS C: $15\text{s} < \text{Control Delay} \leq 25$ s
- LOS D: $25\text{s} < \text{Control Delay} \leq 35$ s
- LOS E: $35\text{s} < \text{Control Delay} \leq 50$ s
- LOS F: Control Delay > 50 s

12.2 Network-Wide Parameters

Key analysis parameters were assumed based on requirements contained in the City of Toronto's *Guidelines for Using Synchro 11 (Including SimTraffic 11)* (January 2021), summarized as follows:

Network Assumptions

The existing area road network lane configuration and traffic control are illustrated in Figure 3.



Lane Widths

In accordance with the City of Toronto's *Guidelines for Using Synchro 11 (Including SimTraffic 11)* (January 2021), through and turning lanes are modelled with lane widths of 3.5 metres and 3.0 metres, respectively.

Traffic Signal Timings

Existing traffic signal timings, phasing plans and cycle lengths were obtained from the City of Toronto. Existing traffic signal timings were maintained during the analysis of future conditions. Existing traffic signal timings adopted as the basis for the traffic operations analysis are provided in Appendix G.

Base Saturation Flow Rates

The City of Toronto *Guidelines for Using Synchro 11 (Including SimTraffic 11)* (January 2021), specifies a base saturation flow rate of 1,900 passenger cars per hour of green time per lane (pcphgpl) for signalized and unsignalized intersections. These default rates were adopted in the analysis for the proposed development.

Heavy Vehicle Assumptions

Heavy and medium truck percentages incorporated into the analysis were based upon information provided as part of intersection turning movement counts.

Lost Time Adjustments

The City of Toronto *Guidelines for Using Synchro 11 (Including SimTraffic 11)* (January 2021), specify a base lost time adjustment factor of -1.0 seconds (i.e. a total loss time per phase equal to the amber plus all-red time minus 1 second). This default value was adopted in the analysis.

Peak Hour Factors

The City of Toronto *Guidelines for Using Synchro 11 (Including SimTraffic 11)* (January 2021), specifies that default peak hour factors should be used except where site-specific values can be calculated from existing traffic count information. These guidelines specify that a default peak hour factor of 0.90 should be used for through and turn movements during the weekday morning peak hour; and 0.95 for the through movements and 0.90 for turn movements during the weekday afternoon peak hour.

The City of Toronto default values were used in the analysis of the study intersections under future conditions.

13.0 SIGNALIZED INTERSECTION ANALYSIS

Traffic operations analysis results and discussion for the area signalized intersections for the existing, future background and future total conditions are summarized in the following sections. Detailed capacity analysis calculation worksheets for signalized intersections are attached in Appendix H.

13.1.1 Danforth Avenue / Main Street

The results of the signalized intersection traffic operations analysis undertaken for the Danforth Avenue / Main Street are summarized in Table 27.



Table 27 Danforth Avenue / Main Street

Lane Group	Existing		Future Background		Future Total	
	V/C	LOS	V/C	LOS	V/C	LOS
EBL	0.04 (0.09)	B (C)	0.10 (0.24)	B (C)	0.13 (0.24)	B (C)
EBTR	0.82 (0.76)	D (D)	0.87 (0.85)	D (D)	0.88 (0.86)	D (D)
WBL	0.55 (0.50)	B (B)	0.74 (0.71)	C (C)	0.74 (0.71)	C (C)
WBT	0.57 (0.41)	B (B)	0.72 (0.54)	B (B)	0.72 (0.54)	B (B)
WBR	0.07 (0.08)	A (B)	0.08 (0.09)	A (B)	0.08 (0.09)	A (B)
NBTLR	0.61 (0.65)	C (C)	0.69 (0.76)	C (D)	0.70 (0.77)	C (D)
SBTLR	0.37 (0.42)	C (C)	0.43 (0.50)	C (C)	0.43 (0.50)	C (C)
Overall	0.74 (0.70)	C (C)	0.83 (0.81)	C (C)	0.83 (0.82)	C (C)

Note:

1. 00 (00): Weekday morning peak hour (Weekday afternoon peak hour).

Under existing conditions, the intersection operates under acceptable conditions with an overall volume-to-capacity (v/c) ratio of 0.74 during the weekday morning peak hour and 0.70 during the afternoon peak hour.

Under future background conditions, with the addition of background traffic, the intersection will continue to operate under acceptable conditions with overall v/c ratios of 0.83 and 0.81 during the morning and afternoon peak hours, respectively.

Under future total conditions, with the addition of site-related traffic, the intersection will continue to acceptable conditions with overall v/c ratios of 0.83 and 0.82 during the morning and afternoon peak hours, respectively. The addition of site traffic has negligible impacts on the overall intersection operations. All individual movements and the intersection overall continue to operate at acceptable levels of service and within capacity.

13.1.2 Danforth Avenue / Westlake Avenue

The results of the signalized intersection traffic operations analysis undertaken for the Danforth Avenue / Westlake Avenue are summarized in Table 28.



Table 28 Danforth Avenue / Westlake Avenue Summary

Lane Group	Existing		Future Background		Future Total	
	V/C	LOS	V/C	LOS	V/C	LOS
EBL	0.05 (0.08)	A (A)	0.07 (0.09)	A (A)	0.09 (0.11)	B (A)
EBTR	0.40 (0.35)	B (A)	0.44 (0.45)	B (A)	0.53 (0.59)	B (B)
WBL	0.03 (0.02)	A (A)	0.04 (0.02)	A (A)	0.10 (0.07)	B (B)
WBT	0.48 (0.35)	B (A)	0.63 (0.46)	A (B)	0.71 (0.52)	B (B)
WBR	0.03 (0.03)	A (A)	0.03 (0.03)	A (A)	0.03 (0.03)	B (B)
NBTLR	0.18 (0.21)	D (D)	0.25 (0.24)	D (D)	0.43 (0.36)	C (C)
SBTLR	0.16 (0.24)	C (D)	0.17 (0.24)	C (D)	0.11 (0.15)	C (C)
Overall	0.44 (0.33)	B (A)	0.57 (0.43)	B (B)	0.63 (0.53)	B (B)

Note:

1. 00 (00): Weekday morning peak hour (Weekday afternoon peak hour).

Under existing conditions, the intersection operates under acceptable conditions with an overall volume-to-capacity (v/c) ratio of 0.44 during the weekday morning peak hour and 0.33 during the afternoon peak hour.

Under future background conditions, with the addition of background traffic, the intersection will continue to operate acceptable conditions with overall v/c ratios of 0.57 and 0.43 during the morning and afternoon peak hours, respectively.

Under future total conditions, with the addition of site-related traffic, the intersection will continue to operate under acceptable conditions with overall v/c ratios of 0.63 and 0.53 during the morning and afternoon peak hours, respectively. The addition of site traffic has modest impacts on the overall intersection operations. All individual movements and the intersection overall continue to operate at acceptable levels of service and within capacity.

Modest improvements in the SBTLR v/c ratio occur in the future total scenario as compared to the future background scenario. Under the future background scenario, the north-south volumes are sufficiently low such that the signal is not actuated at the 10th and 30th percentiles. Under the future total scenario, site traffic adds additional northbound volumes which actuate the north-south phases of the signal at the 10th and 30th percentile. Actuated signal results for Danforth Avenue and Westlake Avenue are included in **Appendix H**.



13.1.3 Danforth Avenue / Gledhill Avenue / Patricia Drive

The results of the signalized intersection traffic operations analysis undertaken for the Danforth Avenue / Gledhill Avenue / Patricia Drive are summarized in Table 29.

Table 29 Danforth Avenue / Gledhill Avenue / Patricia Drive

Lane Group	Existing		Future Background		Future Total	
	V/C	LOS	V/C	LOS	V/C	LOS
EBL	0.09 (0.08)	A (A)	0.11 (0.09)	A (A)	0.12 (0.09)	A (A)
EBTR	0.45 (0.42)	A (A)	0.49 (0.54)	A (B)	0.51 (0.58)	A (B)
WBL	0.03 (0.03)	A (B)	0.03 (0.04)	A (B)	0.03 (0.04)	A (A)
WBTR	0.32 (0.22)	A (B)	0.41 (0.29)	A (B)	0.43 (0.30)	A (B)
NBTLR	0.38 (0.25)	C (C)	0.38 (0.25)	C (C)	0.38 (0.25)	C (C)
SBTLR	0.21 (0.18)	C (C)	0.21 (0.18)	C (C)	0.22 (0.18)	C (C)
Overall	0.43 (0.38)	A (B)	0.47 (0.47)	A (B)	0.48 (0.50)	A (B)

Note:

1. 00 (00): Weekday morning peak hour (Weekday afternoon peak hour).

Under existing conditions, the intersection operates under acceptable conditions with an overall volume-to-capacity (v/c) ratio of 0.43 during the weekday morning peak hour and 0.38 during the afternoon peak hour.

Under future background conditions, with the addition of background traffic, the intersection will continue to operate under acceptable conditions with overall v/c ratios of 0.47 during the morning and afternoon peak hours, respectively.

Under future total conditions, with the addition of site-related traffic, the intersection will continue to operate acceptable conditions with overall v/c ratios of 0.48 and 0.50 during the morning and afternoon peak hours, respectively.



13.2 UNSIGNALIZED INTERSECTION ANALYSIS

Unsignalized intersection operations were analyzed under existing, future background and future total traffic conditions for both the morning and afternoon peak hours. Detailed Synchro analysis sheets are attached in Appendix H.

13.2.1 Danforth Avenue / Chisholm Avenue

The intersection of Danforth Avenue / Chisholm Avenue operates with stop control on Chisholm Avenue and free flow on Danforth Avenue. The intersection of Danforth Avenue / Chisholm Avenue is expected to operate acceptably (range of LOS A to LOS C), under the future total scenario. The results of the traffic operations analysis are summarized in Table 30.

Table 30 Danforth Avenue / Chisholm Analysis Summary

Lane Group	Existing		Future Background		Future Total	
	Delay (s)	LOS	Delay (s)	LOS	Delay (s)	LOS
EBTL	0.6 (0.6)	A (A)	0.8 (0.7)	A (A)	0.8 (0.7)	A (A)
WBT	0.0 (0.0)	A (A)	0.0 (0.0)	A (A)	0.0 (0.0)	A (A)
WBR	0.0 (0.0)	A (A)	0.0 (0.0)	A (A)	0.0 (0.0)	A (A)
SBLR	19.0 (20.9)	C (C)	25.7 (27.1)	D (D)	23.8 (24.0)	C (C)

Note:

1. 00 (00): Weekday morning peak hour (Weekday afternoon peak hour).

13.2.2 Westlake Avenue / Westlake Site Access

The intersection of Westlake Avenue / Westlake Site Access operates with stop control on the Site access and free flow on Westlake Avenue. The intersection of Westlake Avenue / Westlake Site Access is expected to operate acceptably (range of LOS A), under the future total scenario. The results of the traffic operations analysis are summarized in Table 31.

Table 31 Westlake Avenue / Westlake Site Access Analysis Summary

Lane Group	Existing		Future Background		Future Total	
	Delay (s)	LOS	Delay (s)	LOS	Delay (s)	LOS
WBLR	8.6 (9.0)	A (A)	8.7 (9.1)	A (A)	9.3 (9.5)	A (A)
NBTR	0.0 (0.0)	A (A)	0.0 (0.0)	A (A)	0.0 (0.0)	A (A)
SBTL	1.2 (2.5)	A (A)	1.0 (1.9)	A (A)	5.2 (5.7)	A (A)

Note:

1. 00 (00): Weekday morning peak hour (Weekday afternoon peak hour).

13.2.3 Westlake Avenue / Stephenson Avenue

The intersection of Westlake Avenue / Stephenson Avenue operates as an all-way stop control. The intersection of Westlake Avenue / Stephenson Avenue is expected to operate acceptably (range of LOS A), under the future total scenario. The results of the traffic operations analysis are summarized in Table 32.



Table 32 Westlake Avenue / Stephenson Avenue Analysis Summary

Lane Group	Existing		Future Background		Future Total	
	Delay (s)	LOS	Delay (s)	LOS	Delay (s)	LOS
EBTLR	7.1 (7.3)	A (A)	7.1 (7.3)	A (A)	7.2 (7.4)	A (A)
WBTLR	7.0 (7.2)	A (A)	7.0 (7.2)	A (A)	7.0 (7.2)	A (A)
NBTLR	7.1 (7.1)	A (A)	7.1 (7.1)	A (A)	7.2 (7.1)	A (A)
SBTLR	7.3 (7.3)	A (A)	7.4 (7.4)	A (A)	7.5 (7.5)	A (A)

Note:

1. 00 (00): Weekday morning peak hour (Weekday afternoon peak hour).

13.2.4 Morton Road / Danforth Avenue / Oak Park Avenue

The intersection of Morton Road / Danforth Avenue / Oak Park Avenue operates with stop control on Oak Park Avenue and Morton Road and free flow on Danforth Avenue. The intersection of Morton Road / Danforth Avenue / Oak Park Avenue is expected to operate acceptably (range of LOS A to LOS D), under the future total scenario. The results of the traffic operations analysis are summarized in Table 33.

Table 33 Morton Road / Danforth Avenue / Oak Park Avenue Analysis Summary

Lane Group	Existing		Future Background		Future Total	
	Delay (s)	LOS	Delay (s)	LOS	Delay (s)	LOS
EBTLR	0.0 (0.0)	A (A)	0.0 (0.0)	A (A)	0.0 (0.0)	A (A)
WBTLR	0.5 (0.4)	A (A)	0.6 (0.4)	A (A)	0.6 (0.5)	A (A)
NBLR	17.4 (20.7)	C (C)	20.9 (25.8)	C (D)	22.5 (26.3)	C (D)
SBLR	0.0 (0.0)	A (A)	0.0 (0.0)	A (A)	0.0 (0.0)	A (A)

Note:

1. 00 (00): Weekday morning peak hour (Weekday afternoon peak hour).



14.0 SUMMARY AND CONCLUSIONS

BA Group has been retained by First Capital to provide transportation consulting services in relation to a concurrent Official Plan Amendment (OPA) and Zoning By-law Amendment (ZBA) being made to the City of Toronto for a proposed redevelopment of the site municipally known as 2451 – 2495 Danforth Avenue. The site is located on the southeast corner of the Danforth Avenue and Westlake Avenue intersection, approximately 100 metres west of the intersection of Danforth Avenue and Main Street.

The site is currently occupied by a grocery store that fronts onto Danforth Avenue abutted by surface parking lots on the east and west sides. The site currently has three driveway access, two connecting to Danforth Avenue and one connecting to Westlake Avenue. The mixed-use development proposal includes 620 residential units and 3,197 m² GFA of non-residential. The non-residential component includes 2,258 m² GFA grocery store and 939 m² GFA of at-grade retail spaces.

It is proposed to maintain the general location of the existing site driveway on Westlake Avenue which will provide access to the underground parking, pick-up/drop-off spaces and loading facilities. A primary pedestrian access to the east residential building is provided from Danforth Avenue and a primary pedestrian access to the west residential building is provided from Westlake Avenue.

Site Context

1. The site consists of a rectangular shaped plot of land in the southeast quadrant of the Danforth Avenue and Westlake Avenue intersection. It is bounded by Danforth Avenue to the north, existing low-rise commercial developments to the east, existing 2-storey residential homes to the south, and Westlake Avenue to the west. The site currently consists of a grocery store (Sobeys) and its associated parking lot to the east and west.
2. Vehicular access to the site is currently provided via 3 driveways and include:
 - An east-west driveway on the site's western property boundary, connecting to Westlake Avenue; and
 - two north-south driveway accesses along the northern property boundary connecting to Danforth Avenue.

Transportation Context

3. The site is well served by existing and planned transit, including several Toronto Transit Commission (TTC) and GO Transit services made available at Main Street Station and Danforth GO Station.
4. Notably, the site is located approximately 260 metres northwest of Danforth GO Station with access to the GO Transit Lakeshore East line. Additionally, the site is located 190 metres west of the Main Street Station and 650 metres of the Woodbine Station, currently served by the TTC Line 2 Bloor-Danforth subway, and several bus routes that utilize the stations' bus terminals.
5. While the existing level of transit accessibility afforded to the site today provides convenient connectivity across the City of Toronto, planned investments in public transit services will further improve the transit options for future residents and visitors of the site and surrounding area.
6. The level of transit service afforded to the site and surrounding area given the extent of existing and planned transit service operating from Main Street Subway Station and Danforth GO Station, will provide significant levels of convenient transit access across the City of Toronto and are strongly supportive of the proposed development.
7. Raised cycle tracks are proposed to replace the existing bike lane along the Danforth Avenue site frontage. Given site constraints, it was agreed to provide a 1.8m wide cycle track with a 0.9 metre buffer to the north (separation from traffic) and 0.3 metre buffer to the south. No changes are proposed to the eastbound travel lane.



Vehicle Parking Considerations

8. Application of the City of Toronto Zoning By-law 569-2013 'Parking Zone A', parking standards, results in a minimum requirement of 7 residential visitor parking spaces, with a zero resident and retail minimum parking requirement, and a maximum total allowance of 530 parking spaces (367 resident and 163 non-resident parking spaces).
9. Vehicular parking will be provided within a two-level below-grade parking garage. A total of 278 parking spaces are proposed across the P1 and P2 level. The 86 retail spaces and visitor spaces will be located on the P1 level along with 2 car-share spaces. The remaining 190 resident parking spaces will be located across the P1 and P2 level. The proposed parking supply includes 13 accessible parking spaces located adjacent to elevator cores.
10. The proposed total parking supply of 278 spaces (190 resident, 88 retail / visitor spaces and 2 car-share spaces) are within the parking minimum and maximum rates as prescribed by Zoning By-law 569-2013.
11. The proposed parking supply includes 13 accessible parking spaces located in close proximity to elevator cores and meets the accessible parking requirements set out by Zoning By-law 569-2013.
12. Two car-share spaces will be maintained on site and relocated to the parking garage on the P1 level in a preferable location in close vicinity to garage accesses.

Bicycle Parking Considerations

13. Application of Zoning By-law 569-2013 (as amended by 839-2022) standards to the current proposal would require the provision of a minimum of 710 bicycle parking spaces (including 136 short-term and 564 long-term spaces).
14. A total of 714 bicycle parking spaces are provided within the P1 level. The total bicycle parking supply includes 568 long-term parking spaces and 136 short-term parking spaces.
15. As per Toronto Green Standards Version 4, an additional 10 publicly accessible short-term bicycle spaces will be provided.
16. It is also proposed to relocate the existing Toronto Bike Share spaces (located on the west side of the site) along the Danforth Avenue frontage, in proximity to the grocery store entrance.

Loading and Access Considerations

17. Application of the City of Toronto Zoning By-law 569-2013 loading standards to the development programme requires the provision of 1 Type A, 1 Type B, 1 Type C and 1 Type G.
18. Residential and retail loading facilities (one Type 'C' and one Type 'G' space) are proposed at-grade, accessed off the site driveway. One Type C space will be provided in the western building to support the retail loading and residential moving activities. One Type G space will be provided in the eastern building and is proposed to accommodate waste and refuse pick-up for the entire site as well as retail loading and residential moving activities for the eastern building.
19. Dedicated loading (one Type 'A' and one Type 'B' space) will be provided to serve the proposed grocery store on site with access off the site driveway.



Short-Term Parking

20. The PUDO demands of the site will be accommodated on-site via two designated PUDO areas, outlined below:
- Tower A – PUDO activity will be accommodated in 2 at-grade parking spaces adjacent to the lobby; and
 - Tower B – PUDO activity will be accommodated in 2 at-grade parking spaces adjacent to the lobby;
21. Forecast short-term parking accumulation rates for the proposed development ranged from 1-3 vehicles (representing the 85th to peak accumulation). The proposed on-site short-term parking facilities are considered appropriate to meet the short-term parking and pick-up/drop-off demands associated with the site. w

Transportation Demand Management (TDM)

22. A TDM strategy has been developed to encourages the use of alternative travel modes (transit, cycling and walking) beyond the single-occupant, private automobile.
23. TDM measures proposed as part of the development include, but are not limited to:
- Maintain an on-site bike share station;
 - Provision of transit pass loaded with the equivalent of one month's access to the TTC;
 - Provision of pick-up / drop-off spaces;
 - Relocation of two existing car-share spaces within the garage;
 - Provision of bicycle parking adhering to the zoning by-law requirements and TGS V4.0; and
 - Enhance pedestrian realm along the Danforth Avenue and Westlake Avenue site frontages.

Multimodal Travel Demand

24. Application of the adopted site person trip rate to the proposed development will result in **270** and **340** two-way resident trips during the weekday morning and afternoon peak hours, respectively.
25. Application of the selected mode split to the site generated person residential trips results in **75** and **85** two-way auto trips during the weekday morning and afternoon peak hours, respectively.
26. The site is expected to generate **135** and **175** two-way transit trips during the weekday morning and afternoon, respectively, **5** and **10** two-way combined trips will be accommodated as PUDO taxi trips and **45** and **55** two-way combined walking and cycling trips during the weekday morning and afternoon peak hours, respectively.

Vehicular Traffic Assessment

27. Traffic allowances were made for other specific proposed developments in the area based on a review of the City of Toronto's list of current development Projects as of September 2024, a total of 11 background sites are considered.
28. These sites represent a background development total in the order of 7,188 residential units and 10,801 m² of non-residential GFA.
29. Historical weekday traffic volume counts at the Main Street and Danforth Avenue intersection from 2022 to 2024 were reviewed to determine if there have been any changes in traffic Danforth Avenue within the study area.
30. An annual corridor growth rate of 0.5% along the Danforth Avenue corridor is considered during both the AM and PM peak hours.



31. The existing grocery store generates 85 and 100 two-way trips during the AM and PM peak hour respectively. The proposed future grocery store is approximately the same GFA as the existing and therefore, future grocery store trips remain consistent with the existing conditions.
32. Given the proposed consolidation of the site accesses to Westlake Avenue, the existing two-way site traffic utilizing the Danforth Avenue accesses have been redistributed to the Westlake Avenue access.

Traffic Operations Analysis

Danforth Avenue / Main Street

33. Under existing conditions, the intersection operates under acceptable conditions with an overall volume-to-capacity (v/c) ratio of 0.74 during the weekday morning peak hour and 0.70 during the afternoon peak hour.
34. Under future background conditions, with the addition of background traffic, the intersection will continue to operate under acceptable conditions with overall v/c ratios of 0.83 and 0.81 during the morning and afternoon peak hours, respectively.
35. Under future total conditions, with the addition of site-related traffic, the intersection will continue to acceptable conditions with overall v/c ratios of 0.83 and 0.82 during the morning and afternoon peak hours, respectively.

Danforth Avenue / Westlake Avenue

36. Under existing conditions, the intersection operates under acceptable conditions with an overall volume-to-capacity (v/c) ratio of 0.44 during the weekday morning peak hour and 0.33 during the afternoon peak hour.
37. Under future background conditions, with the addition of background traffic, the intersection will continue to operate acceptable conditions with overall v/c ratios of 0.57 and 0.43 during the morning and afternoon peak hours, respectively.
38. Under future total conditions, with the addition of site-related traffic, the intersection will continue to operate under acceptable conditions with overall v/c ratios of 0.63 and 0.53 during the morning and afternoon peak hours, respectively.

Danforth Avenue / Gledhill / Patricia Drive

39. Under existing conditions, the intersection operates under acceptable conditions with an overall volume-to-capacity (v/c) ratio of 0.43 during the weekday morning peak hour and 0.38 during the afternoon peak hour.
40. Under future background conditions, with the addition of background traffic, the intersection will continue to operate under acceptable conditions with overall v/c ratios of 0.47 during the morning and afternoon peak hours, respectively.
41. Under future total conditions, with the addition of site-related traffic, the intersection will continue to operate acceptable conditions with overall v/c ratios of 0.48 and 0.50 during the morning and afternoon peak hours, respectively.
42. All movements for the unsignalized intersections operate with acceptable delays during the weekday morning and afternoon hours in all scenarios. Site traffic has a minimal impact on the delays experienced at the unsignalized intersections within the study area.

The proposed development is appropriate from a transportation perspective and can be reasonably accommodated on the area transportation network.



Appendix A: Reduced Architectural Plans



2451-2495 Danforth



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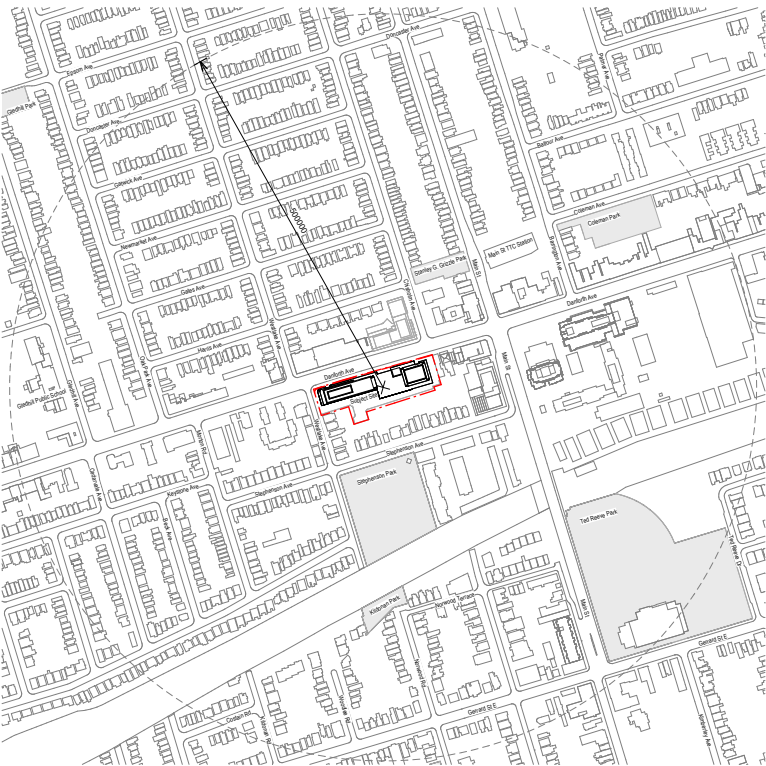
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1 Context Plan
1 : 4000

DRAWING NUMBER DRAWING TITLE		241028 - ISSUED FOR OPA and ZBA
A.000	COVER SHEET	✓
A.001	PROJECT STATISTICS	✓
A.002	PERSPECTIVE VIEWS	✓
A.003	GFA DIAGRAMS PER ZBL 569-2013	✓
A.004	GFA DIAGRAMS PER ZBL 569-2013	✓
A.005	GFA DIAGRAMS PER ZBL 569-2013	✓
A.006	SETBACK DIAGRAM	✓
A.100	SITE PLAN	✓
A.101	FLOOR PLAN - P2	✓
A.102	FLOOR PLAN - P1	✓
A.103	FLOOR PLAN - GROUND FLOOR	✓
A.104	FLOOR PLAN - LEVEL 02	✓
A.105	FLOOR PLAN - LEVEL 03	✓
A.106	FLOOR PLAN - LEVEL 04	✓
A.107	FLOOR PLAN - LEVEL 05-08	✓
A.108	FLOOR PLAN - LEVEL 09-10	✓
A.109	FLOOR PLAN - LEVEL 11-13	✓
A.110	FLOOR PLAN - LEVEL 14-35	✓
A.111	ROOF PLAN	✓
A.201	NORTH ELEVATION	✓
A.202	EAST & WEST ELEVATIONS	✓
A.203	SOUTH ELEVATION	✓
A.301	BUILDING SECTION	✓
A.302	BUILDING SECTION	✓

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Prior to commencement of the Work the Contractor shall verify all drawing dimensions, datums, and levels with the Contract Documents and with the conditions on site; ascertain any discrepancies between the site and the Contract Documents, and bring these items to the attention of the Architect for clarification.

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1 OCT 28, 2024 Issued for OPA and ZBA

No. Date Issue/Revision



2451-2495 Danforth
2451-2495 Danforth
Avenue, Toronto, ON
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Title:
COVER SHEET

Project No. 2216 Scale 1 : 4000

Drawing No.

A.000

ISSUED FOR OPA AND ZBA - OCTOBER 28, 2024



Statistics Template – Toronto Green Standard Version 4.0
City Agency, Corporation & Division-Owned Facilities

The Toronto Green Standard Version 4.0 Statistics Template is submitted with Site Plan Control Applications and stand-alone Zoning Bylaw Amendment applications. Complete the table and copy it directly onto the Site Plan submitted as part of the application.

For Zoning Bylaw Amendment applications: complete General Project Description and Section 1.
For Site Plan Control applications: complete General Project Description, Section 1 and Section 2.
For further information, please visit www.toronto.ca/greendevlopment

General Project Description	Proposed	
Total Gross Floor Area	41251.7 m²	444029.5 ft²
Breakdown of project components (m²):		
Retail GROCERY GFA:	2258.1 m²	24306.3 ft²
Commercial TOTAL RETAIL GFA:	939.2 m²	10109.6 ft² (EXCLUDING GROCERY)
Industrial		
Institutional/Other		
Total number of residential units	620	

Section 1: For Stand Alone Zoning Bylaw Amendment Applications and Site Plan Control Applications

Low Emissions Transportation	Required	Proposed	Proposed %
Number of Parking Spaces	7	278	>100%
Number of EV Parking Spaces (Residential)	190	190	100%
Number of EV Parking Spaces (Non - Residential)	22	22	100%

Cycling Infrastructure	Required	Proposed	Proposed %
Number of long-term bicycle parking spaces (all-uses)	564	568	>100%
Number of long-term bicycle parking located on:			
a) first storey of building			
b) second storey of building			
c) first level below-ground		568	100%
d) second level below-ground			
e) other levels below-ground			
Number of short-term bicycle parking spaces	146	146	100%
Number of shower and change facilities	N/A	N/A	



3-1-1
Toronto at your service Page 1 of 3



Statistics Template – Toronto Green Standard Version 4.0
City Agency, Corporation & Division-Owned Facilities

Tree Canopy	Required	Proposed	Proposed %
Total Soil Volume (40% of the site area + 66 m² x 30 m³)	1374 m³	1156 m³	84%
Soil volume provided within the site area (m³)		644 m³	
Soil Volume provided within the public boulevard (m³)		512 m³	

GCA AND GFA BREAKDOWN						PROJECT STATISTICS SUMMARY																																																																																																																																														
ABOVE GROUND	GROSS CONSTRUCTION AREA		ZBL 569-2013 EXCLUSIONS		CITY WIDE BY-LAW 569-2013 GROSS FLOOR AREA		PROJECT STATISTICS SUMMARY																																																																																																																																													
Level	GCA, m2	GCA, sf	GFA Exclusion, m2	GFA Exclusion, sf	GFA, m2	GFA, sf	NUMBER OF FLOORS:	13/35	GSI (GFA/LOT AREA):	5.54																																																																																																																																										
Ground Floor	3987.9 m²	42925.9 ft²	518.1 m²	5577.2 ft²	3469.8 m²	37348.6 ft²	GROSS CONSTRUCTION AREA (GCA) ABOVE GRADE:			47179.6 m² 507837.0 ft²																																																																																																																																										
Level02	1476.8 m²	15895.7 ft²	145.0 m²	1560.6 ft²	1331.8 m²	14435.1 ft²	TOTAL GROSS FLOOR AREA (GFA):			41251.7 m² 444029.5 ft²																																																																																																																																										
Level03	2425.8 m²	26110.8 ft²	1580.2 m²	17009.2 ft²	845.6 m²	9101.6 ft²	RESIDENTIAL GFA:			38054.4 m² 409613.6 ft²																																																																																																																																										
Level04	2245.3 m²	24168.1 ft²	147.3 m²	1585.3 ft²	2098.0 m²	22582.8 ft²	RETAIL GFA:			939.2 m² 10109.6 ft²																																																																																																																																										
Level05	2193.6 m²	23612.2 ft²	142.2 m²	1530.4 ft²	2051.5 m²	22081.8 ft²	GROCERY STORE GFA:			2258.1 m² 24306.3 ft²																																																																																																																																										
Level06	2193.6 m²	23612.2 ft²	142.2 m²	1530.4 ft²	2051.5 m²	22081.8 ft²	POPS AREA:			353.9 m² 3809.4 ft²																																																																																																																																										
Level07	2193.6 m²	23612.2 ft²	142.2 m²	1530.4 ft²	2051.5 m²	22081.8 ft²	LOT AREA:			7,724 m²																																																																																																																																										
Level08	2193.6 m²	23612.2 ft²	142.2 m²	1530.4 ft²	2051.5 m²	22081.8 ft²	CONVEYANCES AREA:			168m²																																																																																																																																										
Level09	2193.6 m²	23612.2 ft²	142.2 m²	1530.4 ft²	2051.5 m²	22081.8 ft²	ADJUSTED LOT AREA:			7,556m²																																																																																																																																										
Level10	2193.6 m²	23612.2 ft²	142.2 m²	1530.4 ft²	2051.5 m²	22081.8 ft²	TOTAL EFFICIENCY			Total Stables / above ground GCA: 80.7%																																																																																																																																										
Level11	1984.1 m²	21356.9 ft²	142.2 m²	1531.0 ft²	1841.9 m²	19825.9 ft²	NUMBER OF RESIDENTIAL UNITS			620																																																																																																																																										
Level12	1984.1 m²	21356.9 ft²	142.2 m²	1531.0 ft²	1841.9 m²	19825.9 ft²	NUMBER OF AFFORDABLE UNITS			13 (2%)																																																																																																																																										
Level13	1984.1 m²	21356.9 ft²	142.2 m²	1531.0 ft²	1841.9 m²	19825.9 ft²																																																																																																																																														
Level14/MPH	1086.9 m²	11699.1 ft²	368.1 m²	3962.3 ft²	718.8 m²	7736.8 ft²	UNIT COUNT AND MIX																																																																																																																																													
Level15	778.1 m²	8374.9 ft²	68.1 m²	732.8 ft²	710.0 m²	7642.2 ft²	BUILDING A*: See unit mix contained within table																																																																																																																																													
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Grand total	47179.6 m²	507837.0 ft²	5927.9 m²	63807.5 ft²	41251.7 m²	444029.5 ft²	<table><thead><tr><th>Name</th><th>Count</th><th>Unit Mix</th><th>Average Area, m2</th><th>Average Area, sf</th></tr></thead><tbody><tr><td>UNIT 1BD</td><td>111</td><td>46%</td><td>51.7 m²</td><td>556.8 ft²</td></tr><tr><td>UNIT 2BD</td><td>56</td><td>23%</td><td>64.6 m²</td><td>695.4 ft²</td></tr><tr><td>UNIT 3 BD</td><td>25</td><td>10%</td><td>92.1 m²</td><td>991.8 ft²</td></tr><tr><td>UNIT 4 BD</td><td>1</td><td>0%</td><td>126.9 m²</td><td>1366.1 ft²</td></tr><tr><td>UNIT STUDIO</td><td>48</td><td>20%</td><td>35.5 m²</td><td>382.3 ft²</td></tr><tr><td></td><td>241</td><td></td><td>370.9 m²</td><td>3992.5 ft²</td></tr></tbody></table>						Name	Count	Unit Mix	Average Area, m2	Average Area, sf	UNIT 1BD	111	46%	51.7 m²	556.8 ft²	UNIT 2BD	56	23%	64.6 m²	695.4 ft²	UNIT 3 BD	25	10%	92.1 m²	991.8 ft²	UNIT 4 BD	1	0%	126.9 m²	1366.1 ft²	UNIT STUDIO	48	20%	35.5 m²	382.3 ft²		241		370.9 m²	3992.5 ft²																																																																																																					
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DEFINITIONS				CAR PARKING																																																																										
From the City of Toronto ZBL 569-2013: In the Residential Zone category, the Gross Floor Area of an apartment building is reduced by the area in the building used for: (A) parking, loading and bicycle parking below established grade; (B) required loading spaces and required bicycle parking spaces at or above established grade; (C) storage rooms, washrooms, electrical, utility, mechanical and ventilation rooms in the basement; (D) shower and change facilities and bicycle maintenance facilities required by this By-law for required bicycle parking spaces; (Bylaw 610-2012); (E) indoor amenity spaces required by this By-law; (F) elevator shafts; (G) garbage shafts; (H) mechanical penthouses, and (I) stairwells in the building.				<table><thead><tr><th>Level</th><th>Visitor Parking Provided</th><th>Visitor Parking Required</th><th>Car-Share Provided</th></tr></thead><tbody><tr><td>P1</td><td>86</td><td>33</td><td>2</td></tr></tbody></table> <table><thead><tr><th>Level</th><th>Residential Parking Provided</th></tr></thead><tbody><tr><td>P2</td><td>162</td></tr><tr><td>P1</td><td>28</td></tr><tr><td></td><td>190</td></tr><tr><td></td><td>Total Parking Provided</td></tr><tr><td></td><td>278</td></tr></tbody></table>		Level	Visitor Parking Provided	Visitor Parking Required	Car-Share Provided	P1	86	33	2	Level	Residential Parking Provided	P2	162	P1	28		190		Total Parking Provided		278																																																					
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Floor Space Inlay Calculation In the Residential Zone category, the floor space index for a non-residential building is the result of the gross floor area of a building divided by the area of the lot: (A) for a residential building, other than an apartment building, is the result of the gross floor area, plus the area of an attic described in regulation 10.5.45.4(2) and subject to regulation 10.5.45.4(2)(2) minus the areas listed in regulation 10.5.45.4(2)(3), divided by the area of the lot and (B) for an apartment building, is the result of the gross floor area, minus the areas of an apartment building listed in regulation 10.5.45.4(2)(4), divided by the area of the lot.				LOADING PROVISIONS 1x TYPE 'A' LOADING SPACE 1x TYPE 'B' LOADING SPACE 1x TYPE 'C' LOADING SPACE 1x TYPE 'D' LOADING SPACE 4x PUDD PARKING SPACES *REFER TO FLOOR PLANS FOR LOCATIONS																																																																										
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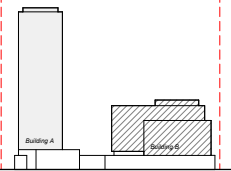
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UNIT MIX DIAGRAM

1 OCT 28, 2024 Issued for OPA and ZBA

No. Date Issue/Revision



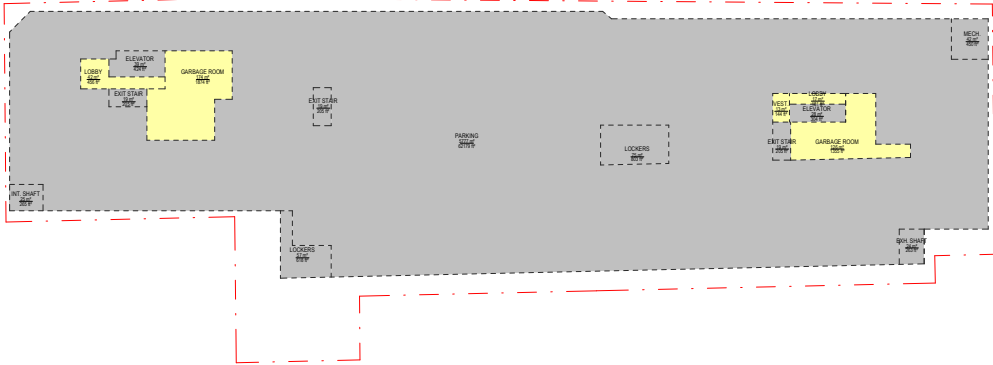
2451-2495 Danforth
2451-2495 Danforth
Avenue, Toronto, ON
M6R 2J5

Title:
PROJECT STATISTICS

Project No. 2216 Scale 1 : 1

Drawing No.

A.001

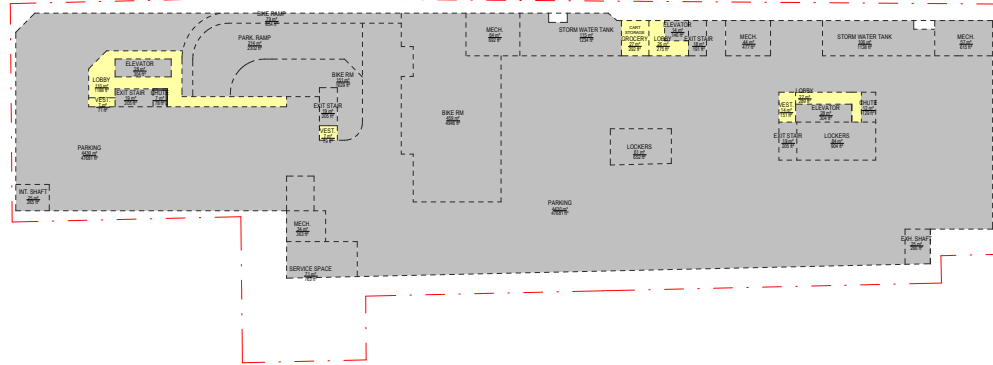


1 GFA Level P2
1 : 500

GFA EXCLUSION PER ZBL-569-2013 LEVEL P2		
Name	Area	
ELEVATOR	39.4 m²	
EXH. SHAFT	28.2 m²	
EXIT STAIR	24.4 m²	
EXIT STAIR	19.1 m²	
EXIT STAIR	19.1 m²	
EXIT STAIR	19.1 m²	
INT. SHAFT	24.6 m²	
LOCKERS	74.6 m²	
LOCKERS	57.4 m²	
MECH.	41.8 m²	
PARKING	5776.7 m²	
Grand total	6124.4 m²	

GFA PER ZBL-569-2013 LEVEL P2		
Name	Area	
GARBAGE ROOM	174.1 m²	
GARBAGE ROOM	125.9 m²	
LOBBY	42.4 m²	
LOBBY	16.8 m²	
VEST.	13.4 m²	
Grand total	372.6 m²	

GROSS CONSTRUCTION AREA (GCA) - LEVEL P2		
Name	Area	
ELEVATOR	39.4 m²	
ELEVATOR	28.2 m²	
BIKE RM	151.3 m²	
BIKE RM	26.7 m²	
CHUTE	12.0 m²	
CHUTE	7.2 m²	
ELEVATOR	28.2 m²	
ELEVATOR	28.2 m²	
EXH. SHAFT	13.5 m²	
EXH. SHAFT	24.7 m²	
EXIT STAIR	19.1 m²	
EXIT STAIR	19.1 m²	
LOCKERS	57.4 m²	
LOCKERS	16.8 m²	
MECH.	41.8 m²	
PARKING	5776.7 m²	
VEST.	13.4 m²	
Grand total	6497.0 m²	



2 GFA Level P1
1 : 500

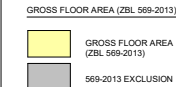
GFA EXCLUSION PER ZBL-569-2013 LEVEL P1		
Name	Area	
BIKE RAMP	79.3 m²	
BIKE RM	459.5 m²	
BIKE RM	151.3 m²	
BIKE RM	26.7 m²	
CHUTE	12.0 m²	
CHUTE	7.2 m²	
ELEVATOR	28.2 m²	
ELEVATOR	28.2 m²	
EXH. SHAFT	13.5 m²	
EXH. SHAFT	24.7 m²	
EXIT STAIR	19.1 m²	
EXIT STAIR	19.1 m²	
LOCKERS	57.4 m²	
LOCKERS	16.8 m²	
MECH.	41.8 m²	
PARKING	5776.7 m²	
VEST.	13.4 m²	
Grand total	6497.0 m²	

GFA EXCLUSION PER ZBL-569-2013 LEVEL P1		
Name	Area	
BIKE RAMP	79.3 m²	
BIKE RM	459.5 m²	
BIKE RM	151.3 m²	
BIKE RM	26.7 m²	
CHUTE	12.0 m²	
CHUTE	7.2 m²	
ELEVATOR	28.2 m²	
ELEVATOR	28.2 m²	
EXH. SHAFT	13.5 m²	
EXH. SHAFT	24.7 m²	
EXIT STAIR	19.1 m²	
EXIT STAIR	19.1 m²	
LOCKERS	57.4 m²	
LOCKERS	16.8 m²	
MECH.	41.8 m²	
PARKING	5776.7 m²	
VEST.	13.4 m²	
Grand total	6497.0 m²	

GFA PER ZBL-569-2013 LEVEL P1		
Name	Area	
GROCERY	27.1 m²	
LOBBY	110.4 m²	
LOBBY	26.8 m²	
LOBBY	25.6 m²	
VEST.	14.0 m²	
VEST.	7.3 m²	
VEST.	6.6 m²	
Grand total	217.8 m²	

GROSS CONSTRUCTION AREA (GCA) - LEVEL P1		
Name	Area	
BIKE RAMP	79.3 m²	
BIKE RM	459.5 m²	
BIKE RM	151.3 m²	
BIKE RM	26.7 m²	
CHUTE	12.0 m²	
CHUTE	7.2 m²	
ELEVATOR	28.2 m²	
ELEVATOR	28.2 m²	
EXH. SHAFT	13.5 m²	
EXH. SHAFT	24.7 m²	
EXIT STAIR	19.1 m²	
EXIT STAIR	19.1 m²	
EXIT STAIR	19.1 m²	
EXIT STAIR	19.1 m²	
GROCERY	27.1 m²	
INT. SHAFT	24.6 m²	
LOBBY	110.4 m²	
LOBBY	26.8 m²	
LOBBY	25.6 m²	

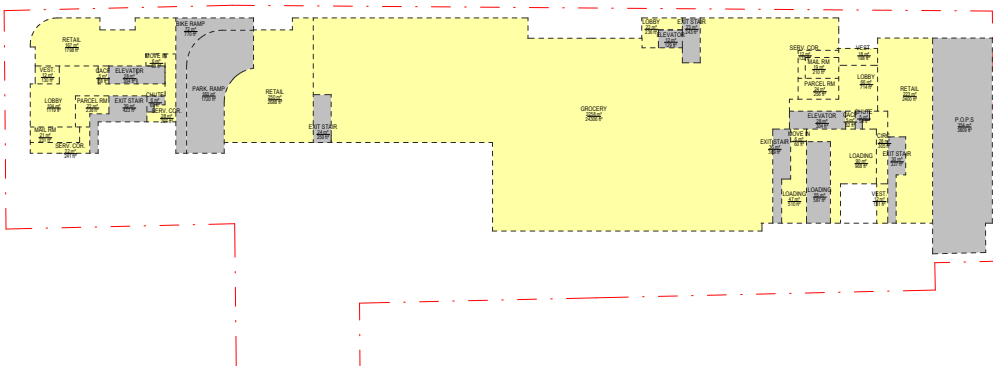
GROSS CONSTRUCTION AREA (GCA) - LEVEL P1		
Name	Area	
BIKE RAMP	79.3 m²	
BIKE RM	459.5 m²	
BIKE RM	151.3 m²	
BIKE RM	26.7 m²	
CHUTE	12.0 m²	
CHUTE	7.2 m²	
ELEVATOR	28.2 m²	
ELEVATOR	28.2 m²	
EXH. SHAFT	13.5 m²	
EXH. SHAFT	24.7 m²	
EXIT STAIR	19.1 m²	
EXIT STAIR	19.1 m²	
EXIT STAIR	19.1 m²	
EXIT STAIR	19.1 m²	
GROCERY	27.1 m²	
INT. SHAFT	24.6 m²	
LOBBY	110.4 m²	
LOBBY	26.8 m²	
LOBBY	25.6 m²	



Gross Floor Area Calculations for an Apartment Building in the Commercial Residential Zone Category per ZBL 569-2013, Section 45.5 (4) (b)

In the Commercial Residential Zone category the gross floor area of an apartment building is reduced by the area in the building used for:

- (A) parking, loading and bicycle parking below-ground;
- (B) required loading spaces at the ground level and required bicycle parking spaces at or above ground;
- (C) storage rooms, mechanical, utility, mechanical and ventilation rooms in the basement;
- (D) storage and change facilities required by the By-law for required bicycle parking spaces;
- (E) amenity space required by the By-law;
- (F) elevator shafts;
- (G) garbage shafts;
- (H) mechanical penthouses; and
- (I) wet stairwells in the building.

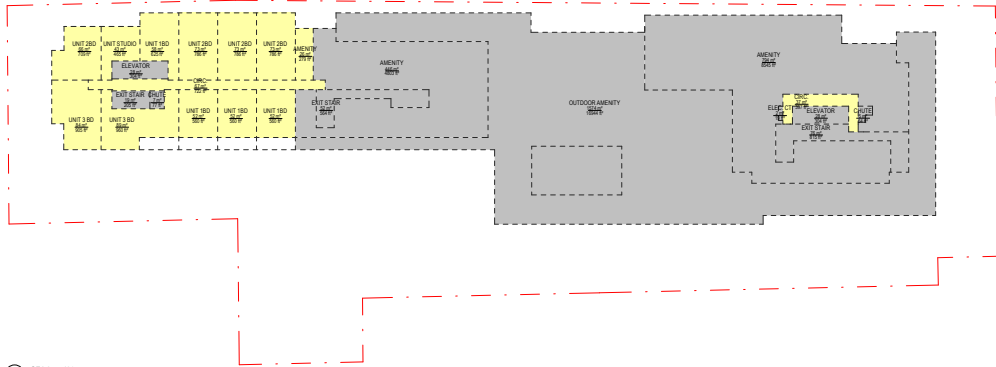


3 GFA Ground Floor
1 : 500

GFA EXCLUSION PER ZBL-569-2013 GROUND FLOOR		
Name	Area	
BIKE RAMP	71.6 m²	
CHUTE	6.0 m²	
CHUTE	5.4 m²	
ELEVATOR	28.2 m²	
ELEVATOR	28.2 m²	
ELEVATOR	12.0 m²	
EXIT STAIR	39.3 m²	
EXIT STAIR	36.2 m²	
EXIT STAIR	30.4 m²	
EXIT STAIR	24.0 m²	
EXIT STAIR	22.5 m²	
LOADING	54.6 m²	
P.O.P.S	353.9 m²	
PARK. RAMP	159.8 m²	
Grand total	872.0 m²	

GFA PER ZBL-569-2013 GROUND FLOOR		
Name	Area	
CACF	5.4 m²	
CACF	4.8 m²	
CIRC.	28.4 m²	
GROCERY	2258.1 m²	
LOADING	89.9 m²	
LOBBY	47.4 m²	
LOBBY	104.0 m²	
LOBBY	66.4 m²	
LOBBY	21.9 m²	
MAIL RM	21.5 m²	
MAIL RM	19.5 m²	
MOVE IN	6.4 m²	
MOVE IN	5.6 m²	
PARCEL RM	23.8 m²	
PARCEL RM	22.1 m²	
RETAIL	249.7 m²	
RETAIL	223.0 m²	
RETAIL	167.1 m²	
SERV. COR.	27.6 m²	
SERV. COR.	22.3 m²	
SERV. COR.	12.3 m²	
VEST.	18.4 m²	
VEST.	12.2 m²	
VEST.	12.1 m²	
Grand total	3469.8 m²	

GROSS CONSTRUCTION AREA (GCA) - GROUND FLOOR		GROSS CONSTRUCTION AREA (GCA) - GROUND FLOOR	
Name	Area	Name	Area
BIKE RAMP	71.6 m ²	LOBBY	66.4 m ²
CACF	5.4 m ²	LOBBY	21.9 m ²
CACF	4.8 m ²	MAIL RM	21.9 m ²
CHUTE	6.0 m ²	MAIL RM	19.9 m ²
CHUTE	5.4 m ²	MOVE IN	6.4 m ²
CIRC.	28.4 m ²	MOVE IN	5.6 m ²
ELEVATOR	28.2 m ²	P.O.P.S	353.9 m ²
ELEVATOR	28.2 m ²	PARCEL RM	23.8 m ²
ELEVATOR	12.0 m ²	PARCEL RM	22.1 m ²
EXIT STAIR	39.3 m ²	PARK RAMP	159.8 m ²
EXIT STAIR	36.2 m ²	RETAIL	17.2 m ²
EXIT STAIR	30.4 m ²	RETAIL	223.0 m ²
EXIT STAIR	24.0 m ²		167.1 m ²
EXIT STAIR	22.5 m ²	SERV. COR.	27.6 m ²
GROCERY	2258.1 m ²	SERV. COR.	22.3 m ²
LOADING	89.9 m ²	SERV. COR.	12.3 m ²
LOADING	54.6 m ²	VEST.	18.4 m ²
LOADING	47.4 m ²	VEST.	12.2 m ²
LOBBY	104.0 m ²	VEST.	12.1 m ²
		Grand total	4341.8 m ²



1 GFA Level 03
1: 500

GFA EXCLUSION PER ZBL-569-2013 LEVEL 03		
Name	Area	
AMENITY	793.8 m²	
AMENITY	446.2 m²	
CHUTE	7.2 m²	
CHUTE	5.0 m²	
ELEC. CT	1.8 m²	
ELEVATOR	28.2 m²	
EXIT STAIR	75.7 m²	
EXIT STAIR	52.4 m²	
EXIT STAIR	19.1 m²	
MECH.	122.6 m²	
OUTDOOR AMENITY	1574.2 m²	
Grand total	3154.4 m²	

GFA PER ZBL-569-2013 LEVEL 03		
Name	Area	
AMENITY	25.9 m²	
CIRC.	67.1 m²	
CIRC.	36.9 m²	
UNIT 1BD	58.0 m²	
UNIT 1BD	52.0 m²	
UNIT 1BD	52.0 m²	
UNIT 1BD	52.0 m²	
UNIT 2BD	73.1 m²	
UNIT 2BD	73.1 m²	
UNIT 2BD	73.1 m²	
UNIT 2BD	65.9 m²	
UNIT 3 BD	84.1 m²	
UNIT STUDIO	43.2 m²	
Grand total	845.6 m²	

GROSS CONSTRUCTION AREA (GCA) - LEVEL 03		GROSS CONSTRUCTION AREA (GCA) - LEVEL 03	
Name	Area	Name	Area
AMENITY	793.8 m²	MECH.	122.6 m²
AMENITY	446.2 m²	OUTDOOR AMENITY	1574.2 m²
AMENITY	25.9 m²	UNIT 1BD	52.0 m²
CHUTE	7.2 m²	UNIT 1BD	52.0 m²
CHUTE	5.0 m²	UNIT 1BD	52.0 m²
CIRC.	67.1 m²	UNIT 1BD	52.0 m²
CIRC.	36.9 m²	UNIT 2BD	73.1 m²
ELEC. CT	1.8 m²	UNIT 2BD	73.1 m²
ELEVATOR	28.2 m²	UNIT 2BD	73.1 m²
EXIT STAIR	75.7 m²	UNIT 3 BD	84.1 m²
EXIT STAIR	52.4 m²	UNIT 3 BD	84.1 m²
EXIT STAIR	19.1 m²	UNIT STUDIO	43.2 m²
		Grand total	3999.9 m²

2 GFA Level 04
1: 500

GFA EXCLUSION PER ZBL-569-2013 LEVEL 04		
Name	Area	
CHUTE	7.2 m²	
CHUTE	5.0 m²	
ELEC. CT	1.8 m²	
ELEVATOR	28.2 m²	
EXIT STAIR	19.7 m²	
EXIT STAIR	19.1 m²	
MECH.	2.9 m²	
MECH.	2.0 m²	
MECH.	2.0 m²	
MECH.	1.9 m²	
MECH.	1.9 m²	
MECH.	1.2 m²	
Grand total	147.3 m²	

GFA PER ZBL-569-2013 LEVEL 04		GFA PER ZBL-569-2013 LEVEL 04	
Name	Area	Name	Area
CIRC.	102.5 m²	UNIT 2BD	72.1 m²
CIRC.	50.2 m²	UNIT 2BD	70.1 m²
UNIT 1BD	62.8 m²	UNIT 2BD	68.6 m²
UNIT 1BD	62.5 m²	UNIT 2BD	68.6 m²
ELEVATOR	59.2 m²	UNIT 2BD	66.1 m²
UNIT 1BD	58.8 m²	UNIT 2BD	59.6 m²
UNIT 1BD	58.6 m²	UNIT 2BD	58.1 m²
UNIT 1BD	56.6 m²	UNIT 2BD	57.8 m²
UNIT 1BD	55.1 m²	UNIT 3 BD	83.3 m²
UNIT 1BD	50.7 m²	UNIT 3 BD	81.7 m²
UNIT 1BD	49.8 m²	UNIT 3 BD	81.7 m²
UNIT 1BD	48.1 m²	UNIT STUDIO	46.1 m²
UNIT 1BD	48.0 m²	UNIT STUDIO	43.2 m²
UNIT 1BD	46.8 m²	UNIT STUDIO	39.8 m²
UNIT 1BD	46.8 m²	UNIT STUDIO	31.9 m²
UNIT 2BD	73.1 m²	UNIT STUDIO	31.9 m²
UNIT 2BD	72.1 m²	UNIT STUDIO	30.9 m²
		UNIT STUDIO	26.6 m²
		Grand total	2098.0 m²

GROSS CONSTRUCTION AREA (GCA) - LEVEL 04		GROSS CONSTRUCTION AREA (GCA) - LEVEL 04	
Name	Area	Name	Area
CHUTE	7.2 m²	UNIT 1BD	48.0 m²
CHUTE	5.0 m²	UNIT 1BD	46.8 m²
CIRC.	102.5 m²	UNIT 1BD	46.8 m²
CIRC.	50.2 m²	UNIT 1BD	46.8 m²
ELEC. CT	1.8 m²	UNIT 2BD	73.1 m²
ELEVATOR	28.2 m²	UNIT 2BD	72.1 m²
ELEVATOR	28.2 m²	UNIT 2BD	72.1 m²
EXIT STAIR	26.3 m²	UNIT 2BD	70.1 m²
EXIT STAIR	19.7 m²	UNIT 2BD	68.6 m²
EXIT STAIR	19.1 m²	UNIT 2BD	68.6 m²
MECH.	2.9 m²	UNIT 2BD	66.1 m²
MECH.	2.0 m²	UNIT 2BD	59.6 m²
MECH.	2.0 m²	UNIT 2BD	58.1 m²
MECH.	1.9 m²	UNIT 2BD	57.8 m²
MECH.	1.9 m²	UNIT 3 BD	83.3 m²
MECH.	1.2 m²	UNIT 3 BD	81.7 m²
UNIT 1BD	62.8 m²	UNIT 3 BD	81.7 m²
UNIT 1BD	62.5 m²	UNIT STUDIO	46.1 m²
UNIT 1BD	59.2 m²	UNIT STUDIO	43.2 m²
UNIT 1BD	58.8 m²	UNIT STUDIO	39.8 m²
UNIT 1BD	58.6 m²	UNIT STUDIO	31.9 m²
UNIT 1BD	56.6 m²	UNIT STUDIO	31.9 m²
UNIT 1BD	55.1 m²	UNIT STUDIO	31.9 m²
UNIT 1BD	50.7 m²	UNIT STUDIO	30.9 m²
UNIT 1BD	49.8 m²	UNIT STUDIO	26.6 m²
UNIT 1BD	48.1 m²	Grand total	2245.3 m²

3 GFA Level 05-08
1: 500

GFA EXCLUSION PER ZBL-569-2013 LEVEL 07		
Name	Area	
CHUTE	7.2 m²	
CHUTE	5.0 m²	
ELEC. CT	1.8 m²	
ELEVATOR	28.2 m²	
EXIT STAIR	26.3 m²	
EXIT STAIR	19.7 m²	
EXIT STAIR	19.1 m²	
MECH.	2.9 m²	
MECH.	2.0 m²	
MECH.	1.9 m²	
Grand total	142.2 m²	

GFA PER ZBL-569-2013 LEVEL 07		GFA PER ZBL-569-2013 LEVEL 07	
Name	Area	Name	Area
CIRC.	102.5 m²	UNIT 2BD	67.6 m²
CIRC.	50.2 m²	UNIT 2BD	63.7 m²
UNIT 1BD	59.2 m²	UNIT 2BD	63.7 m²
UNIT 1BD	58.8 m²	UNIT 2BD	63.7 m²
UNIT 1BD	58.6 m²	UNIT 2BD	63.7 m²
UNIT 1BD	58.6 m²	UNIT 2BD	58.5 m²
UNIT 1BD	56.9 m²	UNIT 3 BD	108.8 m²
UNIT 1BD	55.1 m²	UNIT 3 BD	85.0 m²
UNIT 1BD	50.7 m²	UNIT 3 BD	81.7 m²
UNIT 1BD	49.6 m²	UNIT 3 BD	81.7 m²
UNIT 1BD	48.1 m²	UNIT STUDIO	43.7 m²
UNIT 1BD	46.8 m²	UNIT STUDIO	43.5 m²
UNIT 1BD	46.8 m²	UNIT STUDIO	39.8 m²
UNIT 1BD	46.8 m²	UNIT STUDIO	29.3 m²
UNIT 1BD	46.4 m²	UNIT STUDIO	29.3 m²
UNIT 2BD	70.1 m²	UNIT STUDIO	29.3 m²
UNIT 2BD	68.6 m²	UNIT STUDIO	29.3 m²
UNIT 2BD	68.6 m²	UNIT STUDIO	26.6 m²
		Grand total	2051.5 m²

GROSS CONSTRUCTION AREA (GCA) - LEVEL 07		GROSS CONSTRUCTION AREA (GCA) - LEVEL 07	
Name	Area	Name	Area
CHUTE	7.2 m²	UNIT 1BD	46.8 m²
CHUTE	5.0 m²	UNIT 1BD	46.4 m²
CIRC.	102.5 m²	UNIT 1BD	70.1 m²
CIRC.	50.2 m²	UNIT 2BD	68.6 m²
ELEC. CT	1.8 m²	UNIT 2BD	68.6 m²
ELEVATOR	28.2 m²	UNIT 2BD	67.6 m²
EXIT STAIR	26.3 m²	UNIT 2BD	63.7 m²
EXIT STAIR	19.7 m²	UNIT 2BD	63.7 m²
EXIT STAIR	19.1 m²	UNIT 2BD	63.7 m²
MECH.	2.9 m²	UNIT 2BD	58.5 m²
MECH.	2.0 m²	UNIT 3 BD	108.8 m²
MECH.	1.9 m²	UNIT 3 BD	85.0 m²
UNIT 1BD	59.2 m²	UNIT 3 BD	81.7 m²
UNIT 1BD	58.8 m²	UNIT 3 BD	81.7 m²
UNIT 1BD	58.6 m²	UNIT STUDIO	43.7 m²
UNIT 1BD	58.6 m²	UNIT STUDIO	43.5 m²
UNIT 1BD	56.9 m²	UNIT STUDIO	39.8 m²
UNIT 1BD	55.1 m²	UNIT STUDIO	29.3 m²
UNIT 1BD	50.7 m²	UNIT STUDIO	29.3 m²
UNIT 1BD	49.6 m²	UNIT STUDIO	29.3 m²
UNIT 1BD	48.1 m²	UNIT STUDIO	29.3 m²
UNIT 1BD	46.8 m²	UNIT STUDIO	26.6 m²
UNIT 1BD	46.8 m²	Grand total	2193.6 m²

4 Level 09
1: 500

GFA EXCLUSION PER ZBL-569-2013 LEVEL 08		
Name	Area	
CHUTE	7.2 m²	
CHUTE	5.0 m²	
ELEC. CT	1.8 m²	
ELEVATOR	28.2 m²	
EXIT STAIR	19.7 m²	
EXIT STAIR	19.1 m²	
MECH.	2.9 m²	
MECH.	2.0 m²	
MECH.	1.9 m²	
Grand total	142.2 m²	

GFA PER ZBL-569-2013 LEVEL 09		GFA PER ZBL-569-2013 LEVEL 09	
Name	Area	Name	Area
CIRC.	102.5 m²	UNIT 2BD	67.6 m²
CIRC.	50.2 m²	UNIT 2BD	63.7 m²
UNIT 1BD	59.2 m²	UNIT 2BD	63.7 m²
UNIT 1BD	58.8 m²	UNIT 2BD	63.7 m²
UNIT 1BD	58.6 m²	UNIT 2BD	63.7 m²
UNIT 1BD	56.9 m²	UNIT 2BD	58.5 m²
UNIT 1BD	55.1 m²	UNIT 3 BD	108.8 m²
UNIT 1BD	50.7 m²	UNIT 3 BD	85.0 m²
UNIT 1BD	49.6 m²	UNIT 3 BD	81.7 m²
UNIT 1BD	48.1 m²	UNIT STUDIO	43.7 m²
UNIT 1BD	46.8 m²	UNIT STUDIO	43.5 m²
UNIT 1BD	46.8 m²	UNIT STUDIO	39.8 m²
UNIT 1BD	46.8 m²	UNIT STUDIO	29.3 m²
UNIT 1BD	46.4 m²	UNIT STUDIO	29.3 m²
UNIT 2BD	70.1 m²	UNIT STUDIO	26.6 m²
UNIT 2BD	68.6 m²	UNIT STUDIO	26.6 m²
UNIT 2BD	68.6 m²	Grand total	2051.5 m²

GROSS CONSTRUCTION AREA (GCA) - LEVEL 09		GROSS CONSTRUCTION AREA (GCA) - LEVEL 09	
Name	Area	Name	Area
CHUTE	7.2 m²	UNIT 1BD	46.8 m²
CHUTE	5.0 m²	UNIT 1BD	46.8 m²
CIRC.	102.5 m²	UNIT 1BD	46.8 m²
CIRC.	50.2 m²	UNIT 2BD	70.1 m²
ELEC. CT	1.8 m²	UNIT 2BD	68.6 m²
ELEVATOR	28.2 m²	UNIT 2BD	68.6 m²
ELEVATOR	28.2 m²	UNIT 2BD	67.6 m²
EXIT STAIR	26.3 m²	UNIT 2BD	63.7 m²
EXIT STAIR	19.7 m²	UNIT 2BD	63.7 m²
EXIT STAIR	19.1 m²	UNIT 2BD	63.7 m²
MECH.	2.9 m²	UNIT 2BD	63.7 m²
MECH.	2.0 m²	UNIT 2BD	58.5 m²
MECH.	1.9 m²	UNIT 3 BD	108.8 m²
UNIT 1BD	59.2 m²	UNIT 3 BD	85.0 m²
UNIT 1BD	58.8 m²	UNIT 3 BD	81.7 m²
UNIT 1BD	58.6 m²	UNIT STUDIO	43.7 m²
UNIT 1BD	58.6 m²	UNIT STUDIO	43.5 m²
UNIT 1BD	56.9 m²	UNIT STUDIO	39.8 m²
UNIT 1BD	55.1 m²	UNIT STUDIO	29.3 m²
UNIT 1BD	50.7 m²	UNIT STUDIO	29.3 m²
UNIT 1BD	49.6 m²	UNIT STUDIO	29.3 m²
UNIT 1BD	48.1 m²	UNIT STUDIO	26.6 m²
UNIT 1BD	46.8 m²	UNIT STUDIO	26.6 m²
		Grand total	2193.6 m²

GROSS FLOOR AREA (ZBL 569-2013)

GROSS FLOOR AREA (ZBL 569-2013)

569-2013 EXCLUSION

Gross Floor Area Calculations for an Apartment Building in the Commercial Residential Zone Category per ZBL 569-2013, Section 4.5.4(1) (b)

In the Commercial Residential Zone category the gross floor area of an apartment building is reduced by the area in the building used for:

- (A) parking, loading and bicycle parking below-ground;
- (B) required loading spaces at the ground level and required bicycle parking spaces at or above ground;
- (C) storage and equipment rooms, mechanical, utility, mechanical and ventilation rooms in the basement;
- (D) shower and change facilities required by the By-law for required bicycle parking spaces;
- (E) elevator shafts;
- (F) garbage shafts;
- (G) mechanical penthouses; and
- (H) wet stairwells in the building.

1 OCT 28, 2024 Issued for OPA and ZBA

No. Date Issue/Revision



2451-2495 Danforth
2451-2495 Danforth
Avenue, Toronto, ON
M6R 2J5

Title:
GFA DIAGRAMS PER
ZBL 569-2013

Project No. 2216 Scale As indicated

Drawing No.

A.004

④ GFA Level11-13
1: 500

GFA EXCLUSION PER ZBL-569-2013 LEVEL 11		
Name	Area	
CHUTE	7.2 m²	
CHUTE	5.0 m²	
ELEC. CT	1.8 m²	
ELEVATOR	28.2 m²	
ELEVATOR	28.2 m²	
EXIT STAIR	19.7 m²	
EXIT STAIR	19.7 m²	
EXIT STAIR	19.1 m²	
MECH.	2.9 m²	
MECH.	2.0 m²	
MECH.	1.9 m²	
Grand total	142.2 m²	

GFA PER ZBL-569-2013 LEVEL 11				GFA PER ZBL-569-2013 LEVEL 11			
Name	Area	Name	Area	Name	Area	Name	Area
CIRC.	99.7 m²	UNIT 1BD	46.8 m²	CIRC.	50.2 m²	UNIT 1BD	46.8 m²
UNIT 1BD	59.2 m²	UNIT 1BD	46.5 m²	UNIT 1BD	59.2 m²	UNIT 1BD	46.5 m²
UNIT 1BD	58.8 m²	UNIT 2BD	70.1 m²	UNIT 1BD	58.8 m²	UNIT 2BD	70.1 m²
UNIT 1BD	56.9 m²	UNIT 2BD	68.6 m²	UNIT 1BD	56.9 m²	UNIT 2BD	68.6 m²
UNIT 1BD	56.9 m²	UNIT 2BD	64.8 m²	UNIT 1BD	56.9 m²	UNIT 2BD	64.8 m²
UNIT 1BD	56.9 m²	UNIT 2BD	58.1 m²	UNIT 1BD	56.9 m²	UNIT 2BD	58.1 m²
UNIT 1BD	56.9 m²	UNIT 3 BD	108.4 m²	UNIT 1BD	56.9 m²	UNIT 3 BD	108.4 m²
UNIT 1BD	55.1 m²	UNIT 3 BD	81.7 m²	UNIT 1BD	55.1 m²	UNIT 3 BD	81.7 m²
UNIT 1BD	55.1 m²	UNIT 3 BD	74.8 m²	UNIT 1BD	55.1 m²	UNIT 3 BD	74.8 m²
UNIT 1BD	49.6 m²	UNIT 3 BD	70.5 m²	UNIT 1BD	49.6 m²	UNIT 3 BD	70.5 m²
UNIT 1BD	48.8 m²	UNIT STUDIO	43.9 m²	UNIT 1BD	48.8 m²	UNIT STUDIO	43.9 m²
UNIT 1BD	46.8 m²	UNIT STUDIO	39.8 m²	UNIT 1BD	46.8 m²	UNIT STUDIO	39.8 m²
UNIT 1BD	46.8 m²	UNIT STUDIO	26.6 m²	UNIT 1BD	46.8 m²	UNIT STUDIO	26.6 m²
UNIT 1BD	46.8 m²	UNIT STUDIO	26.6 m²	UNIT 1BD	46.8 m²	UNIT STUDIO	26.6 m²
				Grand total	1841.9 m²		

GROSS CONSTRUCTION AREA (GCA) - LEVEL 11				GROSS CONSTRUCTION AREA (GCA) - LEVEL 11			
Name	Area	Name	Area	Name	Area	Name	Area
CHUTE	7.2 m²	UNIT 1BD	55.1 m²	CHUTE	5.0 m²	UNIT 1BD	55.1 m²
CHUTE	5.0 m²	UNIT 1BD	49.6 m²	CIRC.	99.7 m²	UNIT 1BD	49.6 m²
CIRC.	50.2 m²	UNIT 1BD	46.8 m²	CIRC.	50.2 m²	UNIT 1BD	46.8 m²
ELEC. CT	1.8 m²	UNIT 1BD	46.8 m²	ELEC. CT	1.8 m²	UNIT 1BD	46.8 m²
ELEVATOR	28.2 m²	UNIT 1BD	46.8 m²	ELEVATOR	28.2 m²	UNIT 1BD	46.8 m²
ELEVATOR	28.2 m²	UNIT 1BD	46.8 m²	ELEVATOR	28.2 m²	UNIT 1BD	46.8 m²
EXIT STAIR	19.7 m²	UNIT 1BD	46.8 m²	EXIT STAIR	19.7 m²	UNIT 1BD	46.8 m²
EXIT STAIR	19.1 m²	UNIT 1BD	46.5 m²	EXIT STAIR	19.1 m²	UNIT 1BD	46.5 m²
MECH.	2.9 m²	UNIT 2BD	70.1 m²	MECH.	2.9 m²	UNIT 2BD	70.1 m²
MECH.	2.0 m²	UNIT 2BD	68.6 m²	MECH.	2.0 m²	UNIT 2BD	68.6 m²
MECH.	1.9 m²	UNIT 2BD	68.6 m²	MECH.	1.9 m²	UNIT 2BD	68.6 m²
UNIT 1BD	59.2 m²	UNIT 2BD	64.8 m²	UNIT 1BD	59.2 m²	UNIT 2BD	64.8 m²
UNIT 1BD	58.8 m²	UNIT 2BD	58.1 m²	UNIT 1BD	58.8 m²	UNIT 2BD	58.1 m²
UNIT 1BD	56.9 m²	UNIT 3 BD	108.4 m²	UNIT 1BD	56.9 m²	UNIT 3 BD	108.4 m²
UNIT 1BD	56.9 m²	UNIT 3 BD	81.7 m²	UNIT 1BD	56.9 m²	UNIT 3 BD	81.7 m²
UNIT 1BD	56.9 m²	UNIT 3 BD	74.8 m²	UNIT 1BD	56.9 m²	UNIT 3 BD	74.8 m²
UNIT 1BD	56.9 m²	UNIT 3 BD	70.5 m²	UNIT 1BD	56.9 m²	UNIT 3 BD	70.5 m²
UNIT 1BD	56.9 m²	UNIT STUDIO	43.9 m²	UNIT 1BD	56.9 m²	UNIT STUDIO	43.9 m²
				UNIT STUDIO	39.8 m²		
				UNIT STUDIO	26.6 m²		
				UNIT STUDIO	26.6 m²		
				Grand total	1984.1 m²		

① GFA Level14MPH
1: 500

GFA EXCLUSION PER ZBL-569-2013 LEVEL 14		
Name	Area	
CHUTE	5.0 m²	
ELEC. CT	1.8 m²	
ELEVATOR	28.2 m²	
ELEVATOR	10.3 m²	
EXIT STAIR	26.3 m²	
EXIT STAIR	18.6 m²	
MECH.	271.2 m²	
MECH.	2.9 m²	
MECH.	2.0 m²	
MECH.	1.9 m²	
Grand total	368.1 m²	

GFA PER ZBL-569-2013 LEVEL 14		
Name	Area	
CIRC.	50.2 m²	
CIRC.	8.8 m²	
UNIT 1BD	59.2 m²	
UNIT 1BD	58.8 m²	
UNIT 1BD	55.1 m²	
UNIT 1BD	55.1 m²	
UNIT 1BD	49.6 m²	
UNIT 2BD	70.1 m²	
UNIT 2BD	68.6 m²	
UNIT 2BD	68.6 m²	
UNIT 3 BD	81.7 m²	
UNIT STUDIO	39.8 m²	
UNIT STUDIO	26.6 m²	
UNIT STUDIO	26.6 m²	
Grand total	718.8 m²	

GROSS CONSTRUCTION AREA (GCA) - LEVEL 14		
Name	Area	
CHUTE	5.0 m²	
CIRC.	50.2 m²	
CIRC.	8.8 m²	
ELEC. CT	1.8 m²	
ELEVATOR	28.2 m²	
ELEVATOR	10.3 m²	
EXIT STAIR	26.3 m²	
EXIT STAIR	18.6 m²	
MECH.	271.2 m²	
MECH.	2.9 m²	
MECH.	2.0 m²	
MECH.	1.9 m²	
UNIT 1BD	59.2 m²	
UNIT 1BD	58.8 m²	
UNIT 1BD	55.1 m²	
UNIT 1BD	55.1 m²	
UNIT 1BD	49.6 m²	
UNIT 2BD	70.1 m²	
UNIT 2BD	68.6 m²	
UNIT 2BD	68.6 m²	
UNIT 3 BD	81.7 m²	
UNIT STUDIO	39.8 m²	
UNIT STUDIO	26.6 m²	
UNIT STUDIO	26.6 m²	
Grand total	1086.9 m²	

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GROSS FLOOR AREA (ZBL 569-2013)

	GROSS FLOOR AREA (ZBL 569-2013)
	569-2013 EXCLUSION

Gross Floor Area Calculations for an Apartment Building in the Commercial Residential Zone Category per ZBL 569-2013, Section 42.5.40.41 (5).

In the Commercial Residential Zone category the gross floor area of an apartment building is reduced by the area in the building used for:

- (A) parking, loading and bicycle parking below-ground;
(B) required loading spaces at the ground level and required bicycle parking spaces at or above ground;
(C) storage room for equipment, mechanical, utility, mechanical and ventilation rooms in the basement;
(D) shower and change facilities required by the By-law for required bicycle parking spaces;
(E) elevator shafts;
(F) elevator shafts;
(G) garbage shafts;
(H) mechanical penthouses; and
(I) wet stairwells in the building.

1 OCT 28, 2024 Issued for OPA and ZBA
No. Date Issue/Revision



2451-2495 Danforth
2451-2495 Danforth
Avenue, Toronto, ON
M6R 2J5

Title:
GFA DIAGRAMS PER
ZBL 569-2013

Project No. 2216 Scale As indicated

Drawing No.

A.005

<u>GREEN ROOF STATISTICS</u>		Proposed
Gross Floor Area, as defined in Green Roof Bylaw (m ²)		46879.5 m ²
Total Roof Area (m²)		4231.57 m²
Area of Residential Private Terraces (m ²)		390.04 m ²
Rooftop Outdoor Amenity Space, if in a Residential Building (m ²)		1376.17 m ²
Area of Renewable Energy Devices (m ²)		0 m ²
Tower (s)/Roof Area with floor plate less than 750 m ²		N/A
Total Available Roof Space (m²)		2465.36 m²
Green Roof Coverage		Required Proposed
Coverage of Available Roof Space (m ²)	1479.22 m ²	1990 m ²
Coverage of Available Roof Space (%)	60 %	80.7 %

Gross Floor Area, as defined in Green Roof Bylaw

The total area of each floor level of a building, above and below average grade, measured from the exterior of the main wall of each floor level, including voids at the level of each floor, such as an atrium, mezzanine, stairwell, escalator, elevator, ventilation duct or utility shaft, but *excluding areas used for the purpose of parking or loading*

Definitions

FLOOR PLATE AREA - The total area of a floor of a building, measured from the exterior of the main wall of the floor level, including voids at the level of the floor, such as an atrium, mezzanine, stairwell, escalator, elevator, ventilation duct or utility shaft.

GROSS FLOOR AREA - The total area of each floor level of a building, above and below average grade, measured from the exterior of the main wall of each floor level, including voids at the level of each floor, such as an atrium, mezzanine, stairwell, escalator, elevator, ventilation duct or utility shaft, but excluding areas used for the purpose of parking or loading

LOADING NOTES

1. TYPE I LOADING SPACE AND ADJACENT STAGING PAD HAVE VERTICAL CLEARANCE OF 8 IN. MIN.
2. TYPE II LOADING SPACE SHALL BE SHARED BETWEEN RESIDENTIAL AND NON-RESIDENTIAL USES. NON-RESIDENTIAL COMPONENT WILL ONLY SCHEDULE USE OF THE TYPE I LOADING SPACE ON DIFFERENT DAYS.
3. NON-RESIDENTIAL WASTE WILL BE LABELED AND STORED SEPARATELY FROM THE BINIS FOR RESIDENTIAL USE.
4. TYPE II LOADING SPACE SHALL BE LEVEL, 20' X 30' MIN. WITH 10' MIN. CLEARANCE FOR CITY WASTE COLLECTION.
5. A WARNING SYSTEM WILL BE PROVIDED, ALERTING PERSONNEL WHEN EXTING THE UNDERGROUND PARKING GARAGE THAT LARGE TRUCKS ARE MANEUVERING WITHIN THE PUBLIC LAKE.
6. TYPE II LOADING SPACE SHALL BE USED BY THE LOADING COLLECTION VEHICLE ONLY.
7. MINIMUM VERTICAL CLEARANCE OF 4 METERS THROUGHOUT.
8. TYPE II LOADING SPACE SHALL BE 4 METERS WIDE.
9. 4 METERS WIDE AT POINT OF INGRESS AND EGRESS.
10. TYPE II LOADING SPACE SHALL BE PROVIDED AND MAINTAINED ADJACENT TO THE LOADING SPACE.
11. CONSTRUCT A NEW TYPE I LOADING SPACE AND ALL DRIVEWAYS AND PASSAGEWAYS PROVIDING ACCESS THERE TO, TO THE REQUIREMENTS OF THE ONTARIO BUILDING CODE, INCLUDING ALLOWANCE FOR CITY OF TORONTO PARKING SPOTS TO BE PROVIDED AND MAINTAINED ADJACENT TO THE LOADING SPACE.
12. THE RESIDENTIAL SOLID WASTE ROOM WILL ACCOMMODATE GARAGE, RECYCLING AND PROVIDING ACCESS FOR THE RESIDENTIAL COMPONENT OF THE BUILDING VIA OF A BSIORNER IN THE DEVELOPMENT.
13. COLLECTION OF WASTE MATERIALS FOR THIS DEVELOPMENT WILL TAKE PLACE IN AN ENCLOSED LOADING PAD. AN ON-SITE STAFF PERSON IS RESPONSIBLE FOR MOVING THE WASTE MATERIALS TO THE COLLECTION POINTS. THE WASTE COLLECTION VEHICLE WILL BE DIRECTED TO THE WASTE COLLECTION POINTS BY THE ON-SITE STAFF PERSON.
14. THIS BUILDING IS DESIGNED WITH A TYPE I LOADING SPACE, A FLAGGING WARNING LIGHT SYSTEM AND/OR APPROPRIATE SIGNAGE ADJACENT TO THE SPACE, AT NO COST TO THE CITY WILL BE IN PLACE AND MAINTAINED.
15. SOLID WASTE MANAGEMENT TO BE NOTIFIED UPON COMPLETION OF THE DEVELOPMENT AND SHOULD PUBLIC WASTE COLLECTION BE USED. ALL NECESSARY APPLICATION AND WAIVER FORMS TO BE SUBMITTED TO THE CITY OF TORONTO.
16. NON-RESIDENTIAL GARBAGE WILL BE COLLECTED BY LICENSED PRIVATE WASTE MANAGEMENT COMPANY.
17. THE CITY OF TORONTO WILL BE AVAILABLE TO ADVISE THE DEVELOPER OF THE COLLECTION VEHICLE AND ALSO ACT AS A FLAGMAN WHEN THE TRUCK IS REVERSING. IN THE EVENT THE ON-SITE STAFF IS UNAVAILABLE AT THE TIME THE CITY COLLECTION VEHICLE ARRIVES AT THE SITE, THE COLLECTION VEHICLE WILL LEAVE THE SITE AND NOT RETURN UNTIL THE NEXT SCHEDULED COLLECTION DATE. FOR SPECIFIC

SITE PLAN NOTES

1. THE BUILDING IS TO BE SPRINKLED.
2. THE BUILDING SHALL BE INDIVIDUALLY SIGNED AT THE FRONT OF EACH SPACE FOR THE USE OF RESIDENTIAL VISITORS. BUILDING MANAGEMENT SHALL PROVIDE ENFORCEMENT OF THIS ARRANGEMENT.
3. THE BUILDING SHALL BE SIGNED TO INDICATE THE LOCATION OF THE BUILDING AND MAXIMUM AS SLOPE TOWARDS THE ROADWAY
4. REFUSE TO SITE SERVING OCCUPANTS FOR SEWER AND WATER SERVICE INFORMATION.
5. ANY REMAINING WALLS ARE TO BE PROFESSIONALLY ENGINEERED.
6. ANY EXISTING OR NEW TRAFFIC COUNCILS, ETC. ALONG THE DEVELOPMENT SITE FRONTS THAT ARE NO LONGER REQUIRED ARE TO BE REMOVED. THE BOLIVARD WITHIN THE PUBLIC RIGHT OF WAY, IN ACCORDANCE WITH CITY STANDARDS
7. PROPOSED ACCESS TO THE RIGHT-OF-WAY/LANES FOR THIS PROJECT TO BE DESIGNED IN ACCORDANCE WITH CITY STANDARD
8. NO SPEED BUMPS SHALL BE INSTALLED ON ANY DESIGNATED FREE FLOW
9. TRUCK EXITS/TO GREASE TRAP, VENTILATION SHALL BE 20 CM X 20 CM
10. ALL EXTERIOR LIGHT FIXTURES TO BE DARK SKY COMPLIANT.

UTILITY/SERVICES NOTES:
1. THE METHOD OF INSTA

1. THE METHOD OF INSTALLATION FOR THE PROPOSED SERVICE CONNECTIONS WILL BE AT THE DISCRETION OF TORONTO WATER.
2. THE LOCATION OF THE SERVICE CONNECTION SHALL BE DETERMINED BY TORONTO WATER AT THE OWNER'S COST.
3. THE LOCATION OF THE WATER METER SHALL BE TO TORONTO WATER'S SATISFACTION.
4. THE OWNER IS REQUIRED TO INSTALL AND MAINTAIN A PREMISE ISOLATION DEVICE FOR ALL APPLICABLE WATER SERVICES IN CONFORMANCE WITH THE TORONTO MUNICIPAL CODE, CHAPTER B61 WATER SUPPLY, THE BUILDING CODE AND CSA B61.10 STANDARDS.
5. THE BUILDING'S STORE AND SANITARY SYSTEM MUST BE DESIGNED TO BE ABLE TO OPERATE UNDER MUNICIPAL BURCHARGE CONDITIONS.
6. BE ADVISED THAT SHOULD ANY PARTY, INCLUDING THE APPLICANT OR ANY SUBSEQUENT OWNER, APPLY FOR MORE THAN ONE CONDOMINIUM CORPORATION ENCOMPASSING ANY OR ALL OF THIS DEVELOPMENT OR MAKE AN APPLICATION THAT RESULTS IN A LAND DIVISION OF ANY PART OF THIS DEVELOPMENT, THE APPLICANT SHALL BE RESPONSIBLE FOR THE COSTS OF THE APPROVED SERVICES. SUCH ASSURANCES WILL BE DETERMINED AT THE TIME OF APPLICATION FOR CONDOMINIUM APPROVAL.

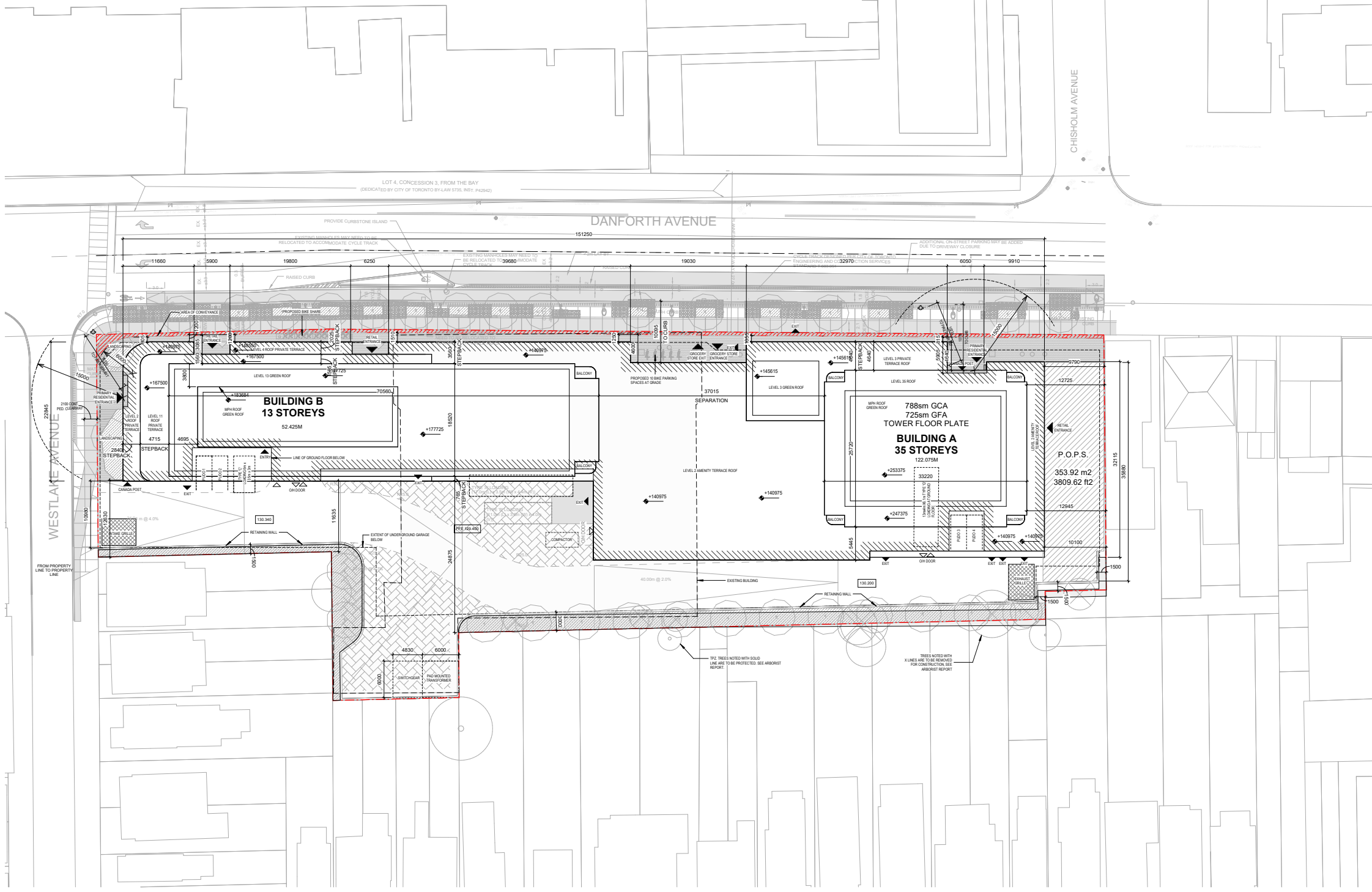
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- | | |
|--|--|
| | PRINCIPAL ENTRY |
| | ENTRY/EXIT |
| | VEHICULAR ENTRY/EXIT |
| | FIRE DEPARTMENT CONNECTION |
| | FIRE HYDRANT |
| | MANHOLE COVER |
| | CATCH BASIN |
| | HYDRO POLE |
| | ELECTRICAL STAND |
| | EXTENT OF BELOW GRADE |
| | BUILDING ELEMENT ABOVE |
| | OPEN TO BELOW |
| | EXTENT OF GROUND FLOOR |
| | 123.45
GEODETIC ELEVATION |
| | HT: 123.45
ELEVATION FROM ESTABLISHED GRADE |
| | 78.20
EXISTING GRADE ELEVATION FH |
| | BARRIER FREE TURNING RADIUS |
| | EXISTING BUILDING |
| | PROPERTY CONVEYANCE |
| | PROPERTY LINE |
| | FFE
FINISHED FLOOR ELEVATION |
| | TOP
TOP OF PARAPET |
| | TOR
TOP OF ROOF |
| | TOS
TOP OF STRUCTURE |
| | TGS
TORONTO GREEN STANDARDS |
| | TPZ
TREE PROTECTION ZONE |

NOTE:
SURVEY INFORMATION TAKEN FROM "LOT 1 AND PART OF LOT 2 REGISTERED PLAN 614 YORK AND PART OF LOT 13 SOUTH SIDE OF DANFORTH AVENUE REGISTERED PLAN 90 YORK AND PART OF LOTS 3,4,5,6,7 AND 8 REGISTERED PLAN 580 YORK CITY OF TORONTO"
BY KRCMAR SURVEYORS LTD.
DATED JULY, 27 2022.

OCT 28, 2024	Issued for OPA and ZBA	
No.	Date	Issue/Revision



2451-2495 Danforth
2451-2495 Danforth
Avenue, Toronto, ON
M6R 2J5

Title: **SITE PLAN**

Project No. 2216	Scale	As indicated
Drawing No.		

A.100

SETBACK DIAGRAM LEGEND

- SETBACK FROM PROPERTY LINE
- SEPARATION DISTANCE BETWEEN UPPER FLOORS AND EXISTING BUILDINGS
- SEPARATION DISTANCE BETWEEN PODIUM AND EXISTING BUILDINGS

1 OCT 28, 2024 Issued for OPA and ZBA
No. Date Issue/Revision

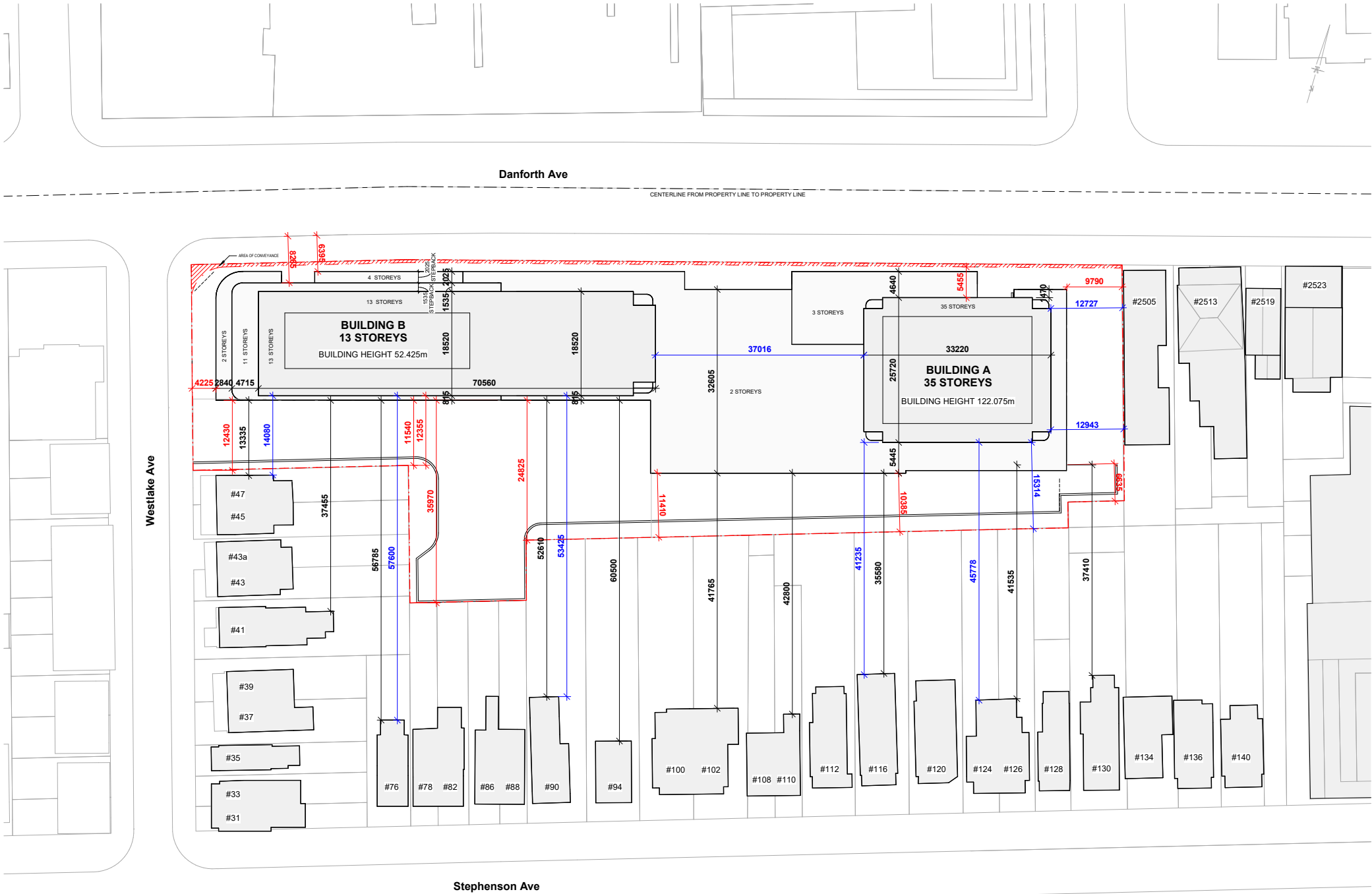


2451-2495 Danforth
2451-2495 Danforth
Avenue, Toronto, ON
M6R 2J5

Title:
SETBACK DIAGRAM

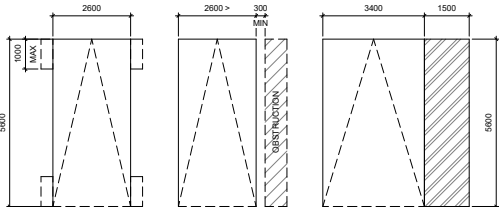
Project No. 2216 Scale As indicated
Drawing No.

A.006



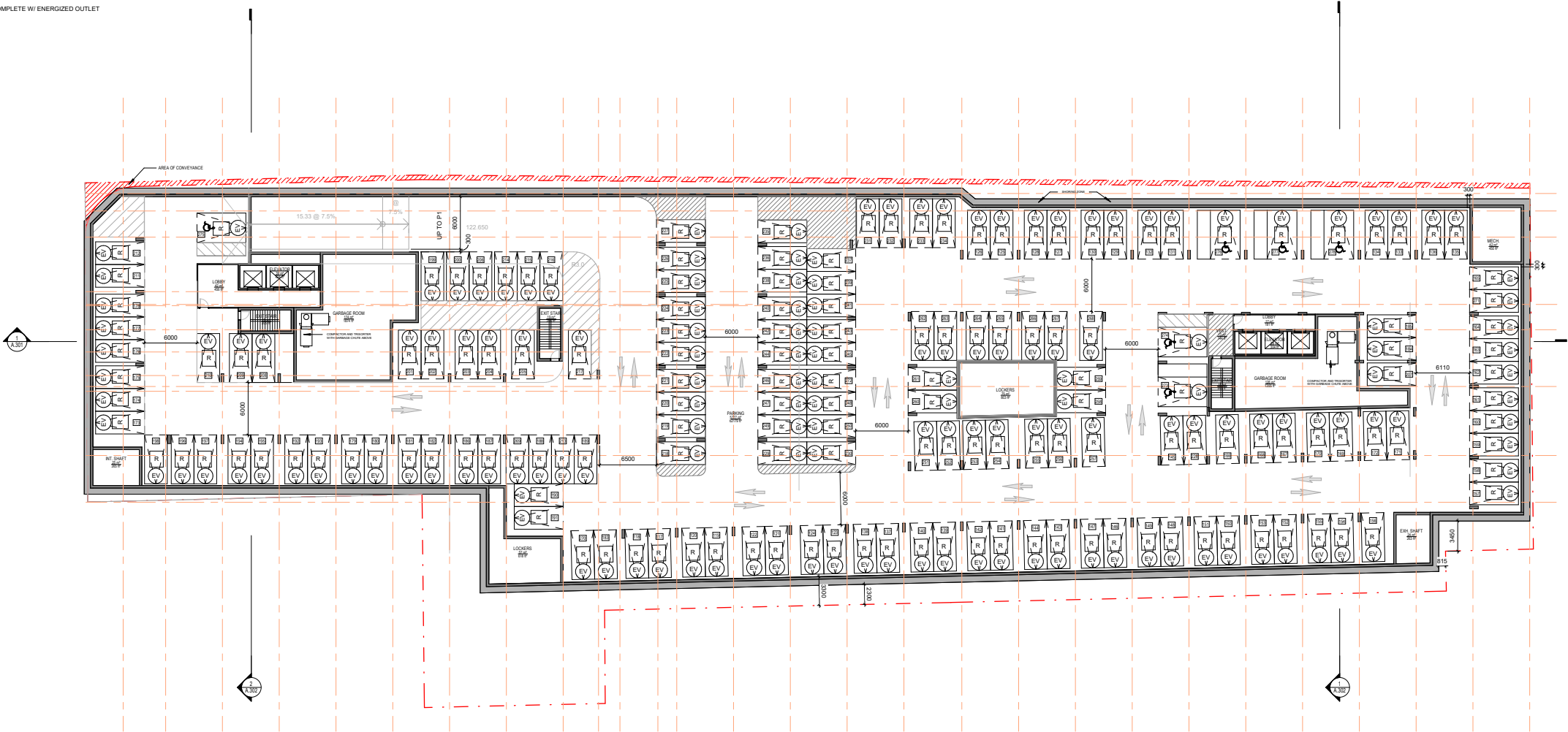
1 Setback Diagram
1:350

VEHICLE PARKING DIMENSIONS



TYPICAL PARKING SPACE
TYPICAL OBSTRUCTED PARKING SPACE
TYPICAL BARRIER FREE PARKING SPACE PER 569-2017

EV - PARKING SPACE COMPLETE W/ ENERGIZED OUTLET
V - VISITOR
R - RESIDENTIAL
EB - BIKE PARKING SPACE COMPLETE W/ ENERGIZED OUTLET



1 P2
1:250

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No. Date Issue/Revision



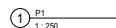
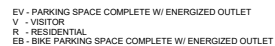
2451-2495 Danforth
2451-2495 Danforth
Avenue, Toronto, ON
M6R 2J5

Title:
FLOOR PLAN - P2

Project No. 2216 Scale As indicated
Drawing No.

A.101

BICYCLE SPACE LEGEND



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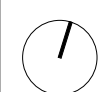
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No.	Date	Issue/Revision
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2451-2495 Danforth
2451-2495 Danforth
Avenue, Toronto, ON
M6R 2J5

Title:
FLOOR PLAN - P1

Project No.	2216	Scale	As indicated
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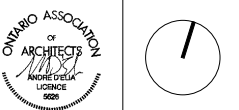
Project No. 2210	Scale	As indicated
Drawing No.		

A.102

- PRINCIPAL ENTRY
- ENTRY/ EXIT
- VEHICULAR ENTRY/ EXIT
- FIRE DEPARTMENT CONNECTION
- FIRE HYDRANT
- MANHOLE COVER
- CATCH BASIN
- HYDRO POLE
- ELECTRICAL STAND
- EXTENT OF BELOW GRADE
- BUILDING ELEMENT ABOVE
- OPEN TO BELOW
- EXTENT OF GROUND FLOOR
- GEODETIC ELEVATION
- ELEVATION FROM ESTABLISHED GRADE
- EXISTING GRADE ELEVATION
- BARRIER FREE TURNING RADIUS
- EXISTING BUILDING
- PROPERTY CONVEYANCE
- PROPERTY LINE
- FFE FINISHED FLOOR ELEVATION
- TOP TOP OF PARAPET
- TOR TOP OF ROOF
- TOS TOP OF STRUCTURE
- TGS TORONTO GREEN STANDARDS
- TPZ TREE PROTECTION ZONE

NOTE:
SURVEY INFORMATION TAKEN FROM LOT 1 AND PART OF LOT 2 REGISTERED PLAN 614 YORK AND PART OF LOT 13 SOUTH SIDE OF DANFORTH AVENUE REGISTERED PLAN 90 YORK AND PART OF LOTS 3,4,5,6,7 AND 8 REGISTERED PLAN 580 YORK CITY OF TORONTO BY KICMAR SURVEYORS LTD. DATED JULY, 27 2022.

1 OCT 28, 2024 Issued for OPA and ZBA
No. Date Issue/Revision

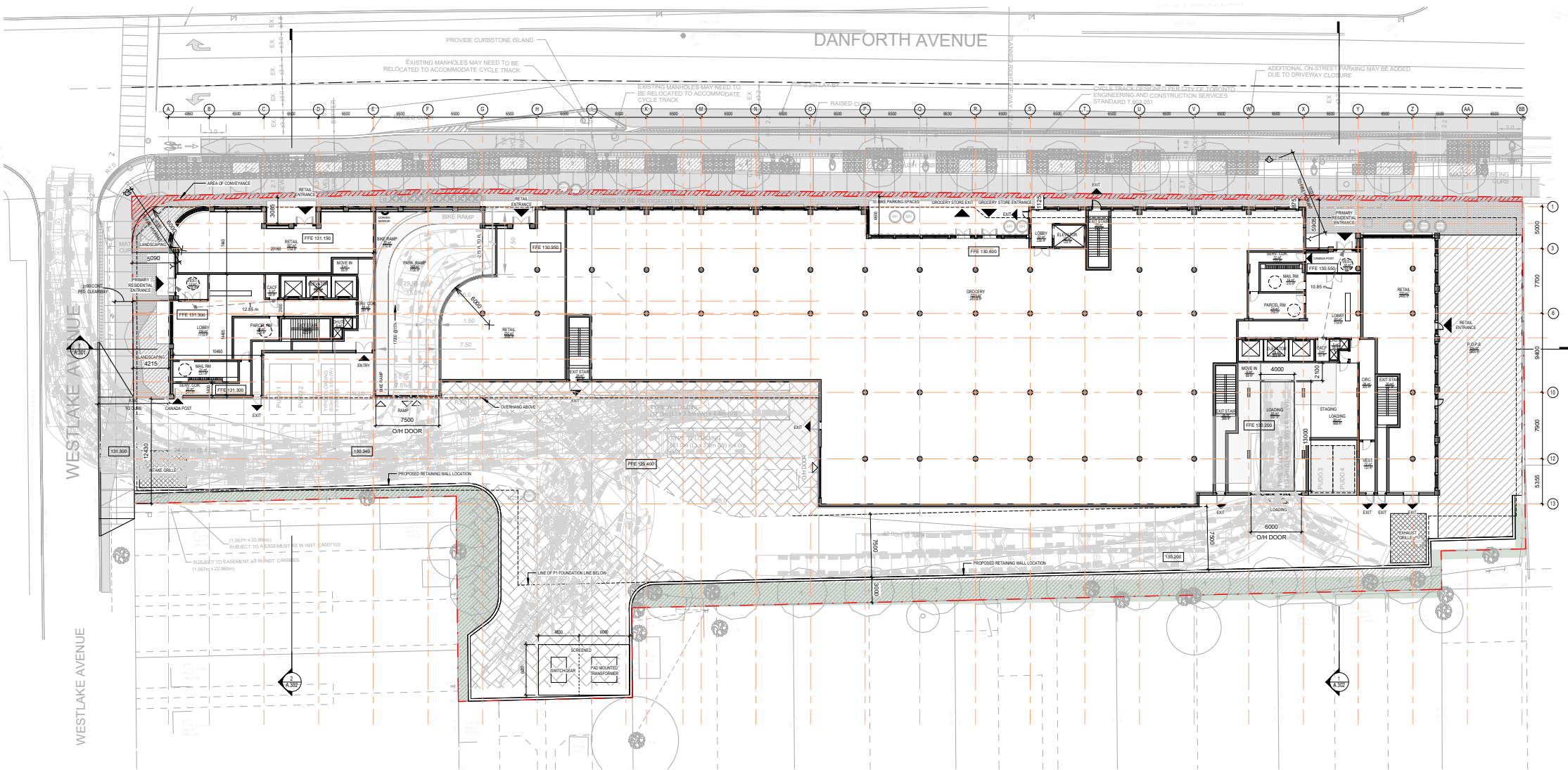


2451-2495 Danforth
2451-2495 Danforth
Avenue, Toronto, ON
M6R 2J5

Title:
FLOOR PLAN - GROUND FLOOR

Project No. 2216 Scale As indicated
Drawing No.

A.103

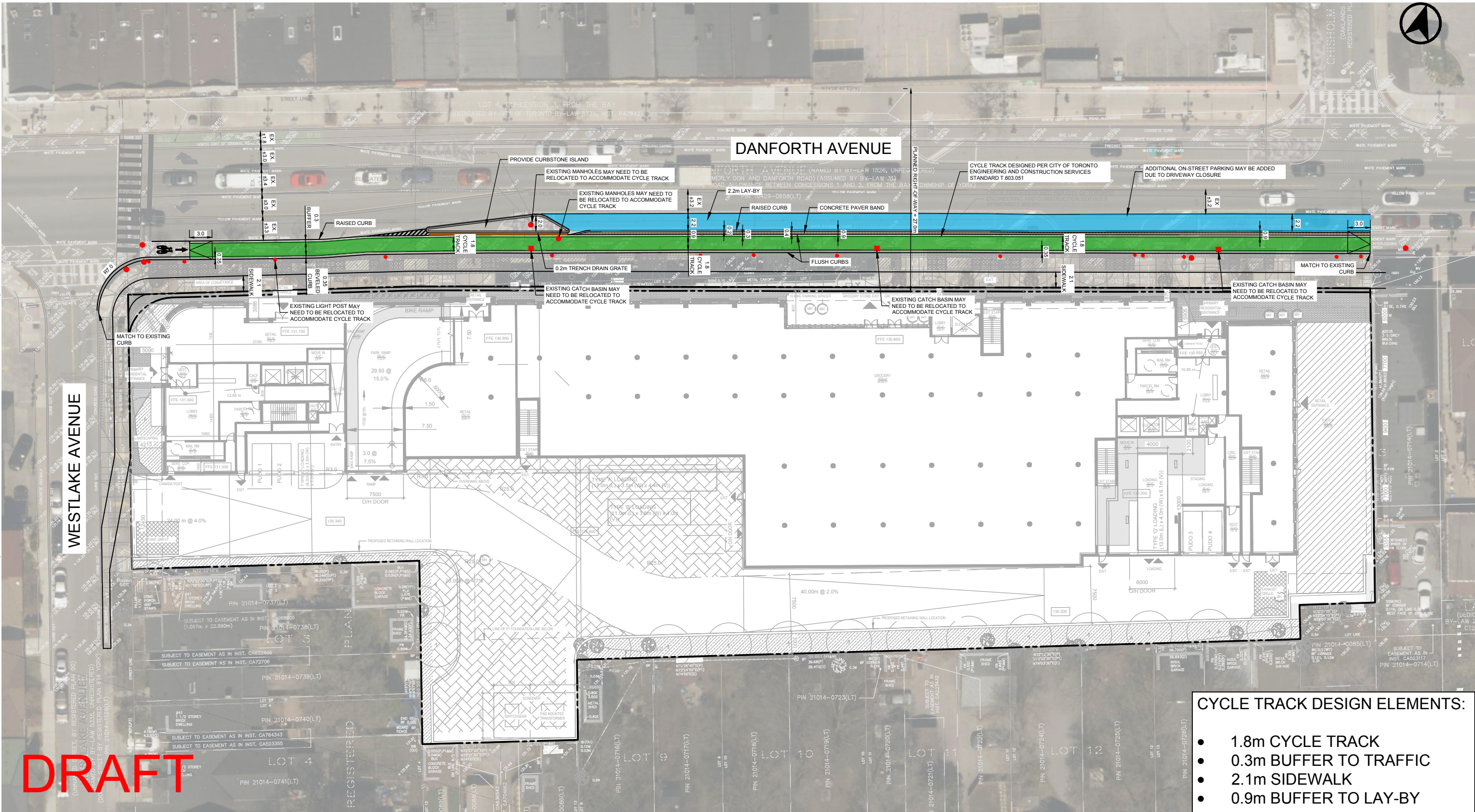


1 Site Plan - Ground Floor
1:250

Appendix B:
Functional Design Plans – Danforth Avenue Cycle Track

Date Plotted: November 8, 2024 Filename: J:\7036-35\BA\Functional Design\2024\7. November 07, 2024\ba-24455 Danforth Ave-FD-R05-7036-35.dwg

DRAFT



- CYCLE TRACK DESIGN ELEMENTS:**
- 1.8m CYCLE TRACK
 - 0.3m BUFFER TO TRAFFIC
 - 2.1m SIDEWALK
 - 0.9m BUFFER TO LAY-BY



2451 - 2495 DANFORTH AVENUE
FUNCTIONAL ROAD PLAN
INTERMEDIATE HEIGHT RAISED CYCLE TRACK WITH BEVELED SERRATION CURB
(T-603.051)

Project: 2451-2495 DANFORTH
Project No. 7036-35
Date: November 8, 2024
Revised:

Scale: 0 2 4 6 8 10 20m
1:400

Drawing No. **FD-01**

Appendix C:

Vehicle Manoeuvring Diagrams

Date Plotted: November 8, 2024 Filename: J:\7036-35\BA\Site Plan Review\2024\11_Nov 06_2024\ba-2455 Danforth Ave-SPR-06-7036-35.dwg

Design Vehicle - MTO WB-17.5, URBAN VEHICLE

Overall Length	19.50m
Overall Width	2.60m
Overall Body Height	4.10m
Inside Turning Radius	Varies
Outside Turning Radius	14.40m

2451 - 2495 DANFORTH AVENUE

VEHICLE MANOEUVRING DIAGRAM

MTO WB-17.5 - INBOUND

TYPE 'A' LOADING SPACE

Project: 2451-2495 DANFORTH

Project No. 7036-35

Date: April 09, 2024

Revised: November 8, 2024

Scale

0 2 4 6 8 10 20m

1:400

Drawing No.

VMD-01

Date Plotted: November 8, 2024 Filename: J:\7036-35\BA\Site Plan Review\2024\11. Nov 06, 2024\ba-2455 Danforth Ave-SPR-06-7036-35.dwg

Design Vehicle - MTO WB-17.5, URBAN VEHICLE

Overall Length

Overall Width

Overall Body Height

Inside Turning Radius

Outside Turning Radius

19.50m

2.60m

4.10m

Varies

14.40m

2451 - 2495 DANFORTH AVENUE

VEHICLE MANOEUVRING DIAGRAM

MTOWB-17.5 - OUTBOUND

TYPE 'A' LOADING SPACE

Project: 2451-2495 DANFORTH

Project No. 7036-35

Date: April 09, 2024

Revised: November 8, 2024

Scale

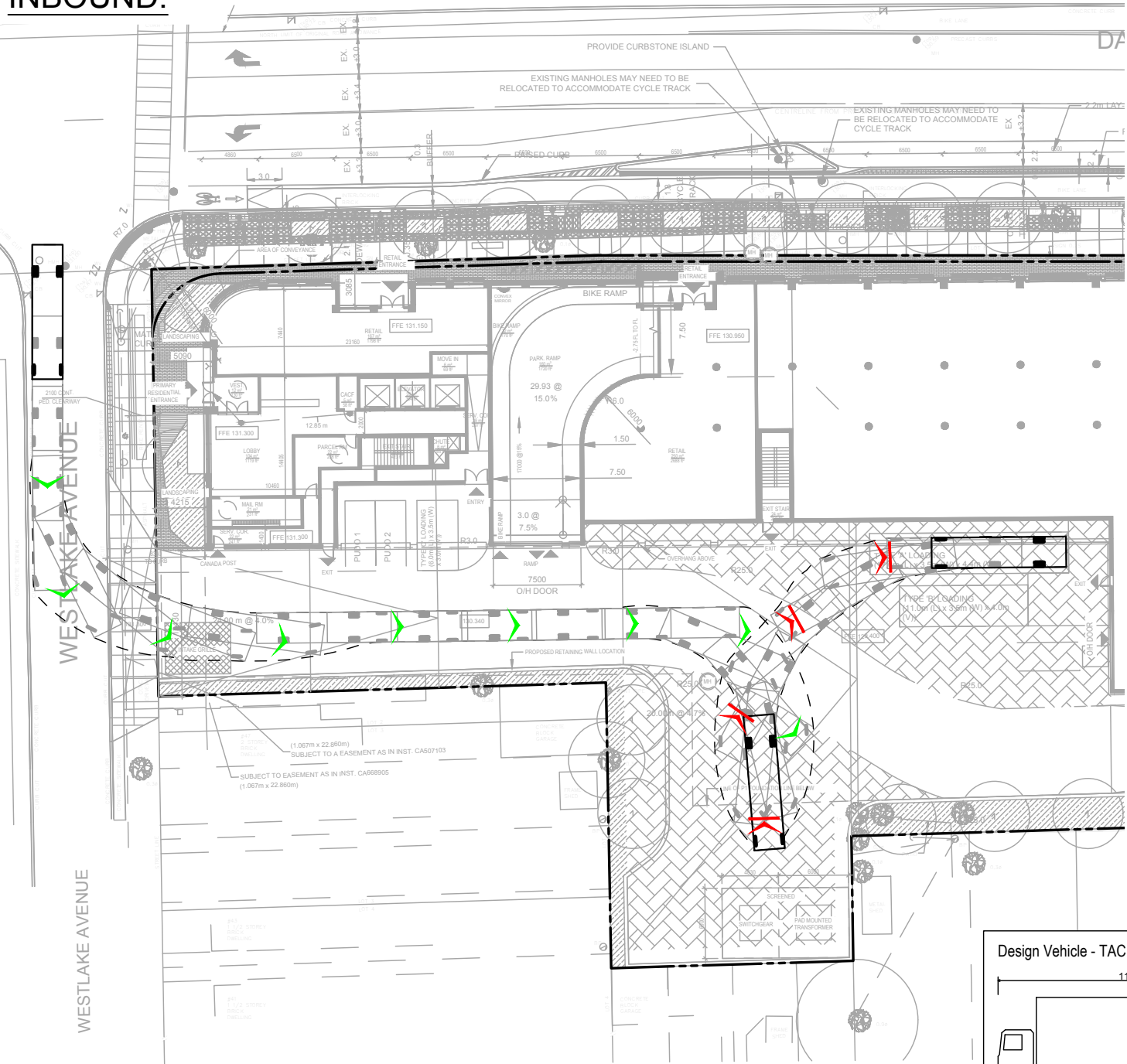
0 2 4 6 8 10 20m

1:400

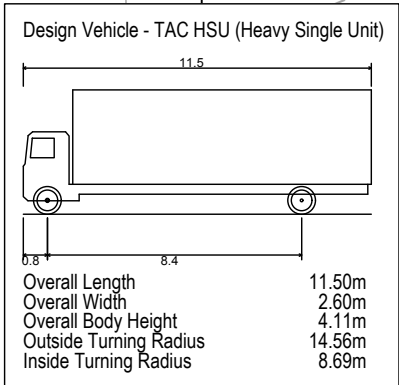
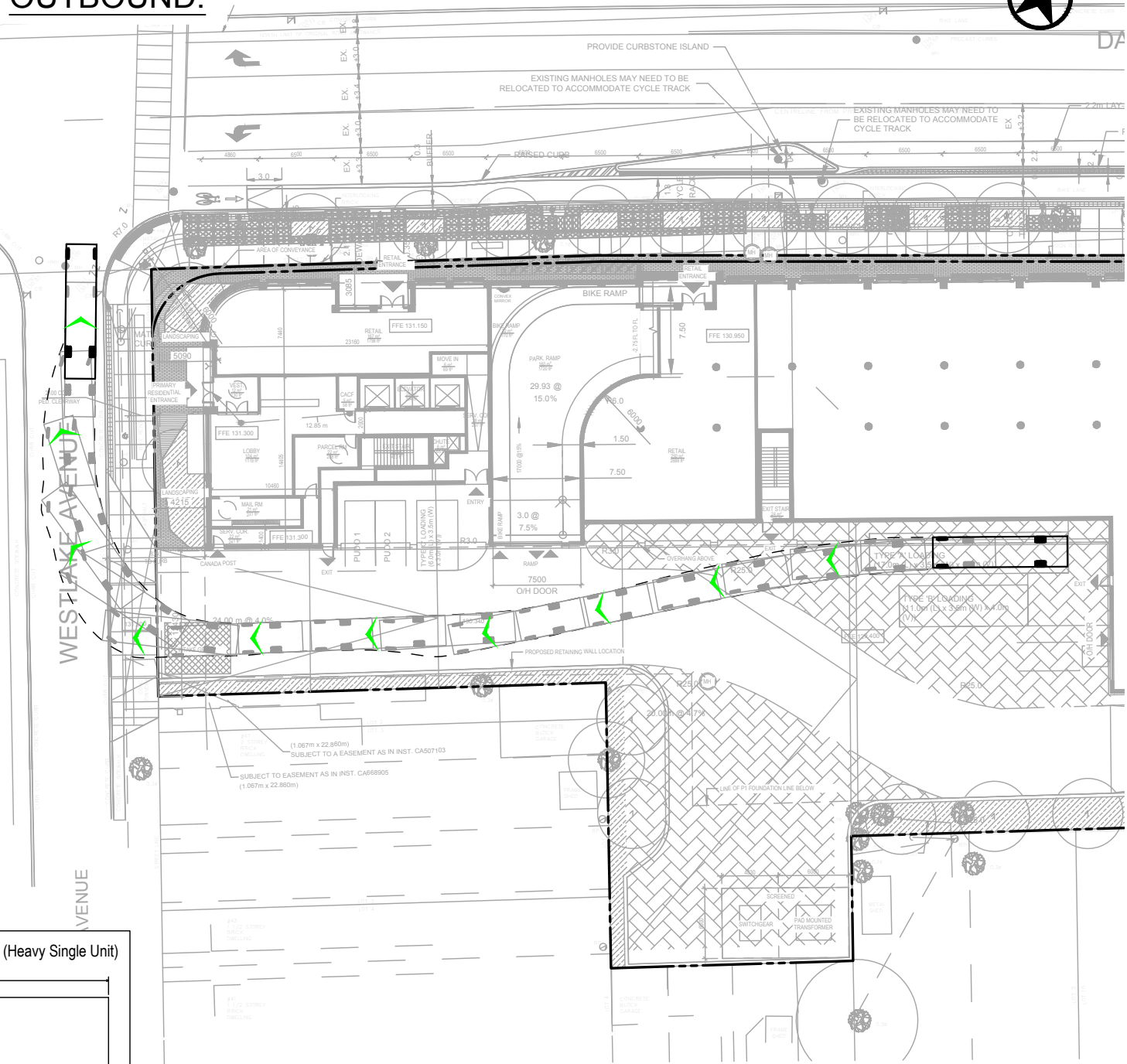
Drawing No.

VMD-02

INBOUND:



OUTBOUND:



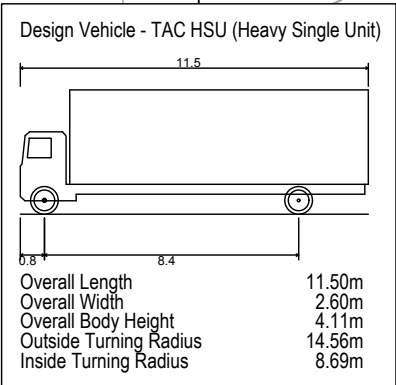
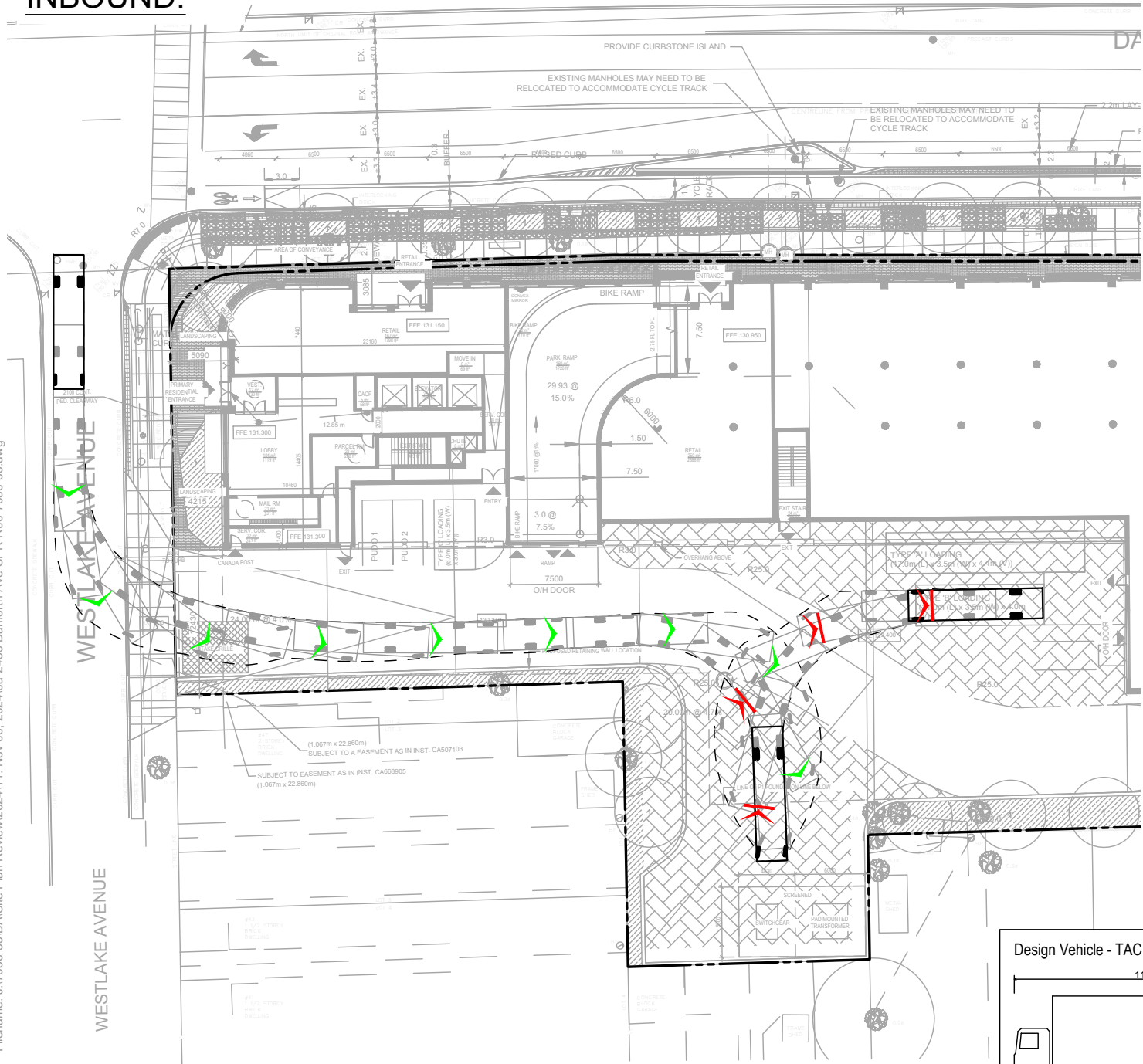
2451 - 2495 DANFORTH AVENUE
VEHICLE MANOEUVRING DIAGRAM
HEAVY SINGLE UNIT (HSU) VEHICLE
TYPE 'A' LOADING SPACE

Project: 2451-2495 DANFORTH
Project No. 7036-35
Date: April 09, 2024
Revised: November 8, 2024

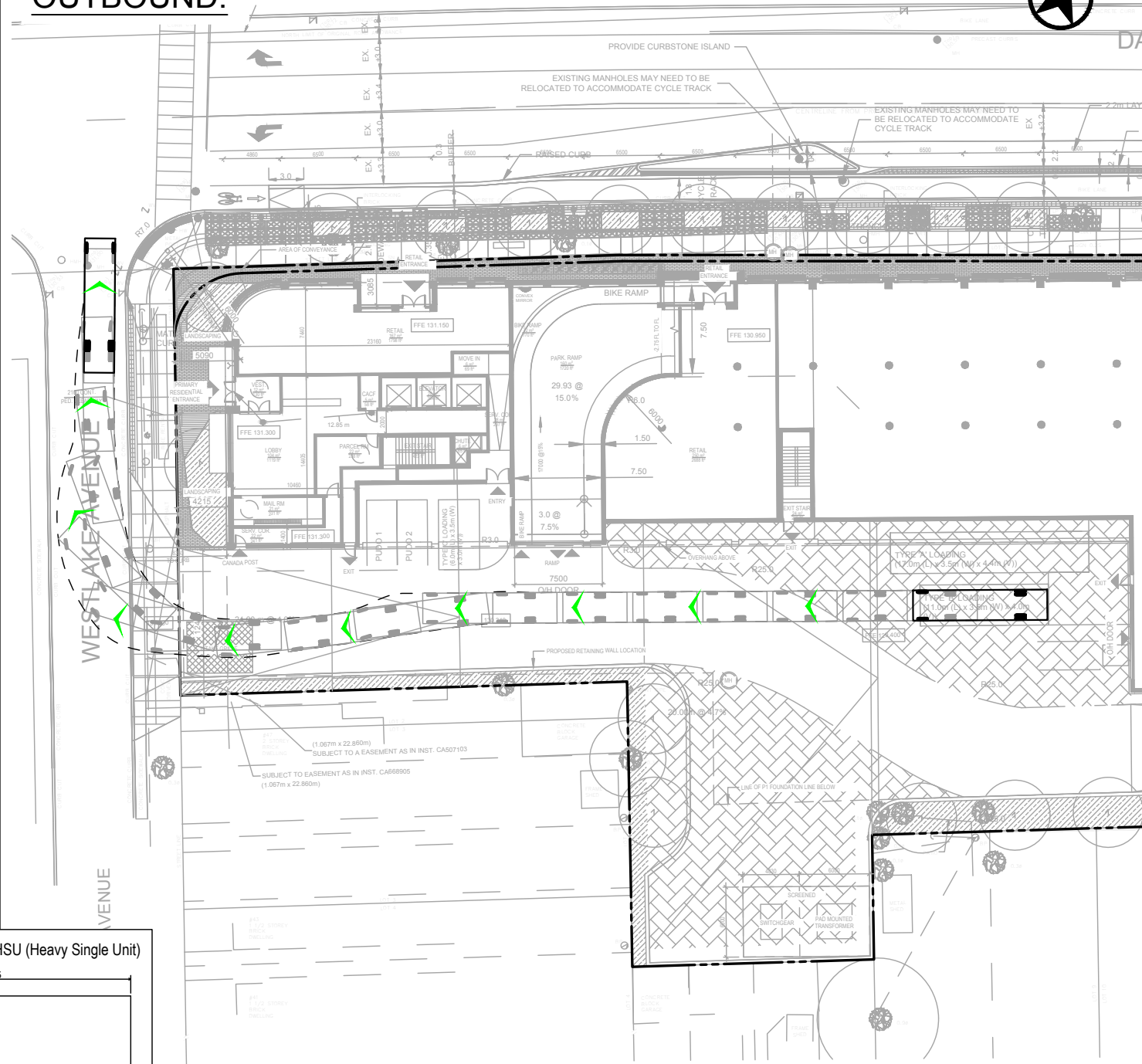
Scale 0 2 4 6 8 10 20m
1:400
Drawing No. VMD-03

Filename: J:\7036-35\BA\Site Plan Review\2024\11_Nov_06_2024\ba-2455 Danforth Ave-SPR-06-7036-35.dwg
Date Plotted: November 8, 2024

INBOUND:



OUTBOUND:



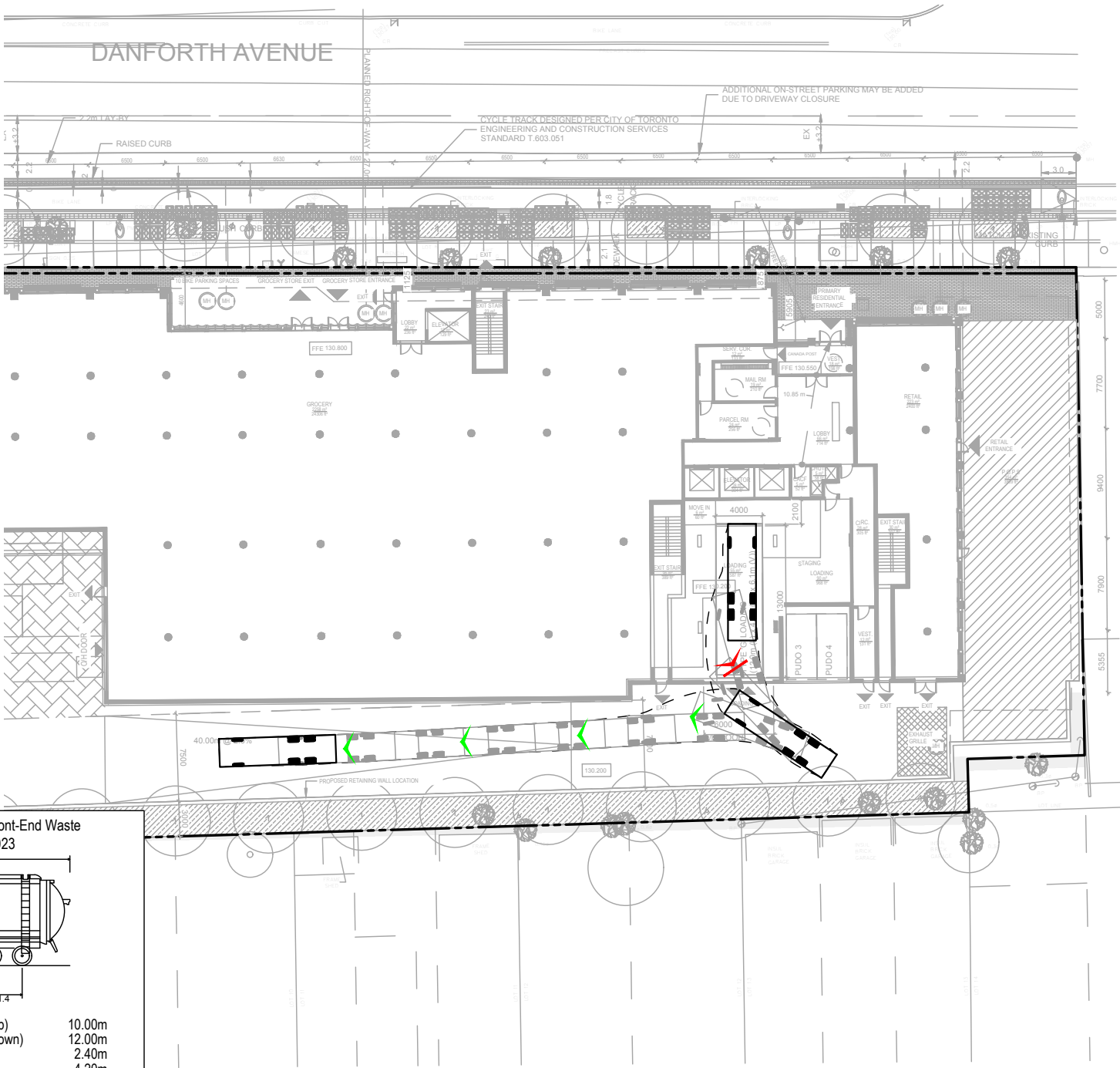
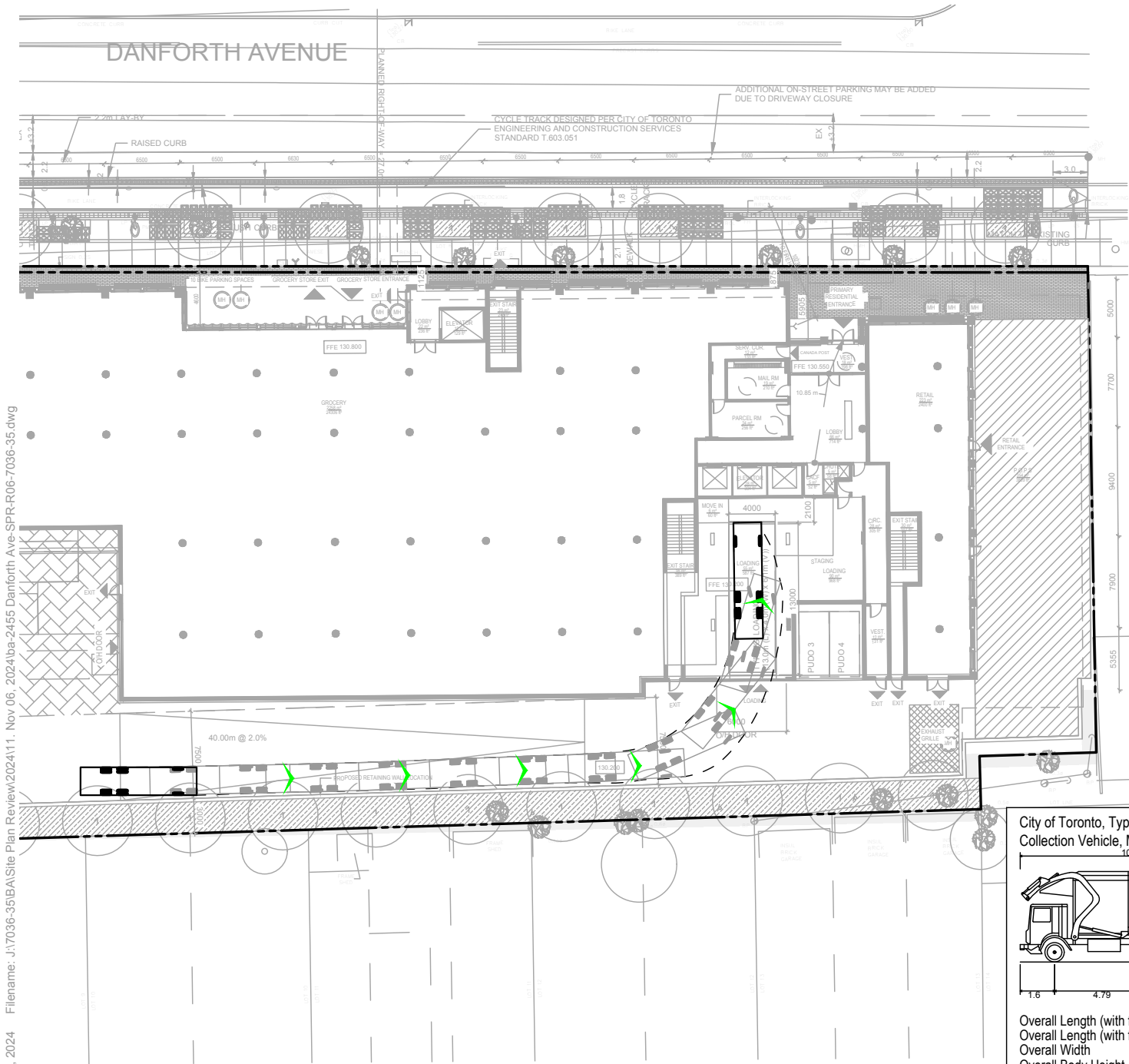
2451 - 2495 DANFORTH AVENUE
VEHICLE MANOEUVRING DIAGRAM
HEAVY SINGLE UNIT (HSU) VEHICLE
TYPE 'B' LOADING SPACE

Project: 2451-2495 DANFORTH
Project No. 7036-35
Date: April 09, 2024
Revised: November 8, 2024

Scale 0 2 4 6 8 10 20m
1:400
Drawing No. VMD-04

INBOUND:

OUTBOUND:



City of Toronto, Typical Front-End Waste Collection Vehicle, May 2023

Overall Length (with forks up)	10.00m
Overall Length (with forks down)	12.00m
Overall Width	2.40m
Overall Body Height	4.20m
Outside Turning Radius	14.00m
Inside Turning Radius	9.50m

(Dimensions as per City of Toronto Requirements for Garbage, Recycling and Organics Collection Services for New Developments and Redevelopments (May 2023))

Date Plotted: November 8, 2024 Filename: J:\7036-35\BA\Site Plan Review\2024\11_Nov 06_2024\ba-2455 Danforth Ave-SPR-R06-7036-35.dwg



2451 - 2495 DANFORTH AVENUE
VEHICLE MANOEUVRING DIAGRAM
CITY OF TORONTO TYPICAL FRONT-LOADING WASTE COLLECTION VEHICLE
TYPE 'G' LOADING SPACE

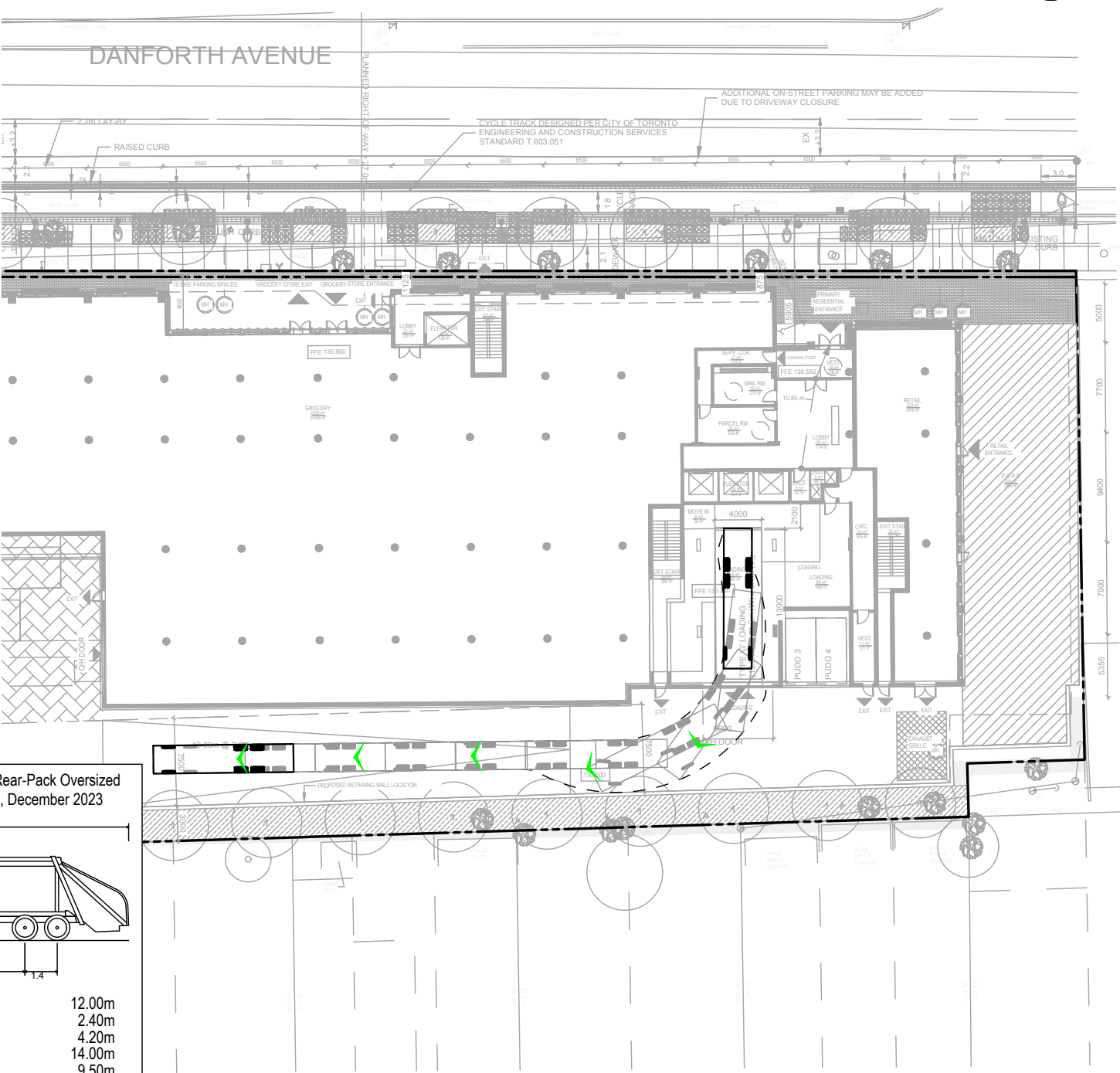
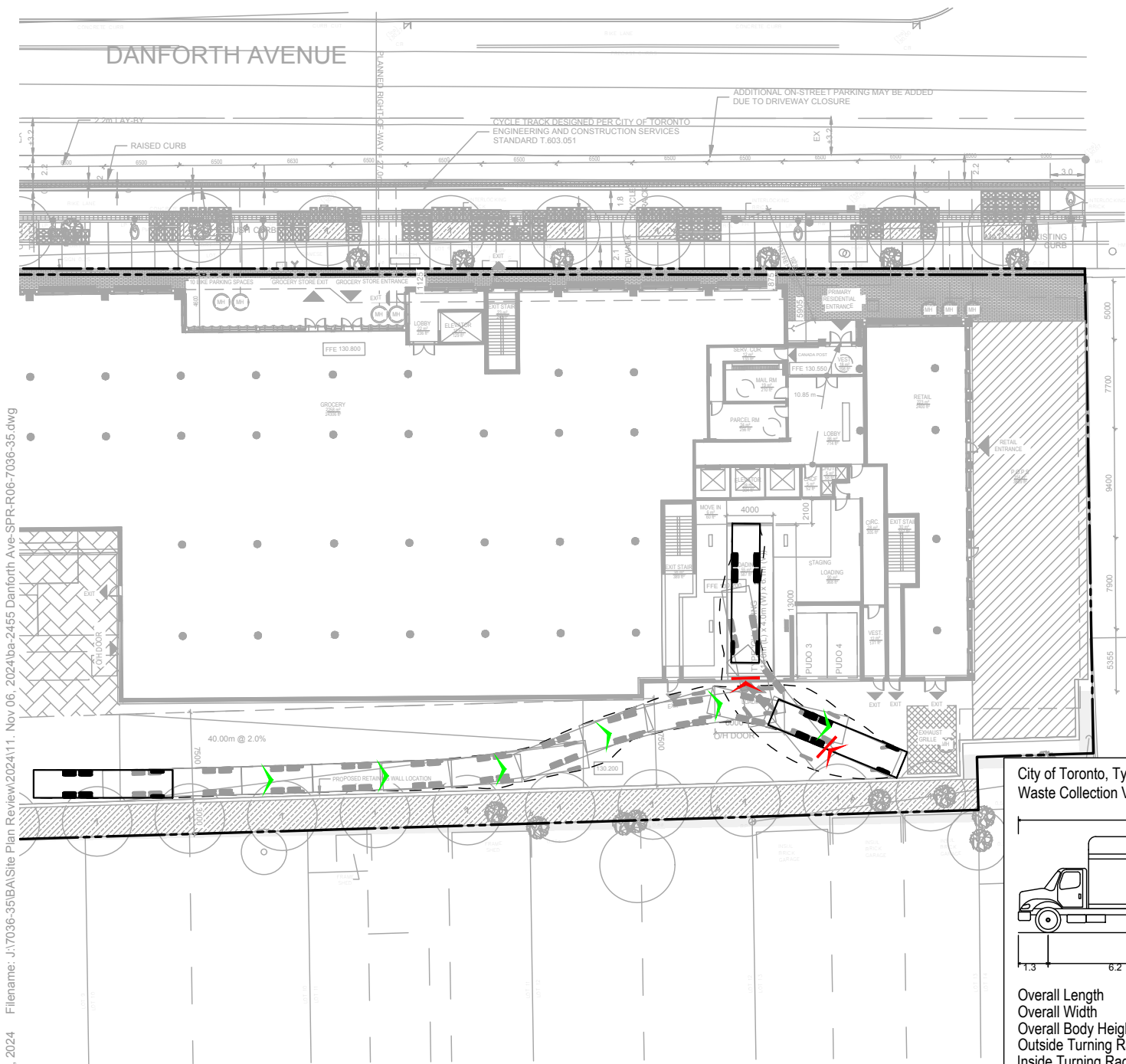
Project: 2451-2495 DANFORTH
Project No. 7036-35
Date: April 09, 2024
Revised: November 8, 2024

Scale 1:400

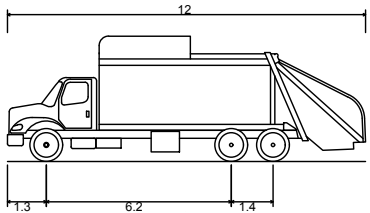
Drawing No. VMD-05

INBOUND:

OUTBOUND:



City of Toronto, Typical Rear-Pack Oversized Waste Collection Vehicle, December 2023



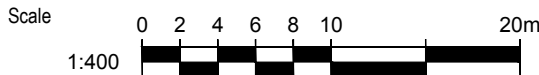
Overall Length 12.00m
Overall Width 2.40m
Overall Body Height 4.20m
Outside Turning Radius 14.00m
Inside Turning Radius 9.50m

(Dimensions as per City of Toronto Requirements for Garbage, Recycling and Organics Collection Services for New Developments and Redevelopments (May 2023))



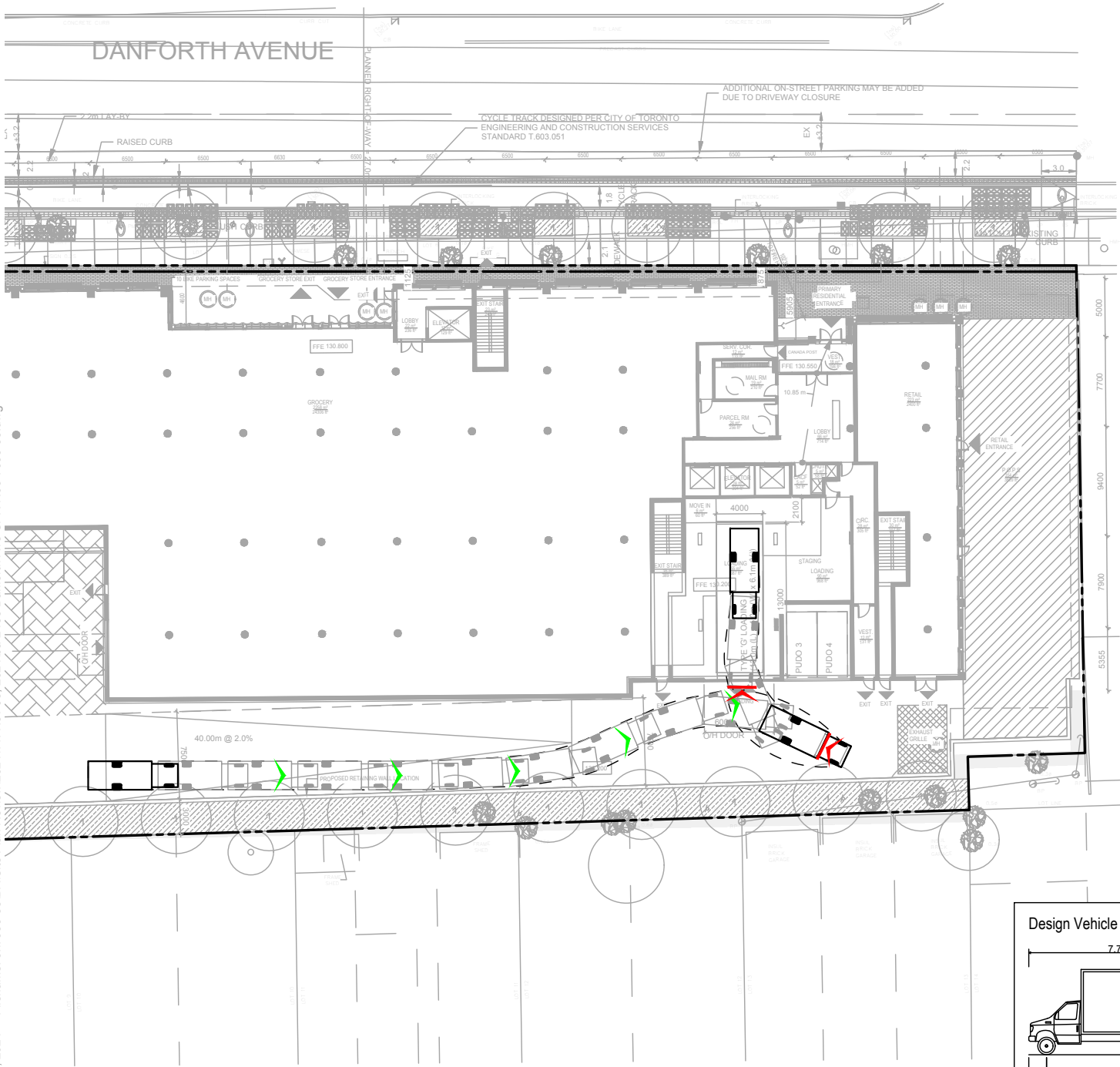
2451 - 2495 DANFORTH AVENUE
VEHICLE MANOEUVRING DIAGRAM
CITY OF TORONTO TYPICAL REAR-PACK WASTE COLLECTION VEHICLE
TYPE 'G' LOADING SPACE

Project: 2451-2495 DANFORTH
Project No. 7036-35
Date: April 09, 2024
Revised: November 8, 2024

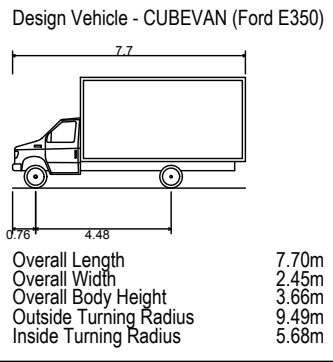
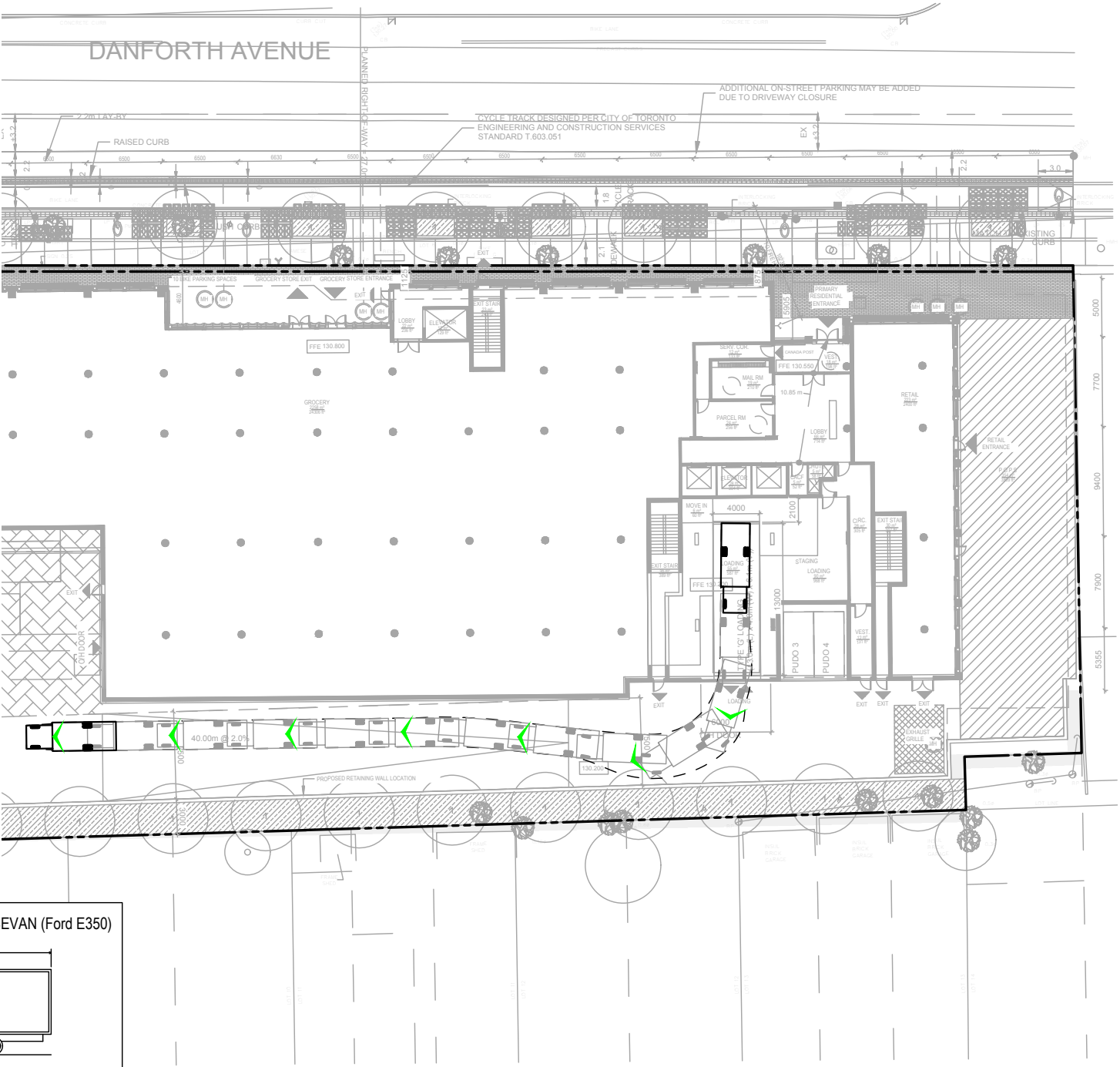


Drawing No. VMD-06

INBOUND:



OUTBOUND:

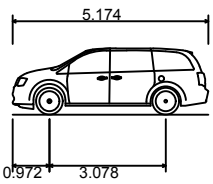
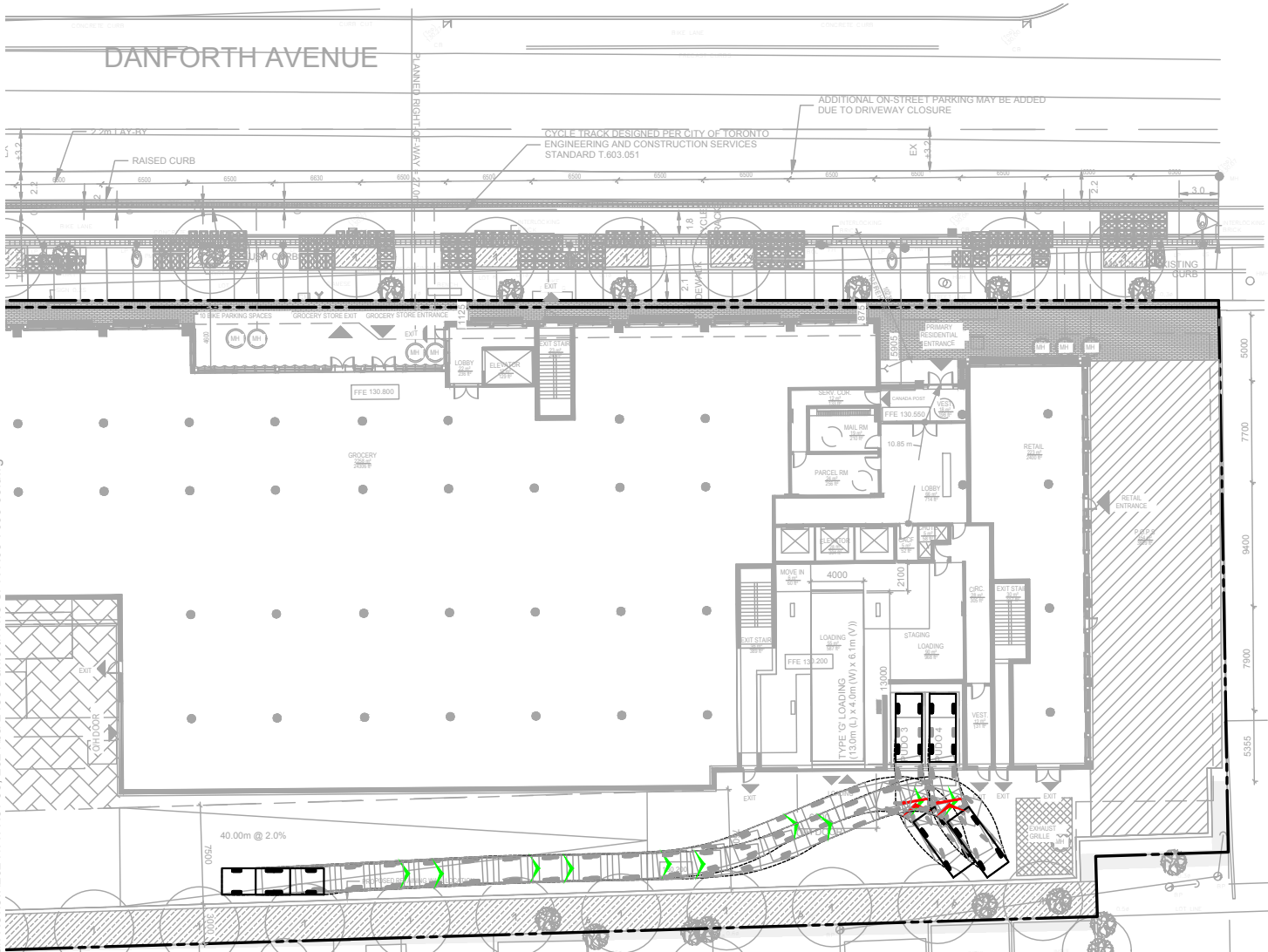


2451 - 2495 DANFORTH AVENUE
VEHICLE MANOEUVRING DIAGRAM
CUBEVAN (FORD E350)
TYPE 'G' LOADING SPACE

Project: 2451-2495 DANFORTH
Project No. 7036-35
Date: April 09, 2024
Revised: November 8, 2024



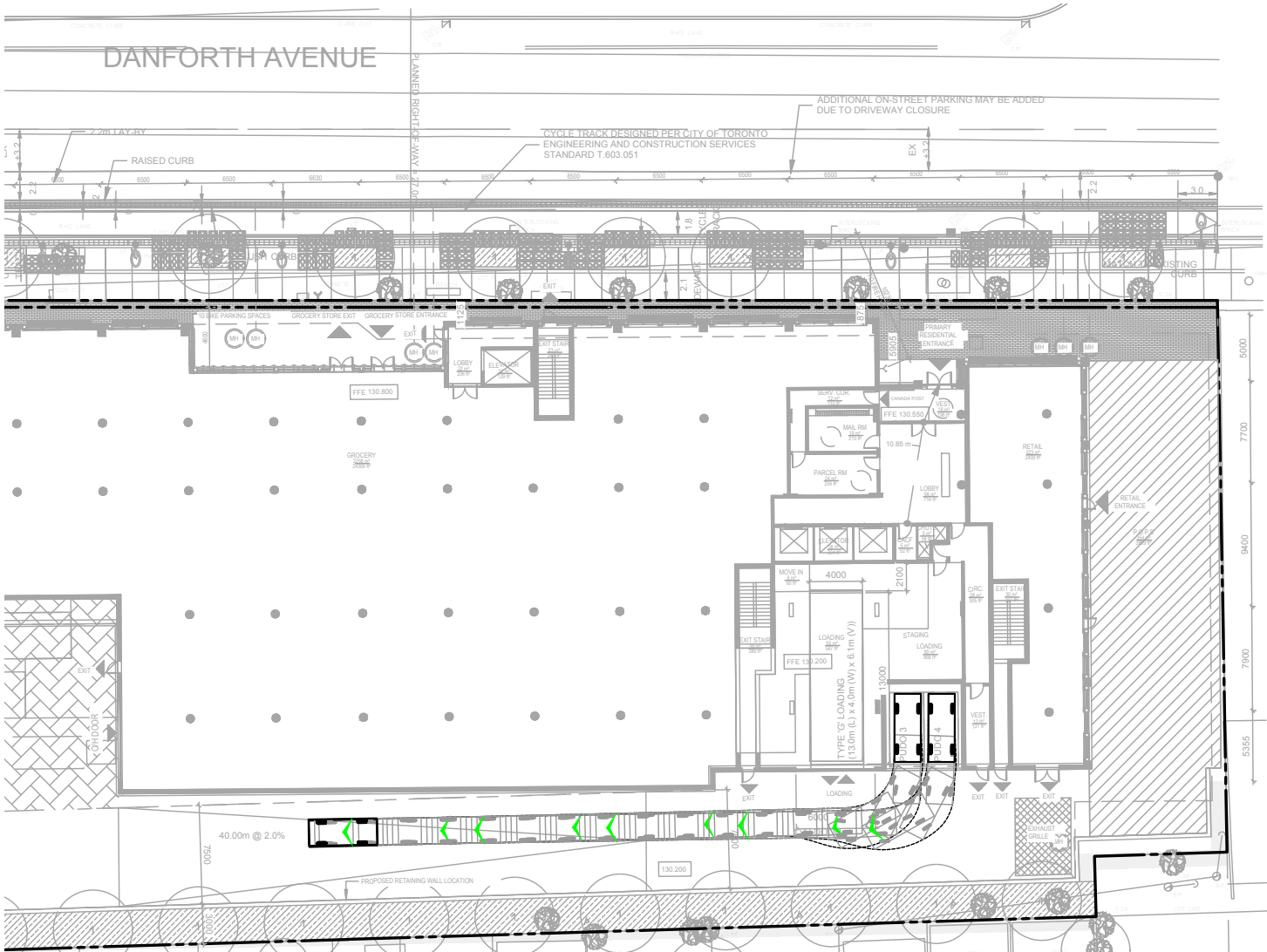
INBOUND:



2020 Dodge Grand Caravan GT

Overall Length	5.17m
Overall Width (without Mirrors)	2.00m
Overall Width (with Mirrors)	2.25m
Overall Body Height	1.75m
Outside Wall to Wall Turning Radius	6.50m
Inside Wall to Wall Turning Radius	3.35m

OUTBOUND:



Date Plotted: November 8, 2024 Filename: J:\7036-35\BA\Site Plan Review\2024\11. Nov 06, 2024\ba-2455 Danforth Ave-SPR-R06-7036-35.dwg



2451 - 2495 DANFORTH AVENUE
VEHICLE MANOEUVRING DIAGRAM
2020 DODGE GRAND CARAVAN (95TH PERCENTILE VEHICLE)
PICK-UP / DROP-OFF

Project: 2451-2495 DANFORTH
Project No. 7036-35
Date: April 09, 2024
Revised: November 8, 2024



Drawing No. VMD-08

[illegible]

an GT

(rors)

(s)

ing Radius

g Radius

5.17m

2.00m

2.25m

1.75m

6.50m

3.35m

T OF A FIXED OBJECT SUCH AS A WALL OR COLUMN IS



VEHICLE MANOEUVRING DIAGRAM

PARKING RAMP - 2020 DODGE GRAND CARAVAN (95TH PERCENTILE VEHICLE)
TWO-WAY MANOEUVRING

Revised: November 8, 2024

Scale

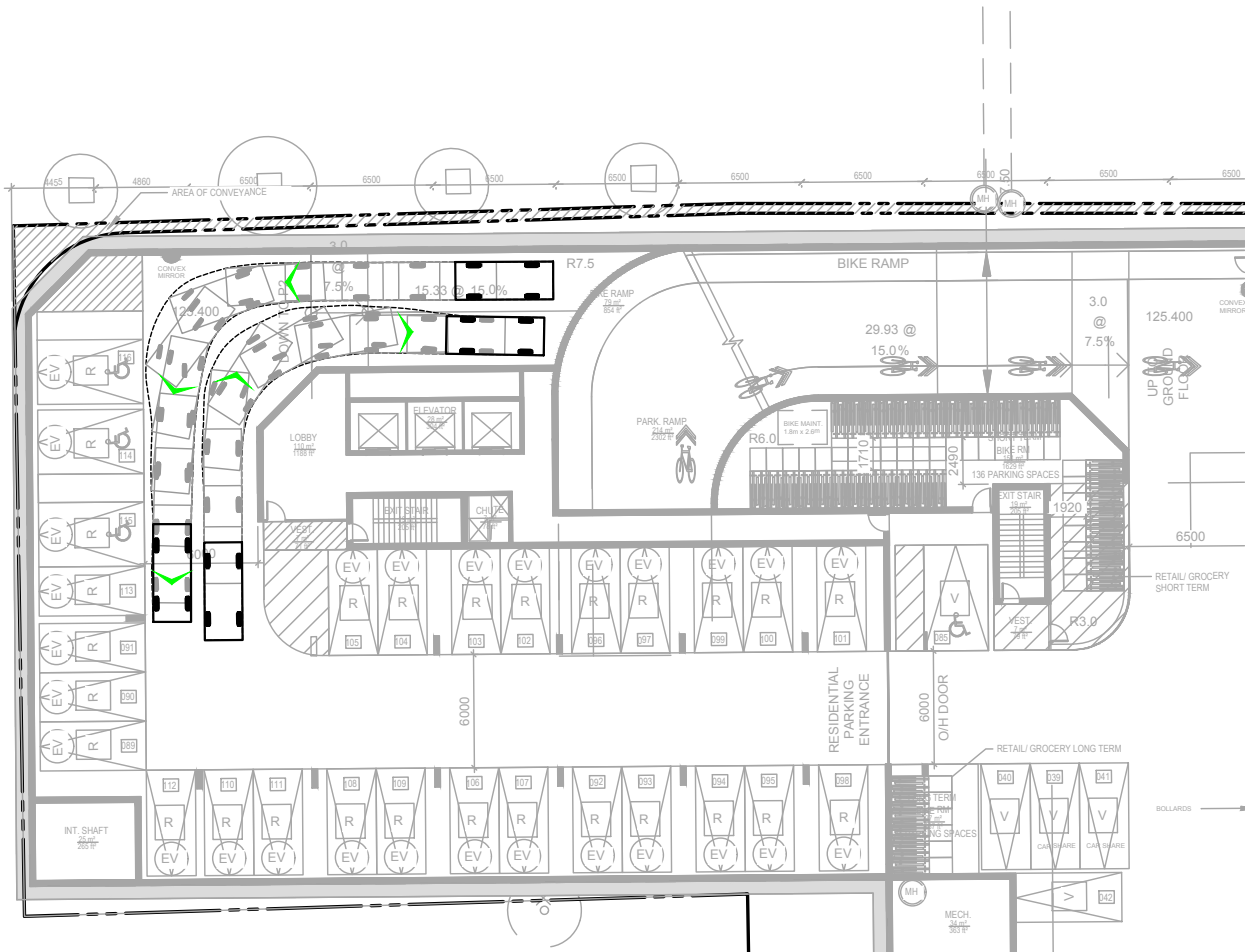
0 2 4 6 8 10 20m

1:400

Drawing No. **VMD-09**

P1 TO P2:

P1 TO P2:



2020 Dodge Grand Caravan GT

Overall Length	5.17m
Overall Width (without Mirrors)	2.00m
Overall Width (with Mirrors)	2.25m
Overall Body Height	1.75m
Outside Wall to Wall Turning Radius	6.50m
Inside Wall to Wall Turning Radius	3.35m



2451 - 2495 DANFORTH AVENUE
VEHICLE MANOEUVRING DIAGRAM
PARKING RAMP - 2020 DODGE GRAND CARAVAN (95TH PERCENTILE VEHICLE)
TWO-WAY MANOEUVRING

Project: 2451-2495 DANFORTH
Project No. 7036-35
Date: April 09, 2024
Revised: November 8, 2024

Scale 0 2 4 6 8 10 20m
1:400
Drawing No. VMD-10

Appendix D:
Transportation Tomorrow Survey 2016 (TTS)

2451 Danforth Ave

7036-35

Residential Vehicular Site Traffic Distribution (AM Peak Hour)

Outbound

BA Group - JS

2024-10-28 Site: Trips from TTS TAZs 251 and 258

			Traffic Volume Allocation					
			NORTH	South	SOUTH	WEST	EAST	TOTAL
Zone	Trips	%	Woodbine	Woodbine	Main St	Danforth Ave	Danforth Ave	
PD 1 of Toronto	398	20%		60%	40%			100%
PD 3 of Toronto	34	2%	50%	50%				100%
PD 4 of Toronto	50	3%	100%					100%
PD 5 of Toronto	265	13%	100%					100%
250	48	2%	100%					100%
251	15	1%				50%	50%	100%
252	10	1%		40%	60%			100%
253	13	1%		20%	80%			100%
254	15	1%		40%	60%			100%
257	16	1%		20%	80%			100%
258	93	5%				100%		100%
259	13	1%	100%					100%
263	63	3%	70%			30%		100%
265	15	1%		60%		40%		100%
269	14	1%				100%		100%
273	23	1%				100%		100%
281	39	2%	30%			70%		100%
PD 8 of Toronto	55	3%	30%	70%				100%
PD 9 of Toronto	66	3%	30%	70%				100%
PD 10 of Toronto	114	6%	100%					100%
PD 11 of Toronto	8	0%	100%					100%
PD 12 of Toronto	55	3%	100%					100%
PD 13 of Toronto	52	3%					100%	100%
PD 14 of Toronto	190	10%					100%	100%
PD 16 of Toronto	57	3%					100%	100%
Oshawa	15	1%					100%	100%
East Gwillimbury	9	0%	100%					100%
Whitchurch-Stou	18	1%	100%					100%
Markham	127	6%	100%					100%
Vaughan	26	1%	100%					100%
Brampton	14	1%	100%					100%
Mississauga	29	1%	30%	70%				100%
Oakville	5	0%	30%	70%				100%
Total Trips	1964	100%						

Assumed Split

Route Split Totals					
NORTH	SOUTH	SOUTH	WEST	EAST	TOTAL
Woodbine Ave	Woodbine Ave	Main Ave	Danforth Ave	Danforth Ave	
0%	12%	8%	0%	0%	20%
1%	1%	0%	0%	0%	2%
3%	0%	0%	0%	0%	3%
13%	0%	0%	0%	0%	13%
2%	0%	0%	0%	0%	2%
0%	0%	0%	0%	0%	1%
0%	0%	0%	0%	0%	1%
0%	0%	1%	0%	0%	1%
0%	0%	0%	0%	0%	1%
0%	0%	0%	0%	0%	1%
0%	0%	1%	0%	0%	1%
0%	0%	0%	5%	0%	5%
1%	0%	0%	0%	0%	1%
2%	0%	0%	1%	0%	3%
0%	0%	0%	0%	0%	1%
0%	0%	0%	1%	0%	1%
0%	0%	0%	1%	0%	1%
1%	0%	0%	1%	0%	2%
1%	2%	0%	0%	0%	3%
1%	2%	0%	0%	0%	3%
6%	0%	0%	0%	0%	6%
0%	0%	0%	0%	0%	0%
3%	0%	0%	0%	0%	3%
0%	0%	0%	0%	3%	3%
0%	0%	0%	0%	10%	10%
0%	0%	0%	0%	3%	3%
0%	0%	0%	0%	1%	1%
0%	0%	0%	0%	0%	0%
1%	0%	0%	0%	0%	1%
6%	0%	0%	0%	0%	6%
1%	0%	0%	0%	0%	1%
1%	0%	0%	0%	0%	1%
0%	1%	0%	0%	0%	1%
0%	0%	0%	0%	0%	0%
44.1%	19.8%	10.1%	9.7%	16.4%	100.0%

45%	20%	10%	10%	15%	100%
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2451 Danforth Ave

7036-35

Residential Vehicular Site Traffic Distribution (AM Peak Hour)

Outbound

BA Group - JS

2024-10-28 ote: Trips from TTS TAZs 251 and 258

			Traffic Volume Allocation					
Zone	Trips	%	NORTH	SOUTH	SOUTH	WEST	EAST	TOTAL
			Woodbine	Woodbine	Main St	Danforth Ave	Danforth Ave	
PD 1 of Toronto	369	18%	20%	80%				100%
PD 2 of Toronto	12	1%	20%	80%				100%
PD 3 of Toronto	69	3%	80%			20%		100%
PD 4 of Toronto	46	2%	100%					100%
PD 5 of Toronto	221	11%	100%					100%
	249	45	2%	100%				100%
	250	21	1%	100%				100%
	251	15	1%			50%	50%	100%
	252	10	0%	50%	50%			100%
	253	11	1%	25%	75%			100%
	254	15	1%	25%	75%			100%
	255	8	0%	75%	25%			100%
	256	15	1%	25%	75%			100%
	257	5	0%	100%				100%
	258	45	2%			100%		100%
	262	9	0%	100%				100%
	263	61	3%			100%		100%
	264	11	1%			100%		100%
	269	59	3%	20%		80%		100%
	272	27	1%			100%		100%
	279	142	7%			100%		100%
	281	15	1%			100%		100%
	282	24	1%			100%		100%
PD 9 of Toronto	66	3%	100%					100%
PD 10 of Torontc	59	3%	100%					100%
PD 11 of Torontc	19	1%	100%					100%
PD 12 of Torontc	83	4%	100%					100%
PD 13 of Torontc	112	6%					100%	100%
PD 14 of Torontc	66	3%					100%	100%
PD 15 of Torontc	55	3%					100%	100%
PD 16 of Torontc	86	4%					100%	100%
Whitchurch-Stou	18	1%	100%					100%
Markham	115	6%	100%					100%
Vaughan	16	1%	100%					100%
Brampton	22	1%	40%	60%				100%
Mississauga	27	1%		100%				100%
Hamilton	10	0%		100%				100%
Total Trips	2009	100%						

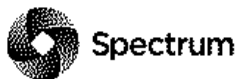
Assumed Split

Route Split Totals					
NORTH	SOUTH	SOUTH	WEST	EAST	TOTAL
Woodbine Ave	Woodbine Ave	Main Street	Danforth Ave	Danforth Ave	
4%	15%	0%	0%	0%	18%
0%	0%	0%	0%	0%	1%
3%	0%	0%	1%	0%	3%
2%	0%	0%	0%	0%	2%
11%	0%	0%	0%	0%	11%
2%	0%	0%	0%	0%	2%
1%	0%	0%	0%	0%	1%
0%	0%	0%	0%	0%	1%
0%	0%	0%	0%	0%	0%
0%	0%	0%	0%	0%	1%
0%	0%	1%	0%	0%	1%
0%	0%	0%	0%	0%	0%
0%	0%	0%	2%	0%	2%
0%	0%	0%	0%	0%	0%
0%	0%	0%	3%	0%	3%
0%	0%	0%	1%	0%	1%
0%	1%	0%	2%	0%	3%
0%	0%	0%	1%	0%	1%
0%	0%	0%	7%	0%	7%
0%	0%	0%	1%	0%	1%
0%	0%	0%	1%	0%	1%
3%	0%	0%	0%	0%	3%
3%	0%	0%	0%	0%	3%
1%	0%	0%	0%	0%	1%
4%	0%	0%	0%	0%	4%
0%	0%	0%	0%	6%	6%
0%	0%	0%	0%	3%	3%
0%	0%	0%	0%	3%	3%
0%	0%	0%	0%	4%	4%
1%	0%	0%	0%	0%	1%
6%	0%	0%	0%	0%	6%
1%	0%	0%	0%	0%	1%
0%	1%	0%	0%	0%	1%
0%	1%	0%	0%	0%	1%
0%	0%	0%	0%	0%	0%
42.7%	19.6%	1.9%	19.6%	16.3%	100.0%

45%	20%	0%	20%	15%	100%
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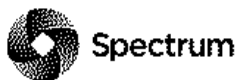
Appendix E:

Turning Movement Counts (TMCs)



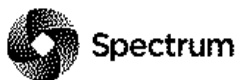
Turning Movement Count (5 . DANFORTH AVE & 2451 DANFORTH AVE SOBEYS EAST DRIVEWAY)

Start Time	E Approach DANFORTH AVE					S Approach 2451 DANFORTH AVE (SOBEYS EAST DRIVEWAY)					W Approach DANFORTH AVE					Int. Total (15 min)	Int. Total (1 hr)
	Thru E:W	Left E:S	UTurn E:E	Peds E:	Approach Total	Right S:E	Left S:W	UTurn S:S	Peds S:	Approach Total	Right W:S	Thru W:E	UTurn W:W	Peds W:	Approach Total		
07:30:00	138	2	0	4	140	3	0	0	31	3	1	72	0	0	73	216	
07:45:00	142	2	0	1	144	1	1	0	24	2	3	95	0	0	98	244	
08:00:00	131	2	0	0	133	3	0	0	23	3	2	128	0	1	130	266	
08:15:00	146	4	0	2	150	6	3	0	30	9	6	115	0	1	121	280	1006
08:30:00	173	9	0	0	182	5	1	0	59	6	3	114	0	2	117	305	1095
08:45:00	143	1	0	3	144	1	3	0	38	4	4	121	0	1	125	273	1124
09:00:00	147	8	0	1	155	8	2	0	22	10	1	116	0	3	117	282	1140
09:15:00	144	3	0	2	147	7	1	0	28	8	0	109	0	0	109	264	1124
BREAK																	
16:00:00	121	4	1	3	126	8	2	0	92	10	0	113	0	0	113	249	
16:15:00	111	4	1	4	116	4	2	0	71	6	4	95	0	0	99	221	
16:30:00	123	2	0	2	125	6	4	0	65	10	4	100	0	1	104	239	
16:45:00	115	2	0	4	117	5	3	0	67	8	4	111	0	2	115	240	949
17:00:00	108	9	1	1	118	8	0	0	74	8	4	81	0	3	85	211	911
17:15:00	110	1	0	6	111	8	1	0	112	9	2	92	0	1	94	214	904
17:30:00	112	2	0	2	114	4	1	0	112	5	3	60	0	2	63	182	847
17:45:00	109	9	0	0	118	2	3	0	74	5	6	97	0	1	103	226	833
Grand Total	2073	64	3	35	2140	79	27	0	922	106	47	1619	0	18	1666	3912	-
Approach%	96.9%	3%	0.1%	-	-	74.5%	25.5%	0%	-	-	2.8%	97.2%	0%	-	-	-	-
Totals %	53%	1.6%	0.1%	-	54.7%	2%	0.7%	0%	-	2.7%	1.2%	41.4%	0%	-	42.6%	-	-
Heavy	74	0	0	-	-	0	0	0	-	-	0	81	0	-	-	-	-
Heavy %	3.6%	0%	0%	-	-	0%	0%	0%	-	-	0%	5%	0%	-	-	-	-
Bicycles	258	0	0	-	-	0	0	0	-	-	0	200	0	-	-	-	-
Bicycle %	12.4%	0%	0%	-	-	0%	0%	0%	-	-	0%	12.4%	0%	-	-	-	-



Peak Hour: 08:15 AM - 09:15 AM Weather: Clear Sky (13.64 °C)

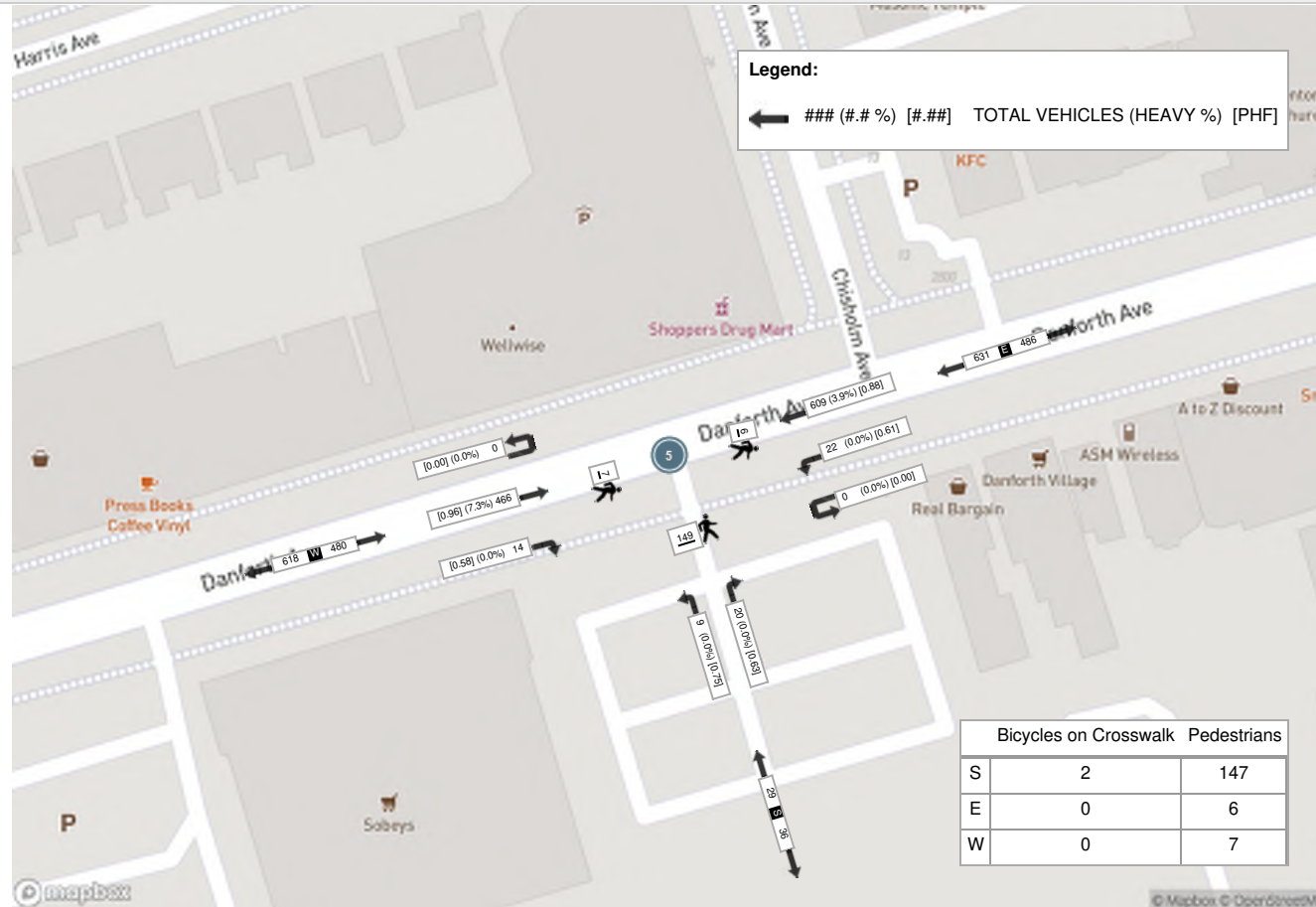
Start Time	E Approach DANFORTH AVE					S Approach 2451 DANFORTH AVE (SOBEYS EAST DRIVEWAY)					W Approach DANFORTH AVE					Int. Total (15 min)
	Thru	Left	UTurn	Peds	Approach Total	Right	Left	UTurn	Peds	Approach Total	Right	Thru	UTurn	Peds	Approach Total	
08:15:00	146	4	0	2	150	6	3	0	30	9	6	115	0	1	121	280
08:30:00	173	9	0	0	182	5	1	0	59	6	3	114	0	2	117	305
08:45:00	143	1	0	3	144	1	3	0	38	4	4	121	0	1	125	273
09:00:00	147	8	0	1	155	8	2	0	22	10	1	116	0	3	117	282
Grand Total	609	22	0	6	631	20	9	0	149	29	14	466	0	7	480	1140
Approach%	96.5%	3.5%	0%		-	69%	31%	0%		-	2.9%	97.1%	0%		-	-
Totals %	53.4%	1.9%	0%		55.4%	1.8%	0.8%	0%		2.5%	1.2%	40.9%	0%		42.1%	-
PHF	0.88	0.61	0		0.87	0.63	0.75	0		0.73	0.58	0.96	0		0.96	-
Heavy	24	0	0		24	0	0	0		0	0	34	0		34	-
Heavy %	3.9%	0%	0%		3.8%	0%	0%	0%		0%	0%	7.3%	0%		7.1%	-
Lights	585	22	0		607	20	9	0		29	14	432	0		446	-
Lights %	96.1%	100%	0%		96.2%	100%	100%	0%		100%	100%	92.7%	0%		92.9%	-
Single-Unit Trucks	10	0	0		10	0	0	0		0	0	16	0		16	-
Single-Unit Trucks %	1.6%	0%	0%		1.6%	0%	0%	0%		0%	0%	3.4%	0%		3.3%	-
Buses	12	0	0		12	0	0	0		0	0	18	0		18	-
Buses %	2%	0%	0%		1.9%	0%	0%	0%		0%	0%	3.9%	0%		3.8%	-
Articulated Trucks	2	0	0		2	0	0	0		0	0	0	0		0	-
Articulated Trucks %	0.3%	0%	0%		0.3%	0%	0%	0%		0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	6	-	-	-	-	147	-	-	-	-	7	-	-
Pedestrians%	-	-	-	3.7%	-	-	-	-	90.7%	-	-	-	-	4.3%	-	-
Bicycles on Road	74	0	0	0	-	0	0	0	0	-	0	28	0	0	-	-
Bicycles on Road%	-	-	-	0%	-	-	-	-	0%	-	-	-	-	0%	-	-
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	2	-	-	-	-	0	-	-
Bicycles on Crosswalk%	-	-	-	0%	-	-	-	-	1.2%	-	-	-	-	0%	-	-



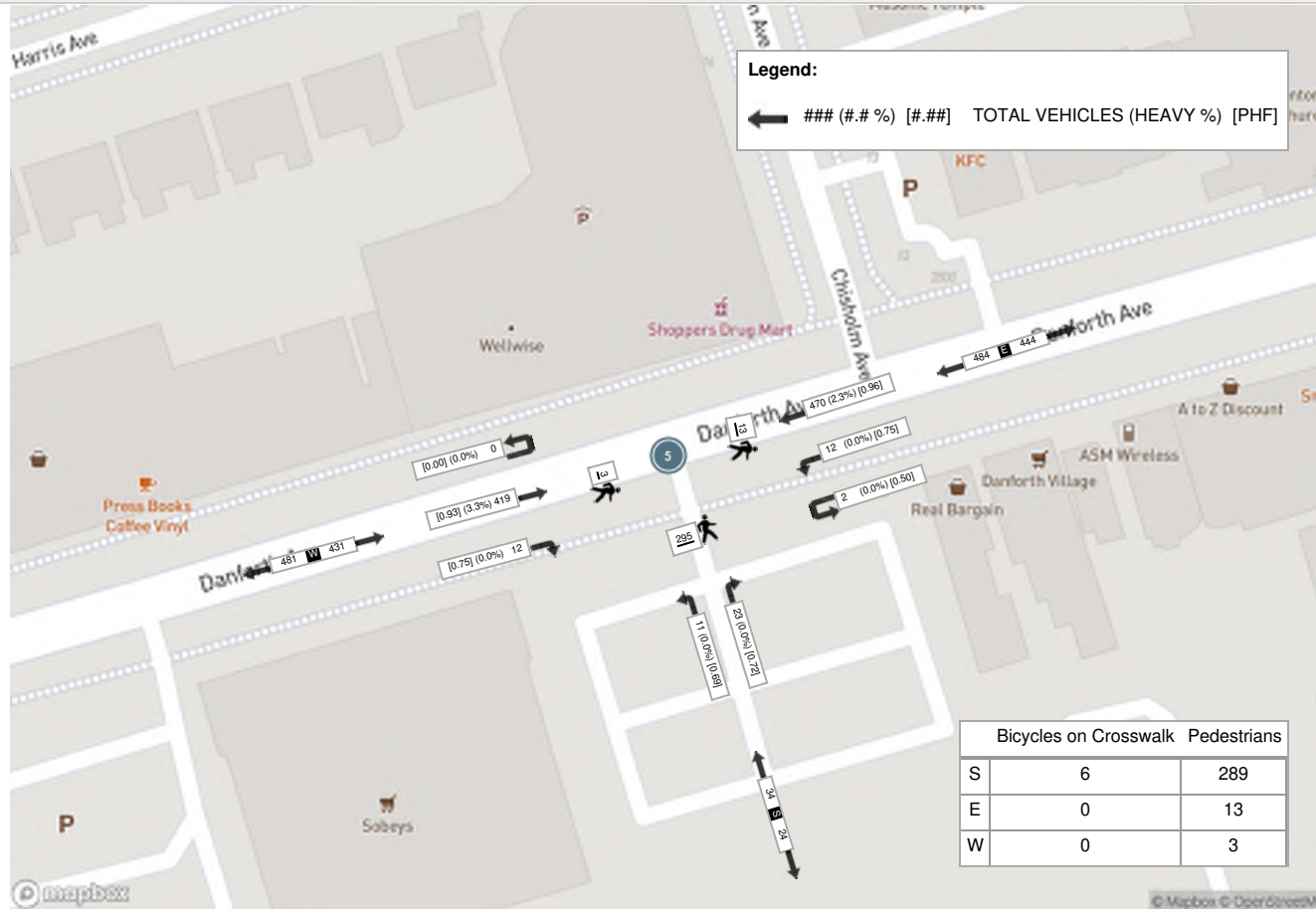
Peak Hour: 04:00 PM - 05:00 PM Weather: Clear Sky (19.95 °C)

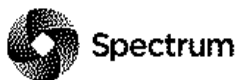
Start Time	E Approach DANFORTH AVE					S Approach 2451 DANFORTH AVE (SOBEYS EAST DRIVEWAY)					W Approach DANFORTH AVE					Int. Total (15 min)
	Thru	Left	UTurn	Peds	Approach Total	Right	Left	UTurn	Peds	Approach Total	Right	Thru	UTurn	Peds	Approach Total	
16:00:00	121	4	1	3	126	8	2	0	92	10	0	113	0	0	113	249
16:15:00	111	4	1	4	116	4	2	0	71	6	4	95	0	0	99	221
16:30:00	123	2	0	2	125	6	4	0	65	10	4	100	0	1	104	239
16:45:00	115	2	0	4	117	5	3	0	67	8	4	111	0	2	115	240
Grand Total	470	12	2	13	484	23	11	0	295	34	12	419	0	3	431	949
Approach%	97.1%	2.5%	0.4%		-	67.6%	32.4%	0%		-	2.8%	97.2%	0%		-	-
Totals %	49.5%	1.3%	0.2%		51%	2.4%	1.2%	0%		3.6%	1.3%	44.2%	0%		45.4%	-
PHF	0.96	0.75	0.5		0.96	0.72	0.69	0		0.85	0.75	0.93	0		0.94	-
Heavy	11	0	0		11	0	0	0		0	0	14	0		14	-
Heavy %	2.3%	0%	0%		2.3%	0%	0%	0%		0%	0%	3.3%	0%		3.2%	-
Lights	459	12	2		473	23	11	0		34	12	405	0		417	-
Lights %	97.7%	100%	100%		97.7%	100%	100%	0%		100%	100%	96.7%	0%		96.8%	-
Single-Unit Trucks	4	0	0		4	0	0	0		0	0	8	0		8	-
Single-Unit Trucks %	0.9%	0%	0%		0.8%	0%	0%	0%		0%	0%	1.9%	0%		1.9%	-
Buses	7	0	0		7	0	0	0		0	0	6	0		6	-
Buses %	1.5%	0%	0%		1.4%	0%	0%	0%		0%	0%	1.4%	0%		1.4%	-
Articulated Trucks	0	0	0		0	0	0	0		0	0	0	0		0	-
Articulated Trucks %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	13	-	-	-	-	289	-	-	-	-	3	-	-
Pedestrians%	-	-	-	4.2%	-	-	-	-	92.9%	-	-	-	-	1%	-	-
Bicycles on Road	56	0	0	0	-	0	0	0	0	-	0	72	0	0	-	-
Bicycles on Road%	-	-	-	0%	-	-	-	-	0%	-	-	-	-	0%	-	-
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	6	-	-	-	-	0	-	-
Bicycles on Crosswalk%	-	-	-	0%	-	-	-	-	1.9%	-	-	-	-	0%	-	-

Peak Hour: 08:15 AM - 09:15 AM Weather: Clear Sky (13.64 °C)



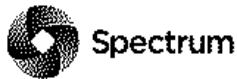
Peak Hour: 04:00 PM - 05:00 PM Weather: Clear Sky (19.95 °C)





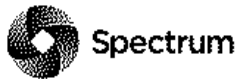
Turning Movement Count (4 . DANFORTH AVE & 2451 DANFORTH AVE SOBEYS WEST DRIVEWAY)

Start Time	E Approach DANFORTH AVE					S Approach 2451 DANFORTH AVE (SOBEYS WEST DRIVEWAY)					W Approach DANFORTH AVE					Int. Total (15 min)	Int. Total (1 hr)
	Thru E:W	Left E:S	UTurn E:E	Peds E:	Approach Total	Right S:E	Left S:W	UTurn S:S	Peds S:	Approach Total	Right W:S	Thru W:E	UTurn W:W	Peds W:	Approach Total		
07:30:00	137	0	0	1	137	0	0	0	24	0	0	71	0	0	71	208	
07:45:00	145	0	0	2	145	0	0	0	20	0	0	102	0	1	102	247	
08:00:00	130	0	0	0	130	0	0	0	16	0	1	126	0	0	127	257	
08:15:00	147	0	0	0	147	2	0	0	27	2	0	119	0	0	119	268	980
08:30:00	172	1	0	0	173	1	0	0	41	1	1	116	0	0	117	291	1063
08:45:00	142	1	0	0	143	3	1	0	40	4	1	123	0	2	124	271	1087
09:00:00	144	1	0	2	145	2	0	0	19	2	0	107	0	0	107	254	1084
09:15:00	145	0	0	0	145	2	0	0	24	2	2	111	0	1	113	260	1076
BREAK																	
16:00:00	129	1	0	0	130	2	0	0	62	2	0	108	0	0	108	240	
16:15:00	109	1	0	1	110	1	0	0	58	1	2	110	0	0	112	223	
16:30:00	122	0	0	2	122	2	0	0	46	2	1	103	0	2	104	228	
16:45:00	111	1	0	1	112	1	0	0	54	1	1	119	0	0	120	233	924
17:00:00	114	1	0	1	115	0	0	0	43	0	1	84	0	1	85	200	884
17:15:00	108	1	1	1	110	3	0	0	62	3	2	84	0	0	86	199	860
17:30:00	115	0	0	1	115	1	1	0	60	2	1	65	0	1	66	183	815
17:45:00	108	0	0	0	108	2	0	0	48	2	2	98	0	1	100	210	792
Grand Total	2078	8	1	12	2087	22	2	0	644	24	15	1646	0	9	1661	3772	-
Approach%	99.6%	0.4%	0%		-	91.7%	8.3%	0%		-	0.9%	99.1%	0%		-	-	-
Totals %	55.1%	0.2%	0%		55.3%	0.6%	0.1%	0%		0.6%	0.4%	43.6%	0%		44%	-	-
Heavy	70	0	0		-	2	0	0		-	0	82	0		-	-	-
Heavy %	3.4%	0%	0%		-	9.1%	0%	0%		-	0%	5%	0%		-	-	-
Bicycles	264	0	0		-	2	0	0		-	0	199	0		-	-	-
Bicycle %	12.7%	0%	0%		-	9.1%	0%	0%		-	0%	12.1%	0%		-	-	-



Peak Hour: 08:00 AM - 09:00 AM Weather: Clear Sky (13.64 °C)

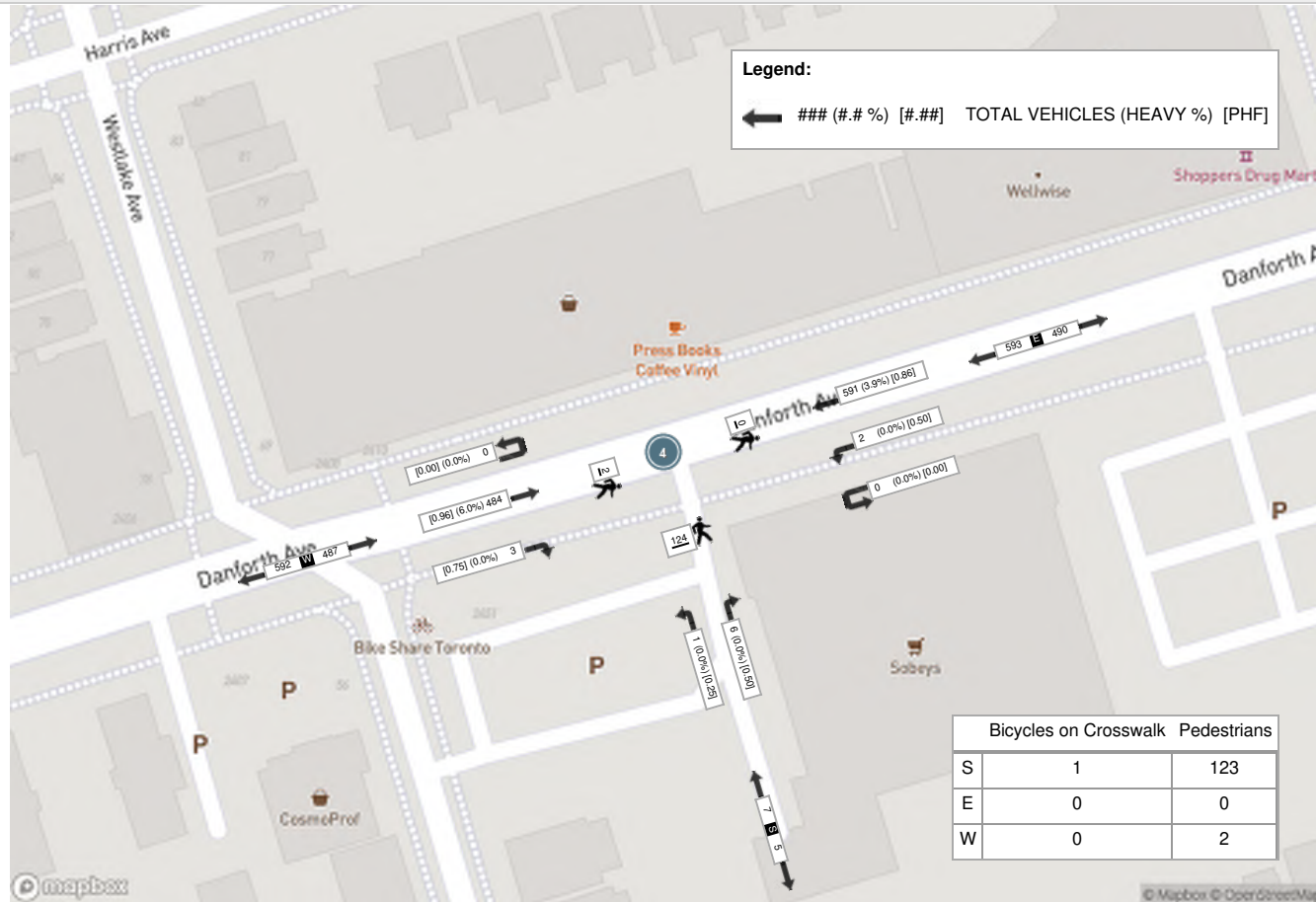
Start Time	E Approach DANFORTH AVE					S Approach 2451 DANFORTH AVE (SOBEYS WEST DRIVEWAY)					W Approach DANFORTH AVE					Int. Total (15 min)
	Thru	Left	UTurn	Peds	Approach Total	Right	Left	UTurn	Peds	Approach Total	Right	Thru	UTurn	Peds	Approach Total	
08:00:00	130	0	0	0	130	0	0	0	16	0	1	126	0	0	127	257
08:15:00	147	0	0	0	147	2	0	0	27	2	0	119	0	0	119	268
08:30:00	172	1	0	0	173	1	0	0	41	1	1	116	0	0	117	291
08:45:00	142	1	0	0	143	3	1	0	40	4	1	123	0	2	124	271
Grand Total	591	2	0	0	593	6	1	0	124	7	3	484	0	2	487	1087
Approach%	99.7%	0.3%	0%		-	85.7%	14.3%	0%		-	0.6%	99.4%	0%		-	-
Totals %	54.4%	0.2%	0%		54.6%	0.6%	0.1%	0%		0.6%	0.3%	44.5%	0%		44.8%	-
PHF	0.86	0.5	0		0.86	0.5	0.25	0		0.44	0.75	0.96	0		0.96	-
Heavy	23	0	0		23	0	0	0		0	0	29	0		29	-
Heavy %	3.9%	0%	0%		3.9%	0%	0%	0%		0%	0%	6%	0%		6%	-
Lights	568	2	0		570	6	1	0		7	3	455	0		458	-
Lights %	96.1%	100%	0%		96.1%	100%	100%	0%		100%	100%	94%	0%		94%	-
Single-Unit Trucks	12	0	0		12	0	0	0		0	0	16	0		16	-
Single-Unit Trucks %	2%	0%	0%		2%	0%	0%	0%		0%	0%	3.3%	0%		3.3%	-
Buses	10	0	0		10	0	0	0		0	0	12	0		12	-
Buses %	1.7%	0%	0%		1.7%	0%	0%	0%		0%	0%	2.5%	0%		2.5%	-
Articulated Trucks	1	0	0		1	0	0	0		0	0	1	0		1	-
Articulated Trucks %	0.2%	0%	0%		0.2%	0%	0%	0%		0%	0%	0.2%	0%		0.2%	-
Pedestrians	-	-	-	0	-	-	-	-	123	-	-	-	-	2	-	-
Pedestrians%	-	-	-	0%	-	-	-	-	97.6%	-	-	-	-	1.6%	-	-
Bicycles on Road	72	0	0	0	-	1	0	0	0	-	0	26	0	0	-	-
Bicycles on Road%	-	-	-	0%	-	-	-	-	0%	-	-	-	-	0%	-	-
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	1	-	-	-	-	0	-	-
Bicycles on Crosswalk%	-	-	-	0%	-	-	-	-	0.8%	-	-	-	-	0%	-	-



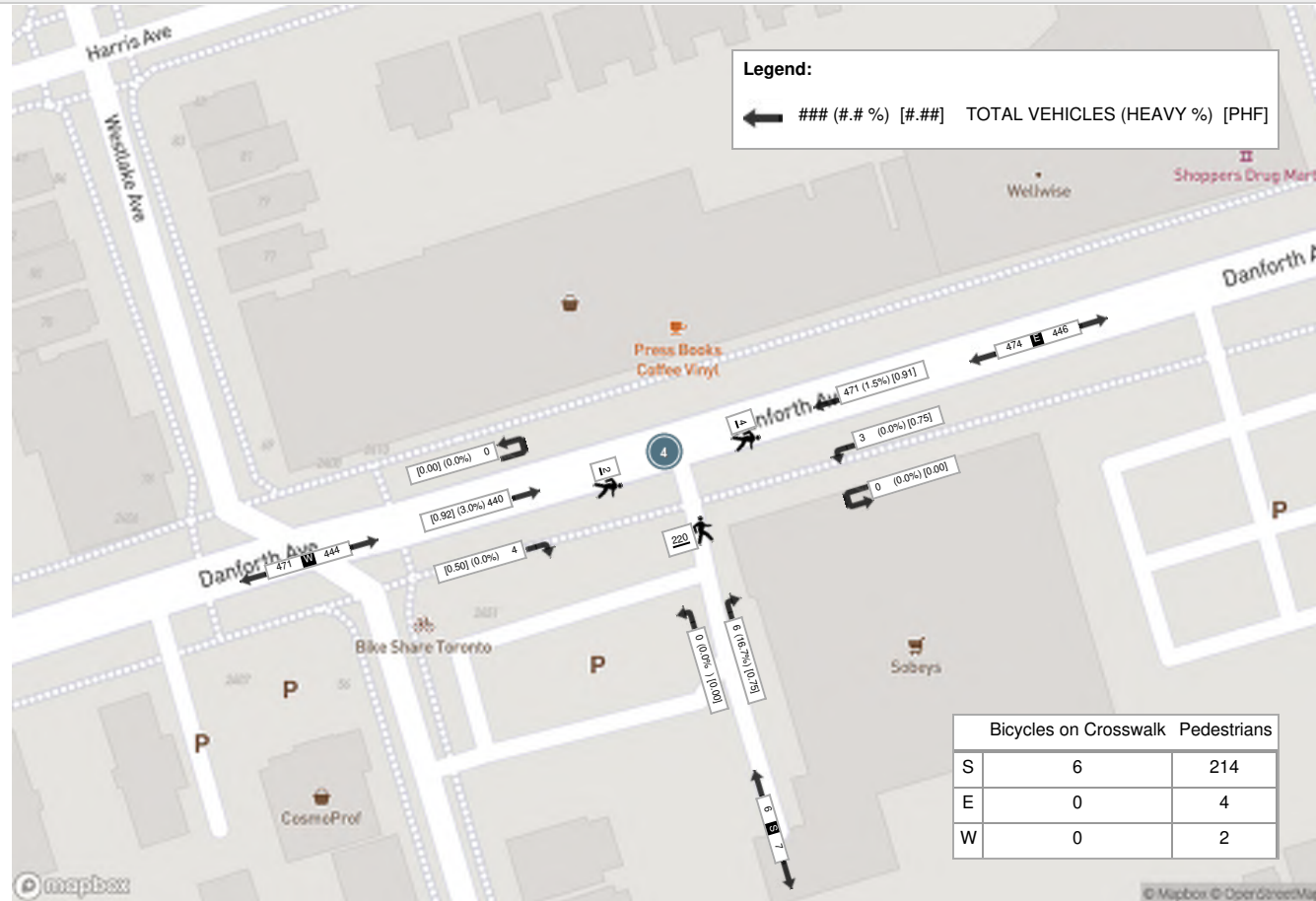
Peak Hour: 04:00 PM - 05:00 PM Weather: Clear Sky (19.95 °C)

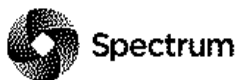
Start Time	E Approach DANFORTH AVE					S Approach 2451 DANFORTH AVE (SOBEYS WEST DRIVEWAY)					W Approach DANFORTH AVE					Int. Total (15 min)
	Thru	Left	UTurn	Peds	Approach Total	Right	Left	UTurn	Peds	Approach Total	Right	Thru	UTurn	Peds	Approach Total	
16:00:00	129	1	0	0	130	2	0	0	62	2	0	108	0	0	108	240
16:15:00	109	1	0	1	110	1	0	0	58	1	2	110	0	0	112	223
16:30:00	122	0	0	2	122	2	0	0	46	2	1	103	0	2	104	228
16:45:00	111	1	0	1	112	1	0	0	54	1	1	119	0	0	120	233
Grand Total	471	3	0	4	474	6	0	0	220	6	4	440	0	2	444	924
Approach%	99.4%	0.6%	0%		-	100%	0%	0%		-	0.9%	99.1%	0%		-	-
Totals %	51%	0.3%	0%		51.3%	0.6%	0%	0%		0.6%	0.4%	47.6%	0%		48.1%	-
PHF	0.91	0.75	0		0.91	0.75	0	0		0.75	0.5	0.92	0		0.93	-
Heavy	7	0	0		7	1	0	0		1	0	13	0		13	-
Heavy %	1.5%	0%	0%		1.5%	16.7%	0%	0%		16.7%	0%	3%	0%		2.9%	-
Lights	464	3	0		467	5	0	0		5	4	427	0		431	-
Lights %	98.5%	100%	0%		98.5%	83.3%	0%	0%		83.3%	100%	97%	0%		97.1%	-
Single-Unit Trucks	3	0	0		3	1	0	0		1	0	8	0		8	-
Single-Unit Trucks %	0.6%	0%	0%		0.6%	16.7%	0%	0%		16.7%	0%	1.8%	0%		1.8%	-
Buses	4	0	0		4	0	0	0		0	0	5	0		5	-
Buses %	0.8%	0%	0%		0.8%	0%	0%	0%		0%	0%	1.1%	0%		1.1%	-
Articulated Trucks	0	0	0		0	0	0	0		0	0	0	0		0	-
Articulated Trucks %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	4	-	-	-	-	214	-	-	-	-	2	-	-
Pedestrians%	-	-	-	1.8%	-	-	-	-	94.7%	-	-	-	-	0.9%	-	-
Bicycles on Road	55	0	0	0	-	1	0	0	0	-	0	71	0	0	-	-
Bicycles on Road%	-	-	-	0%	-	-	-	-	0%	-	-	-	-	0%	-	-
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	6	-	-	-	-	0	-	-
Bicycles on Crosswalk%	-	-	-	0%	-	-	-	-	2.7%	-	-	-	-	0%	-	-

Peak Hour: 08:00 AM - 09:00 AM Weather: Clear Sky (13.64 °C)



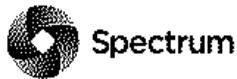
Peak Hour: 04:00 PM - 05:00 PM Weather: Clear Sky (19.95 °C)





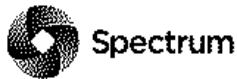
Turning Movement Count (6 . DANFORTH AVE & CHISHOLM AVE)

Start Time	N Approach CHISHOLM AVE					E Approach DANFORTH AVE					W Approach DANFORTH AVE					Int. Total (15 min)	Int. Total (1 hr)
	Right N:W	Left N:E	UTurn N:N	Peds N:	Approach Total	Right E:N	Thru E:W	UTurn E:E	Peds E:	Approach Total	Thru W:E	Left W:N	UTurn W:W	Peds W:	Approach Total		
07:30:00	3	2	0	22	5	9	139	1	0	149	72	3	0	0	75	229	
07:45:00	1	4	0	14	5	11	145	0	0	156	100	1	1	1	102	263	
08:00:00	7	6	0	18	13	6	124	0	1	130	124	3	0	2	127	270	
08:15:00	12	3	0	26	15	8	148	0	0	156	116	5	0	1	121	292	1054
08:30:00	7	2	0	24	9	10	176	0	0	186	113	3	0	1	116	311	1136
08:45:00	5	2	0	35	7	14	140	0	0	154	117	6	0	3	123	284	1157
09:00:00	5	4	0	40	9	9	148	0	2	157	121	3	1	1	125	291	1178
09:15:00	5	1	0	37	6	11	143	0	0	154	113	3	0	2	116	276	1162
BREAK																	
16:00:00	7	5	0	59	12	7	122	0	4	129	124	4	0	1	128	269	
16:15:00	3	2	0	54	5	4	110	0	1	114	105	4	0	0	109	228	
16:30:00	11	5	0	43	16	7	117	1	0	125	96	3	0	1	99	240	
16:45:00	4	6	0	36	10	10	114	0	1	124	113	3	0	2	116	250	987
17:00:00	6	1	0	50	7	6	108	0	1	114	80	6	0	0	86	207	925
17:15:00	8	0	0	60	8	9	105	0	1	114	89	9	0	2	98	220	917
17:30:00	7	3	0	54	10	19	113	0	2	132	55	5	0	1	60	202	879
17:45:00	5	3	0	43	8	8	109	0	1	117	97	4	0	2	101	226	855
Grand Total	96	49	0	615	145	148	2061	2	14	2211	1635	65	2	20	1702	4058	-
Approach%	66.2%	33.8%	0%	-	-	6.7%	93.2%	0.1%	-	-	96.1%	3.8%	0.1%	-	-	-	-
Totals %	2.4%	1.2%	0%	-	3.6%	3.6%	50.8%	0%	-	54.5%	40.3%	1.6%	0%	-	41.9%	-	-
Heavy	4	1	0	-	-	3	70	0	-	-	82	2	0	-	-	-	-
Heavy %	4.2%	2%	0%	-	-	2%	3.4%	0%	-	-	5%	3.1%	0%	-	-	-	-
Bicycles	11	3	0	-	-	5	243	0	-	-	204	0	0	-	-	-	-
Bicycle %	11.5%	6.1%	0%	-	-	3.4%	11.8%	0%	-	-	12.5%	0%	0%	-	-	-	-



Peak Hour: 08:15 AM - 09:15 AM Weather: Clear Sky (13.64 °C)

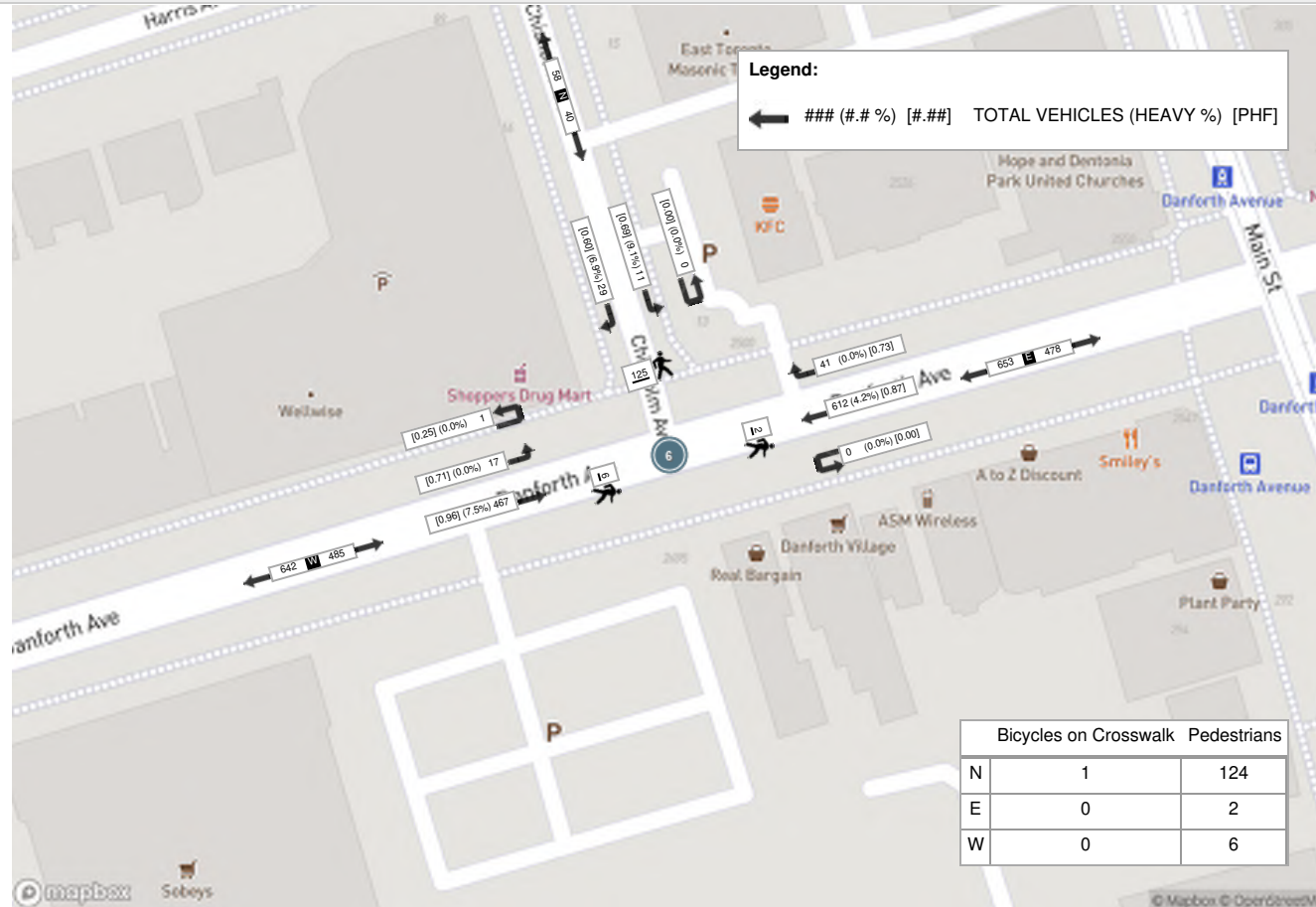
Start Time	N Approach CHISHOLM AVE					E Approach DANFORTH AVE					W Approach DANFORTH AVE					Int. Total (15 min)
	Right	Left	UTurn	Peds	Approach Total	Right	Thru	UTurn	Peds	Approach Total	Thru	Left	UTurn	Peds	Approach Total	
08:15:00	12	3	0	26	15	8	148	0	0	156	116	5	0	1	121	292
08:30:00	7	2	0	24	9	10	176	0	0	186	113	3	0	1	116	311
08:45:00	5	2	0	35	7	14	140	0	0	154	117	6	0	3	123	284
09:00:00	5	4	0	40	9	9	148	0	2	157	121	3	1	1	125	291
Grand Total	29	11	0	125	40	41	612	0	2	653	467	17	1	6	485	1178
Approach%	72.5%	27.5%	0%	-	-	6.3%	93.7%	0%	-	-	96.3%	3.5%	0.2%	-	-	-
Totals %	2.5%	0.9%	0%	3.4%	3.5%	52%	0%	55.4%	39.6%	1.4%	0.1%	41.2%	-	-	-	-
PHF	0.6	0.69	0	0.67	0.73	0.87	0	0.88	0.96	0.71	0.25	0.97	-	-	-	-
Heavy	2	1	0	3	0	26	0	26	35	0	0	35	-	-	-	-
Heavy %	6.9%	9.1%	0%	7.5%	0%	4.2%	0%	4%	7.5%	0%	0%	7.2%	-	-	-	-
Lights	27	10	0	37	41	586	0	627	432	17	1	450	-	-	-	-
Lights %	93.1%	90.9%	0%	92.5%	100%	95.8%	0%	96%	92.5%	100%	100%	92.8%	-	-	-	-
Single-Unit Trucks	0	0	0	0	0	12	0	12	16	0	0	16	-	-	-	-
Single-Unit Trucks %	0%	0%	0%	0%	0%	2%	0%	1.8%	3.4%	0%	0%	3.3%	-	-	-	-
Buses	2	1	0	3	0	12	0	12	18	0	0	18	-	-	-	-
Buses %	6.9%	9.1%	0%	7.5%	0%	2%	0%	1.8%	3.9%	0%	0%	3.7%	-	-	-	-
Articulated Trucks	0	0	0	0	0	2	0	2	1	0	0	1	-	-	-	-
Articulated Trucks %	0%	0%	0%	0%	0%	0.3%	0%	0.3%	0.2%	0%	0%	0.2%	-	-	-	-
Pedestrians	-	-	-	124	-	-	-	2	-	-	-	6	-	-	-	-
Pedestrians%	-	-	-	93.2%	-	-	-	1.5%	-	-	-	4.5%	-	-	-	-
Bicycles on Road	5	0	0	0	-	1	66	0	0	-	29	0	0	0	-	-
Bicycles on Road%	-	-	-	0%	-	-	-	0%	-	-	-	0%	-	-	-	-
Bicycles on Crosswalk	-	-	-	1	-	-	-	0	-	-	-	0	-	-	-	-
Bicycles on Crosswalk%	-	-	-	0.8%	-	-	-	0%	-	-	-	0%	-	-	-	-



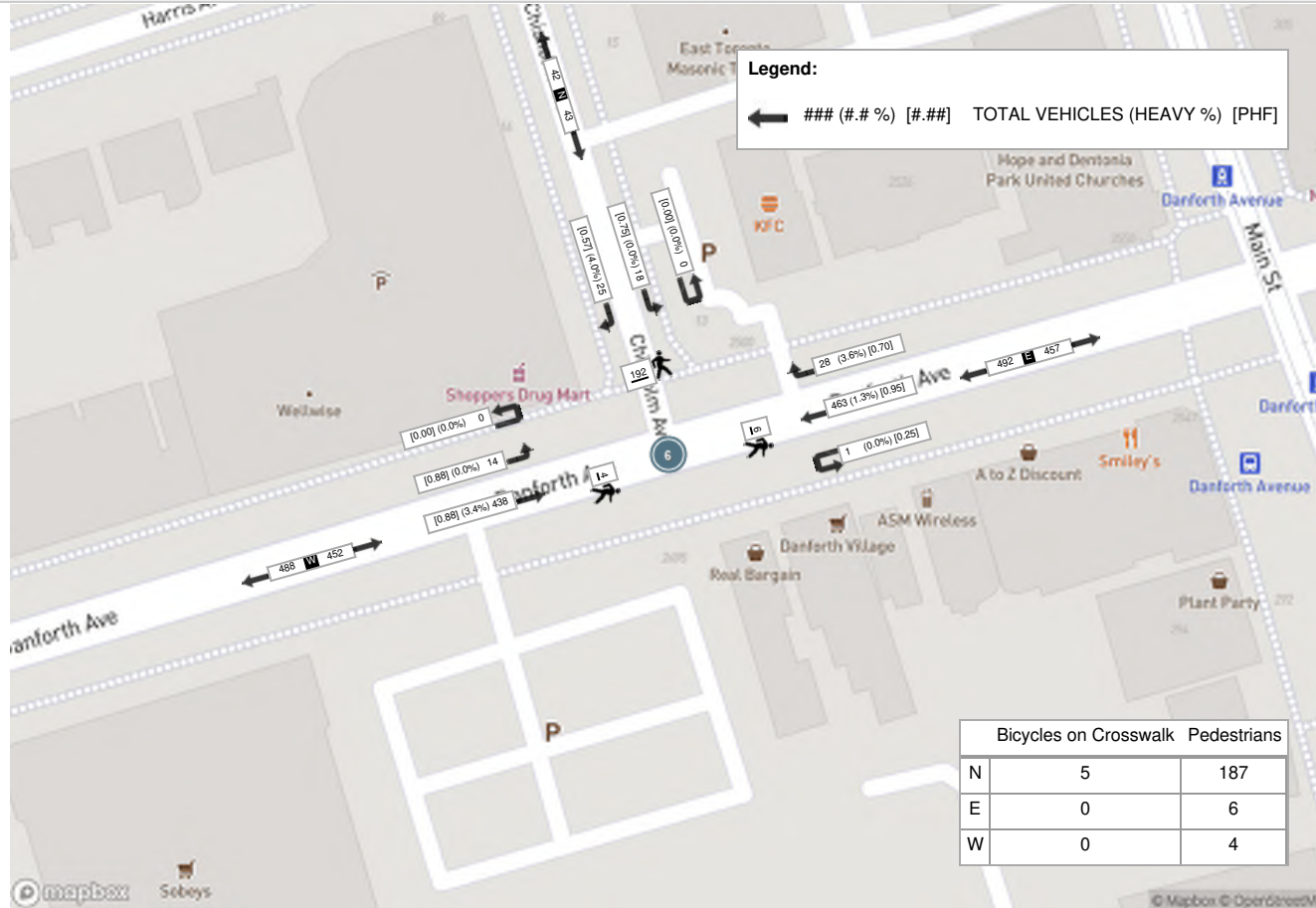
Peak Hour: 04:00 PM - 05:00 PM Weather: Clear Sky (19.95 °C)

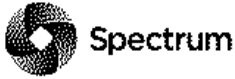
Start Time	N Approach CHISHOLM AVE					E Approach DANFORTH AVE					W Approach DANFORTH AVE					Int. Total (15 min)
	Right	Left	UTurn	Peds	Approach Total	Right	Thru	UTurn	Peds	Approach Total	Thru	Left	UTurn	Peds	Approach Total	
16:00:00	7	5	0	59	12	7	122	0	4	129	124	4	0	1	128	269
16:15:00	3	2	0	54	5	4	110	0	1	114	105	4	0	0	109	228
16:30:00	11	5	0	43	16	7	117	1	0	125	96	3	0	1	99	240
16:45:00	4	6	0	36	10	10	114	0	1	124	113	3	0	2	116	250
Grand Total	25	18	0	192	43	28	463	1	6	492	438	14	0	4	452	987
Approach%	58.1%	41.9%	0%	-	-	5.7%	94.1%	0.2%	-	-	96.9%	3.1%	0%	-	-	-
Totals %	2.5%	1.8%	0%	4.4%	4.4%	2.8%	46.9%	0.1%	49.8%	49.8%	44.4%	1.4%	0%	45.8%	45.8%	-
PHF	0.57	0.75	0	0.67	0.67	0.7	0.95	0.25	0.95	0.95	0.88	0.88	0	0.88	0.88	-
Heavy	1	0	0	1	1	1	6	0	7	7	15	0	0	15	15	-
Heavy %	4%	0%	0%	2.3%	2.3%	3.6%	1.3%	0%	1.4%	1.4%	3.4%	0%	0%	3.3%	3.3%	-
Lights	24	18	0	42	42	27	457	1	485	485	423	14	0	437	437	-
Lights %	96%	100%	0%	97.7%	97.7%	96.4%	98.7%	100%	98.6%	98.6%	96.6%	100%	0%	96.7%	96.7%	-
Single-Unit Trucks	1	0	0	1	1	0	2	0	2	2	9	0	0	9	9	-
Single-Unit Trucks %	4%	0%	0%	2.3%	2.3%	0%	0.4%	0%	0.4%	0.4%	2.1%	0%	0%	2%	2%	-
Buses	0	0	0	0	0	1	4	0	5	5	6	0	0	6	6	-
Buses %	0%	0%	0%	0%	0%	3.6%	0.9%	0%	1%	1%	1.4%	0%	0%	1.3%	1.3%	-
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	-
Articulated Trucks %	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	-
Pedestrians	-	-	-	187	-	-	-	-	6	-	-	-	-	4	-	-
Pedestrians%	-	-	-	92.6%	-	-	-	-	3%	-	-	-	-	2%	-	-
Bicycles on Road	2	1	0	0	-	0	54	0	0	-	77	0	0	0	-	-
Bicycles on Road%	-	-	-	0%	-	-	-	-	0%	-	-	-	-	0%	-	-
Bicycles on Crosswalk	-	-	-	5	-	-	-	-	0	-	-	-	-	0	-	-
Bicycles on Crosswalk%	-	-	-	2.5%	-	-	-	-	0%	-	-	-	-	0%	-	-

Peak Hour: 08:15 AM - 09:15 AM Weather: Clear Sky (13.64 °C)



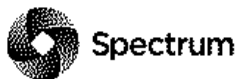
Peak Hour: 04:00 PM - 05:00 PM Weather: Clear Sky (19.95 °C)





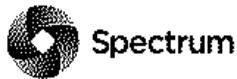
Turning Movement Count (1 . DANFORTH AVE & GLEDHILL AVE)

Start Time	N Approach GLEDHILL AVE						E Approach DANFORTH AVE						S Approach PATRICIA DR						W Approach DANFORTH AVE						Int. Total (15 min)	Int. Total (1 hr)	
	Right N-W	Thru N-S	Left N-E	UTurn N-N	Peds N:	Approach Total	Right E-N	Thru E-W	Left E-S	UTurn E-E	Peds E:	Approach Total	Right S-E	Thru S-N	Left S-W	UTurn S-S	Peds S:	Approach Total	Right W-S	Thru W-E	Left W-N	UTurn W-W	Peds W:	Approach Total			
07:30:00	9	2	3	0	15	14	6	167	0	0	0	173	3	3	10	0	11	16	3	61	3	0	10	67	270		
07:45:00	7	4	5	0	18	16	6	156	3	0	5	165	3	5	13	0	6	21	3	79	5	0	5	87	289		
08:00:00	7	2	3	0	19	12	5	137	3	0	3	145	6	2	17	0	6	25	3	90	4	0	13	97	279		
08:15:00	5	0	8	0	32	13	4	168	2	0	7	174	4	5	15	0	20	24	2	117	5	0	23	124	335	1173	
08:30:00	7	2	6	0	112	15	11	134	5	0	27	150	3	9	24	0	35	36	8	105	7	0	134	120	321	1224	
08:45:00	12	7	14	0	43	33	7	153	2	0	21	162	5	5	13	0	10	23	7	102	8	0	30	117	335	1270	
09:00:00	6	0	6	0	27	12	6	156	1	0	1	163	1	0	10	0	16	11	5	113	9	0	5	127	313	1304	
09:15:00	8	4	2	0	22	14	4	146	1	0	5	151	2	2	18	0	15	22	11	88	7	0	8	106	293	1262	
BREAK																											
16:00:00	11	6	4	0	42	21	4	105	4	0	8	113	2	0	13	0	29	15	9	110	7	0	13	126	275		
16:15:00	10	0	1	0	31	11	8	127	1	0	3	136	1	1	8	0	27	10	7	117	8	0	20	132	289		
16:30:00	6	3	2	0	40	11	3	111	5	0	8	119	1	5	19	0	25	25	10	102	13	0	10	125	280		
16:45:00	5	5	1	0	41	11	2	119	5	0	11	126	1	5	12	0	30	18	12	122	9	1	20	144	299	1143	
17:00:00	10	5	3	0	59	18	6	101	3	0	16	110	2	5	13	0	14	20	14	93	8	0	21	115	263	1131	
17:15:00	8	4	5	0	44	17	5	99	3	0	11	107	4	2	13	0	29	19	15	46	12	0	17	73	216	1058	
17:30:00	7	4	6	0	40	17	7	114	3	0	10	124	0	7	13	0	32	20	16	103	7	0	23	126	287	1065	
17:45:00	10	3	2	0	28	15	5	128	12	0	3	145	2	1	11	0	32	14	6	60	10	0	15	76	250	1016	
Grand Total	128	51	71	0	613	250	89	2121	53	0	139	2263	40	57	222	0	337	319	131	1508	122	1	367	1762	4594	-	
Approach%	51.2%	20.4%	28.4%	0%	-	-	3.9%	93.7%	2.3%	0%	-	-	12.5%	17.9%	69.6%	0%	-	-	7.4%	85.6%	6.9%	0.1%	-	-	-	-	
Totals %	2.8%	1.1%	1.5%	0%	5.4%	1.9%	46.2%	1.2%	0%	49.3%	0.9%	1.2%	4.8%	0%	6.9%	2.9%	32.8%	2.7%	0%	38.4%	-	-	-	-	-	-	
Heavy	5	2	4	0	-	1	57	1	0	-	3	5	3	0	-	8	65	3	0	-	-	-	-	-	-	-	
Heavy %	3.9%	3.9%	5.6%	0%	-	1.1%	2.7%	1.9%	0%	-	7.5%	8.8%	1.4%	0%	-	6.1%	4.3%	2.5%	0%	-	-	-	-	-	-	-	
Bicycles	9	6	7	0	-	3	227	3	0	-	2	6	14	0	-	7	138	4	1	-	-	-	-	-	-	-	
Bicycle %	7%	11.8%	9.9%	0%	-	3.4%	10.7%	5.7%	0%	-	5%	10.5%	6.3%	0%	-	5.3%	9.2%	3.3%	100%	-	-	-	-	-	-	-	



Peak Hour: 08:15 AM - 09:15 AM Weather: Overcast Clouds (15.49 °C)

Start Time	N Approach GLEDHILL AVE						E Approach DANFORTH AVE						S Approach PATRICIA DR						W Approach DANFORTH AVE						Int. Total (15 min)
	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	
08:15:00	5	0	8	0	32	13	4	168	2	0	7	174	4	5	15	0	20	24	2	117	5	0	23	124	335
08:30:00	7	2	6	0	112	15	11	134	5	0	27	150	3	9	24	0	35	36	8	105	7	0	134	120	321
08:45:00	12	7	14	0	43	33	7	153	2	0	21	162	5	5	13	0	10	23	7	102	8	0	30	117	335
09:00:00	6	0	6	0	27	12	6	156	1	0	1	163	1	0	10	0	16	11	5	113	9	0	5	127	313
Grand Total	30	9	34	0	214	73	28	611	10	0	56	649	13	19	62	0	81	94	22	437	29	0	192	488	1304
Approach%	41.1%	12.3%	46.6%	0%	-	-	4.3%	94.1%	1.5%	0%	-	-	13.8%	20.2%	66%	0%	-	-	4.5%	89.5%	5.9%	0%	-	-	-
Totals %	2.3%	0.7%	2.6%	0%	5.6%	2.1%	46.9%	0.8%	0%	49.8%	1%	1.5%	4.8%	0%	7.2%	1.7%	33.5%	2.2%	0%	37.4%	-	-	-	-	-
PHF	0.63	0.32	0.61	0	0.55	0.64	0.91	0.5	0	0.93	0.65	0.53	0.65	0	0.65	0.69	0.93	0.81	0	0.96	-	-	-	-	-
Heavy	3	2	2	0	7	1	18	1	0	20	2	2	2	0	6	3	30	2	0	35	-	-	-	-	-
Heavy %	10%	22.2%	5.9%	0%	9.6%	3.6%	2.9%	10%	0%	3.1%	15.4%	10.5%	3.2%	0%	6.4%	13.6%	6.9%	6.9%	0%	7.2%	-	-	-	-	-
Lights	27	7	32	0	66	27	593	9	0	629	11	17	60	0	88	19	407	27	0	453	-	-	-	-	-
Lights %	90%	77.8%	94.1%	0%	90.4%	96.4%	97.1%	90%	0%	96.9%	84.6%	89.5%	96.8%	0%	93.6%	86.4%	93.1%	93.1%	0%	92.8%	-	-	-	-	-
Single-Unit Trucks	1	0	1	0	2	1	10	0	0	11	0	0	1	0	1	0	14	1	0	15	-	-	-	-	-
Single-Unit Trucks %	3.3%	0%	2.9%	0%	2.7%	3.6%	1.6%	0%	0%	1.7%	0%	0%	1.6%	0%	1.1%	0%	3.2%	3.4%	0%	3.1%	-	-	-	-	-
Buses	2	2	1	0	5	0	8	1	0	9	2	2	1	0	5	3	16	1	0	20	-	-	-	-	-
Buses %	6.7%	22.2%	2.9%	0%	6.8%	0%	1.3%	10%	0%	1.4%	15.4%	10.5%	1.6%	0%	5.3%	13.6%	3.7%	3.4%	0%	4.1%	-	-	-	-	-
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	-	-	-	-	-
Articulated Trucks %	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	-	-	-	-	-
Pedestrians	-	-	-	-	214	-	-	-	-	54	-	-	-	81	-	-	-	-	-	189	-	-	-	-	-
Pedestrians%	-	-	-	-	39.4%	-	-	-	-	9.9%	-	-	-	14.9%	-	-	-	-	-	34.8%	-	-	-	-	-
Bicycles on Road	4	0	2	0	0	-	1	65	1	0	0	-	1	2	6	0	0	-	1	17	0	0	0	-	-
Bicycles on Road%	-	-	-	-	0%	-	-	-	-	0%	-	-	-	-	0%	-	-	-	-	-	-	-	0%	-	-
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	2	-	-	-	-	0	-	-	-	-	-	-	-	3	-	-
Bicycles on Crosswalk%	-	-	-	-	0%	-	-	-	-	0.4%	-	-	-	-	0%	-	-	-	-	-	-	-	0.6%	-	-



Peak Hour: 04:00 PM - 05:00 PM Weather: Light Rain (17.87 °C)

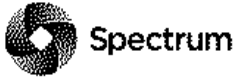
Start Time	N Approach GLEDHILL AVE						E Approach DANFORTH AVE						S Approach PATRICIA DR						W Approach DANFORTH AVE						Int. Total (15 min)
	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	
16:00:00	11	6	4	0	42	21	4	105	4	0	8	113	2	0	13	0	29	15	9	110	7	0	13	126	275
16:15:00	10	0	1	0	31	11	8	127	1	0	3	136	1	1	8	0	27	10	7	117	8	0	20	132	289
16:30:00	6	3	2	0	40	11	3	111	5	0	8	119	1	5	19	0	25	25	10	102	13	0	10	125	280
16:45:00	5	5	1	0	41	11	2	119	5	0	11	126	1	5	12	0	30	18	12	122	9	1	20	144	299
Grand Total	32	14	8	0	154	54	17	462	15	0	30	494	5	11	52	0	111	68	38	451	37	1	63	527	1143
Approach%	59.3%	25.9%	14.8%	0%	-	-	3.4%	93.5%	3%	0%	-	-	7.4%	16.2%	76.5%	0%	-	-	7.2%	85.6%	7%	0.2%	-	-	-
Totals %	2.8%	1.2%	0.7%	0%	4.7%	4.7%	1.5%	40.4%	1.3%	0%	43.2%	43.2%	0.4%	1%	4.5%	0%	5.9%	5.9%	3.3%	39.5%	3.2%	0.1%	46.1%	46.1%	-
PHF	0.73	0.58	0.5	0	0.64	0.64	0.53	0.91	0.75	0	0.91	0.91	0.63	0.55	0.68	0	0.68	0.68	0.79	0.92	0.71	0.25	0.91	0.91	-
Heavy	1	0	1	0	2	2	0	8	0	0	8	8	1	2	1	0	4	4	1	9	1	0	11	11	-
Heavy %	3.1%	0%	12.5%	0%	3.7%	3.7%	0%	1.7%	0%	0%	1.6%	1.6%	20%	18.2%	1.9%	0%	5.9%	5.9%	2.6%	2%	2.7%	0%	2.1%	2.1%	-
Lights	31	14	7	0	52	52	17	454	15	0	486	486	4	9	51	0	64	64	37	442	36	1	516	516	-
Lights %	96.9%	100%	87.5%	0%	96.3%	96.3%	100%	98.3%	100%	0%	98.4%	98.4%	80%	81.8%	98.1%	0%	94.1%	94.1%	97.4%	98%	97.3%	100%	97.9%	97.9%	-
Single-Unit Trucks	0	0	0	0	0	0	0	4	0	0	4	4	0	0	0	0	0	0	0	6	0	0	6	6	-
Single-Unit Trucks %	0%	0%	0%	0%	0%	0%	0%	0.9%	0%	0%	0.8%	0.8%	0%	0%	0%	0%	0%	0%	0%	1.3%	0%	0%	1.1%	1.1%	-
Buses	1	0	1	0	2	2	0	4	0	0	4	4	1	2	1	0	4	4	1	3	0	0	4	4	-
Buses %	3.1%	0%	12.5%	0%	3.7%	3.7%	0%	0.9%	0%	0%	0.8%	0.8%	20%	18.2%	1.9%	0%	5.9%	5.9%	2.6%	0.7%	0%	0%	0.8%	0.8%	-
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	-
Articulated Trucks %	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	2.7%	0%	0.2%	0.2%	-
Pedestrians	-	-	-	-	153	-	-	-	-	-	30	-	-	-	-	-	111	-	-	-	-	-	60	-	-
Pedestrians%	-	-	-	-	42.7%	-	-	-	-	-	8.4%	-	-	-	-	-	31%	-	-	-	-	-	16.8%	-	-
Bicycles on Road	0	4	0	0	0	-	0	44	0	0	0	-	0	0	1	0	0	-	3	49	0	1	0	-	-
Bicycles on Road%	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-
Bicycles on Crosswalk	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	3	-	-
Bicycles on Crosswalk%	-	-	-	-	0.3%	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	0.8%	-	-

Peak Hour: 08:15 AM - 09:15 AM Weather: Overcast Clouds (15.49 °C)



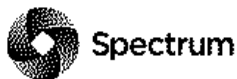
Peak Hour: 04:00 PM - 05:00 PM Weather: Light Rain (17.87 °C)





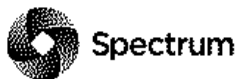
Turning Movement Count (7 . DANFORTH AVE & MAIN ST)

Start Time	N Approach MAIN ST						E Approach DANFORTH AVE						S Approach MAIN ST						W Approach DANFORTH AVE						Int. Total (15 min)	Int. Total (1 hr)	
	Right N:W	Thru N:S	Left N:E	UTurn N:N	Peds N:	Approach Total	Right E:N	Thru E:W	Left E:S	UTurn E:E	Peds E:	Approach Total	Right S:E	Thru S:N	Left S:W	UTurn S:S	Peds S:	Approach Total	Right W:S	Thru W:E	Left W:N	UTurn W:W	Peds W:	Approach Total			
07:30:00	6	33	5	0	29	44	15	128	35	0	127	178	17	54	14	0	39	85	21	45	3	0	23	69	376		
07:45:00	5	53	8	0	24	66	12	126	49	0	134	187	24	44	16	0	49	84	15	84	3	0	21	102	439		
08:00:00	4	55	11	0	35	70	13	111	59	0	167	183	37	56	18	0	32	111	26	107	2	0	21	135	499		
08:15:00	3	55	10	0	52	68	16	149	45	0	215	210	34	65	16	0	43	115	27	87	3	0	37	117	510	1824	
08:30:00	11	53	10	0	46	74	17	139	51	0	223	207	37	57	30	0	54	124	24	86	3	0	39	113	518	1966	
08:45:00	4	43	14	0	52	61	21	132	26	0	163	179	37	60	18	0	44	115	18	99	2	0	42	119	474	2001	
09:00:00	6	51	14	0	44	71	16	120	34	0	116	170	36	46	28	0	33	110	24	99	1	0	19	124	475	1977	
09:15:00	3	36	11	0	33	50	12	132	48	0	95	192	19	51	22	0	38	92	19	92	5	0	31	116	450	1917	
BREAK																											
16:00:00	4	64	15	0	67	83	21	103	43	0	172	167	49	50	21	0	57	120	17	100	8	0	62	125	495		
16:15:00	3	60	15	0	64	78	20	93	35	0	188	148	47	65	17	0	82	129	11	80	5	0	39	96	451		
16:30:00	9	53	7	0	62	69	17	94	40	0	178	151	50	44	26	0	57	120	24	84	11	0	43	119	459		
16:45:00	6	69	15	0	58	90	16	93	36	0	151	145	54	65	24	0	64	143	24	93	3	0	68	120	498	1903	
17:00:00	4	65	14	0	92	83	18	88	46	0	231	152	34	70	20	0	90	124	21	55	2	0	68	78	437	1845	
17:15:00	9	74	14	0	96	97	20	90	53	0	232	163	42	74	20	0	119	136	20	80	2	0	71	102	498	1892	
17:30:00	0	56	12	0	102	68	19	110	41	0	215	170	39	70	20	0	73	129	12	48	2	0	40	62	429	1862	
17:45:00	3	59	8	0	90	70	18	93	37	0	155	148	45	76	24	0	89	145	21	78	2	0	73	101	464	1828	
Grand Total	80	879	183	0	946	1142	271	1801	678	0	2762	2750	601	947	334	0	963	1882	324	1317	57	0	697	1698	7472	-	
Approach%	7%	77%	16%	0%		-	9.9%	65.5%	24.7%	0%		-	31.9%	50.3%	17.7%	0%		-	19.1%	77.6%	3.4%	0%		-	-	-	
Totals %	1.1%	11.8%	2.4%	0%		15.3%	3.6%	24.1%	9.1%	0%		36.8%	8%	12.7%	4.5%	0%		25.2%	4.3%	17.6%	0.8%	0%		22.7%	-	-	
Heavy	3	64	65	0		-	66	59	19	0		-	19	71	11	0		-	5	74	4	0		-	-	-	
Heavy %	3.8%	7.3%	35.5%	0%		-	24.4%	3.3%	2.8%	0%		-	3.2%	7.5%	3.3%	0%		-	1.5%	5.6%	7%	0%		-	-	-	
Bicycles	8	25	3	0		-	19	227	3	0		-	8	41	25	0		-	52	156	2	0		-	-	-	
Bicycle %	10%	2.8%	1.6%	0%		-	7%	12.6%	0.4%	0%		-	1.3%	4.3%	7.5%	0%		-	16%	11.8%	3.5%	0%		-	-	-	



Peak Hour: 08:00 AM - 09:00 AM Weather: Clear Sky (13.64 °C)

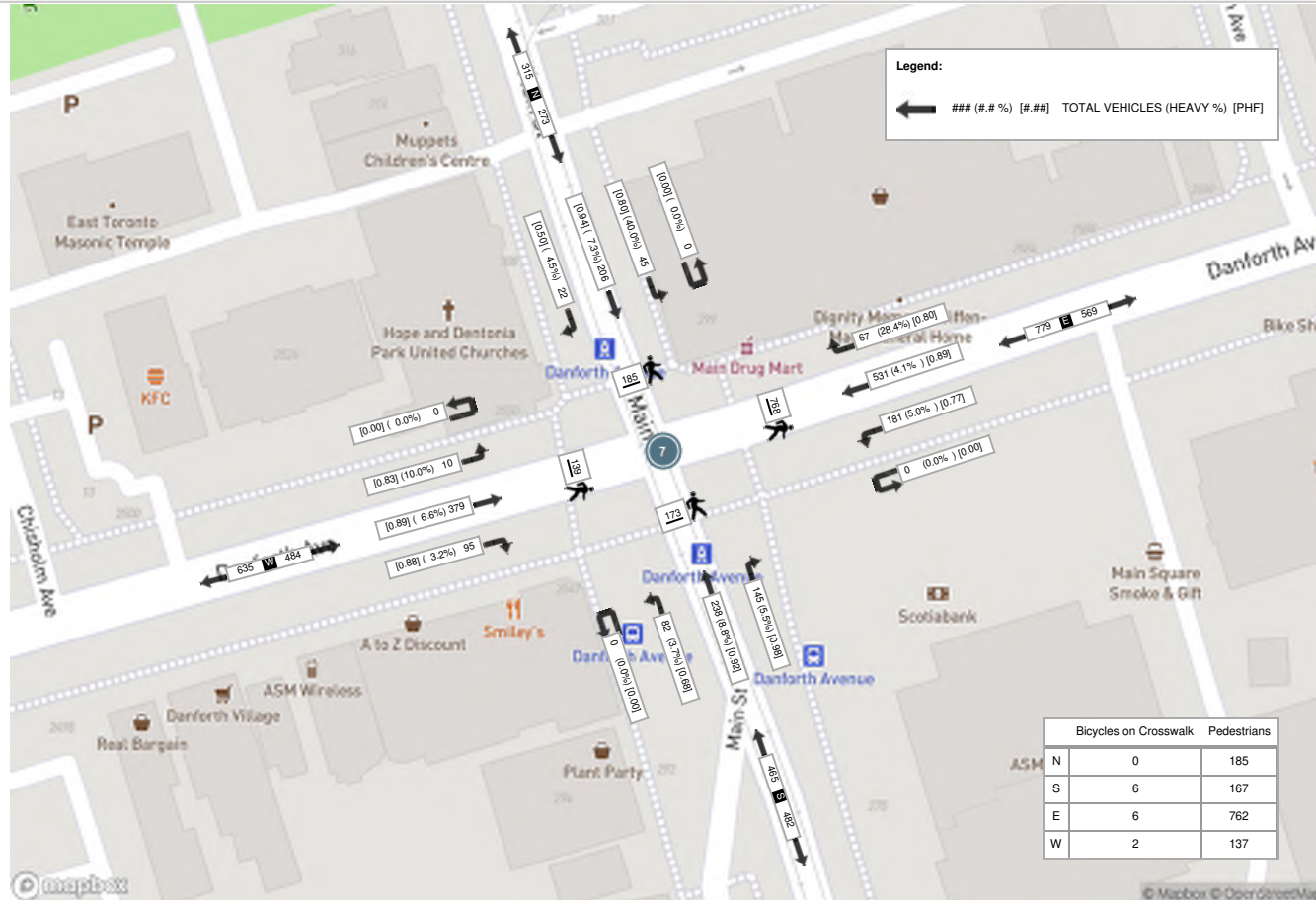
Start Time	N Approach MAIN ST						E Approach DANFORTH AVE						S Approach MAIN ST						W Approach DANFORTH AVE						Int. Total (15 min)
	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	
08:00:00	4	55	11	0	35	70	13	111	59	0	167	183	37	56	18	0	32	111	26	107	2	0	21	135	499
08:15:00	3	55	10	0	52	68	16	149	45	0	215	210	34	65	16	0	43	115	27	87	3	0	37	117	510
08:30:00	11	53	10	0	46	74	17	139	51	0	223	207	37	57	30	0	54	124	24	86	3	0	39	113	518
08:45:00	4	43	14	0	52	61	21	132	26	0	163	179	37	60	18	0	44	115	18	99	2	0	42	119	474
Grand Total	22	206	45	0	185	273	67	531	181	0	768	779	145	238	82	0	173	465	95	379	10	0	139	484	2001
Approach%	8.1%	75.5%	16.5%	0%		-	8.6%	68.2%	23.2%	0%		-	31.2%	51.2%	17.6%	0%		-	19.6%	78.3%	2.1%	0%		-	-
Totals %	1.1%	10.3%	2.2%	0%		13.6%	3.3%	26.5%	9%	0%		38.9%	7.2%	11.9%	4.1%	0%		23.2%	4.7%	18.9%	0.5%	0%		24.2%	-
PHF	0.5	0.94	0.8	0		0.92	0.8	0.89	0.77	0		0.93	0.98	0.92	0.68	0		0.94	0.88	0.89	0.83	0		0.9	-
Heavy	1	15	18	0		34	19	22	9	0		50	8	21	3	0		32	3	25	1	0		29	-
Heavy %	4.5%	7.3%	40%	0%		12.5%	28.4%	4.1%	5%	0%		6.4%	5.5%	8.8%	3.7%	0%		6.9%	3.2%	6.6%	10%	0%		6%	-
Lights	21	191	27	0		239	48	509	172	0		729	137	217	79	0		433	92	354	9	0		455	-
Lights %	95.5%	92.7%	60%	0%		87.5%	71.6%	95.9%	95%	0%		93.6%	94.5%	91.2%	96.3%	0%		93.1%	96.8%	93.4%	90%	0%		94%	-
Single-Unit Trucks	0	0	1	0		1	2	13	5	0		20	2	1	1	0		4	0	14	1	0		15	-
Single-Unit Trucks %	0%	0%	2.2%	0%		0.4%	3%	2.4%	2.8%	0%		2.6%	1.4%	0.4%	1.2%	0%		0.9%	0%	3.7%	10%	0%		3.1%	-
Buses	1	9	17	0		27	17	8	4	0		29	5	14	2	0		21	3	10	0	0		13	-
Buses %	4.5%	4.4%	37.8%	0%		9.9%	25.4%	1.5%	2.2%	0%		3.7%	3.4%	5.9%	2.4%	0%		4.5%	3.2%	2.6%	0%	0%		2.7%	-
Street-Car	0	6	0	0		6	0	0	0	0		0	0	6	0	0		6	0	0	0	0		0	-
Street-Car %	0%	2.9%	0%	0%		2.2%	0%	0%	0%	0%		0%	0%	2.5%	0%	0%		1.3%	0%	0%	0%	0%		0%	-
Articulated Trucks	0	0	0	0		0	0	1	0	0		1	1	0	0	0		1	0	1	0	0		1	-
Articulated Trucks %	0%	0%	0%	0%		0%	0%	0.2%	0%	0%		0.1%	0.7%	0%	0%	0%		0.2%	0%	0.3%	0%	0%		0.2%	-
Pedestrians	-	-	-	-	185	-	-	-	-	-	762	-	-	-	-	-	167	-	-	-	-	-	137	-	-
Pedestrians%	-	-	-	-	14.6%	-	-	-	-	-	60.2%	-	-	-	-	-	13.2%	-	-	-	-	-	10.8%	-	-
Bicycles on Road	1	5	0	0	0	-	1	63	1	0	0	-	1	15	6	0	0	-	10	17	0	0	0	-	-
Bicycles on Road%	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	6	-	-	-	-	-	6	-	-	-	-	-	2	-	-
Bicycles on Crosswalk%	-	-	-	-	0%	-	-	-	-	-	0.5%	-	-	-	-	-	0.5%	-	-	-	-	-	0.2%	-	-



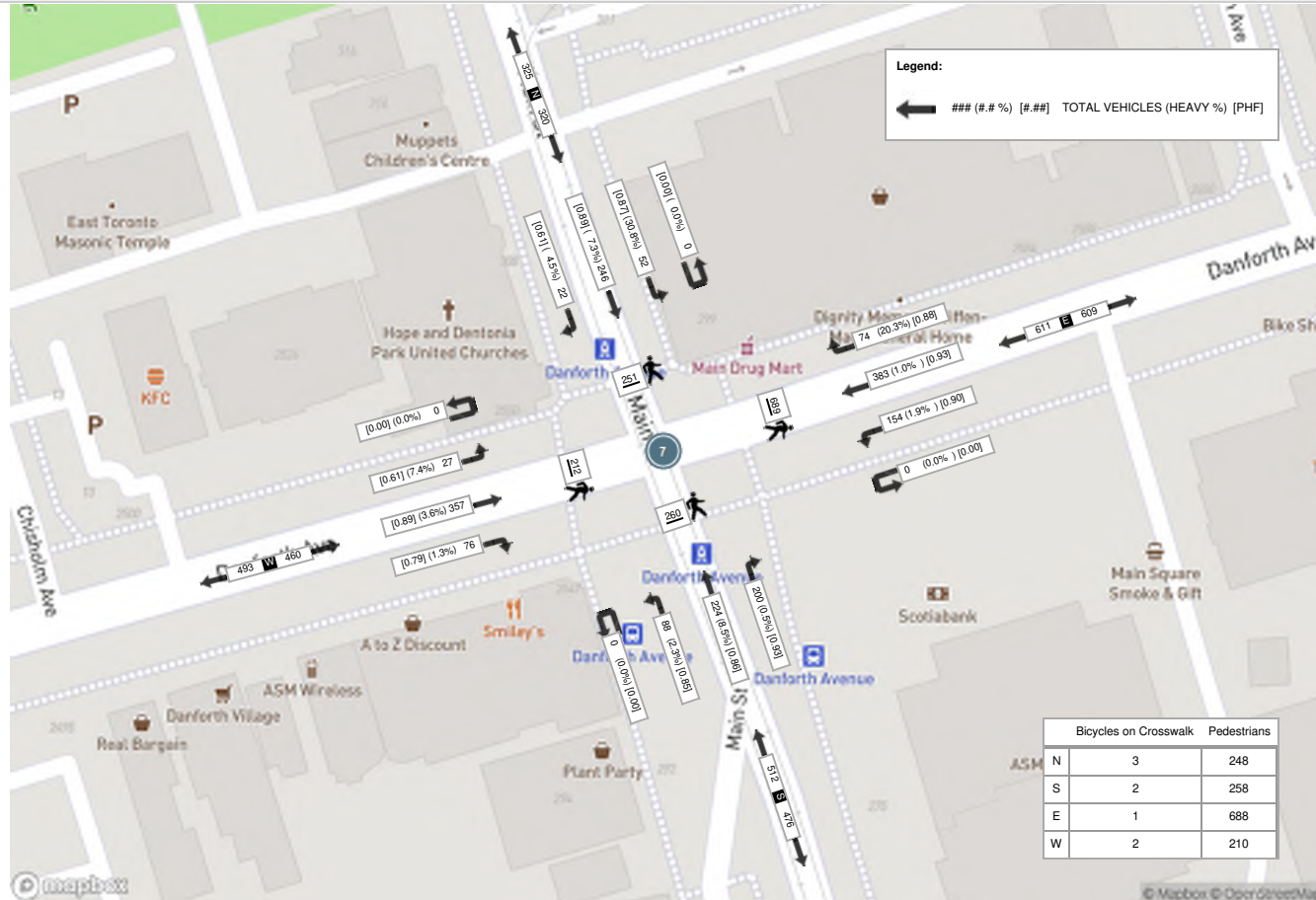
Peak Hour: 04:00 PM - 05:00 PM Weather: Clear Sky (19.95 °C)

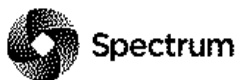
Start Time	N Approach MAIN ST						E Approach DANFORTH AVE						S Approach MAIN ST						W Approach DANFORTH AVE						Int. Total (15 min)
	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	
16:00:00	4	64	15	0	67	83	21	103	43	0	172	167	49	50	21	0	57	120	17	100	8	0	62	125	495
16:15:00	3	60	15	0	64	78	20	93	35	0	188	148	47	65	17	0	82	129	11	80	5	0	39	96	451
16:30:00	9	53	7	0	62	69	17	94	40	0	178	151	50	44	26	0	57	120	24	84	11	0	43	119	459
16:45:00	6	69	15	0	58	90	16	93	36	0	151	145	54	65	24	0	64	143	24	93	3	0	68	120	498
Grand Total	22	246	52	0	251	320	74	383	154	0	689	611	200	224	88	0	260	512	76	357	27	0	212	460	1903
Approach%	6.9%	76.9%	16.3%	0%		-	12.1%	62.7%	25.2%	0%		-	39.1%	43.8%	17.2%	0%		-	16.5%	77.6%	5.9%	0%		-	-
Totals %	1.2%	12.9%	2.7%	0%		16.8%	3.9%	20.1%	8.1%	0%		32.1%	10.5%	11.8%	4.6%	0%		26.9%	4%	18.8%	1.4%	0%		24.2%	-
PHF	0.61	0.89	0.87	0		0.89	0.88	0.93	0.9	0		0.91	0.93	0.86	0.85	0		0.9	0.79	0.89	0.61	0		0.92	-
Heavy	1	18	16	0		35	15	4	3	0		22	1	19	2	0		22	1	13	2	0		16	-
Heavy %	4.5%	7.3%	30.8%	0%		10.9%	20.3%	1%	1.9%	0%		3.6%	0.5%	8.5%	2.3%	0%		4.3%	1.3%	3.6%	7.4%	0%		3.5%	-
Lights	21	228	36	0		285	59	379	151	0		589	199	205	86	0		490	75	344	25	0		444	-
Lights %	95.5%	92.7%	69.2%	0%		89.1%	79.7%	99%	98.1%	0%		96.4%	99.5%	91.5%	97.7%	0%		95.7%	98.7%	96.4%	92.6%	0%		96.5%	-
Single-Unit Trucks	0	2	1	0		3	0	1	3	0		4	1	2	1	0		4	1	8	1	0		10	-
Single-Unit Trucks %	0%	0.8%	1.9%	0%		0.9%	0%	0.3%	1.9%	0%		0.7%	0.5%	0.9%	1.1%	0%		0.8%	1.3%	2.2%	3.7%	0%		2.2%	-
Buses	1	12	15	0		28	15	3	0	0		18	0	14	1	0		15	0	5	1	0		6	-
Buses %	4.5%	4.9%	28.8%	0%		8.8%	20.3%	0.8%	0%	0%		2.9%	0%	6.3%	1.1%	0%		2.9%	0%	1.4%	3.7%	0%		1.3%	-
Street-Car	0	4	0	0		4	0	0	0	0		0	0	3	0	0		3	0	0	0	0		0	-
Street-Car %	0%	1.6%	0%	0%		1.3%	0%	0%	0%	0%		0%	0%	1.3%	0%	0%		0.6%	0%	0%	0%	0%		0%	-
Articulated Trucks	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	-
Articulated Trucks %	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	-	248	-	-	-	-	-	688	-	-	-	-	-	258	-	-	-	-	-	210	-	-
Pedestrians%	-	-	-	-	17.6%	-	-	-	-	-	48.7%	-	-	-	-	-	18.3%	-	-	-	-	-	14.9%	-	-
Bicycles on Road	4	6	1	0	0	-	5	48	1	0	0	-	0	12	4	0	0	-	15	65	1	0	0	-	-
Bicycles on Road%	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-
Bicycles on Crosswalk	-	-	-	-	3	-	-	-	-	-	1	-	-	-	-	-	2	-	-	-	-	-	2	-	-
Bicycles on Crosswalk%	-	-	-	-	0.2%	-	-	-	-	-	0.1%	-	-	-	-	-	0.1%	-	-	-	-	-	0.1%	-	-

Peak Hour: 08:00 AM - 09:00 AM Weather: Clear Sky (13.64 °C)



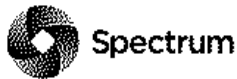
Peak Hour: 04:00 PM - 05:00 PM Weather: Clear Sky (19.95 °C)





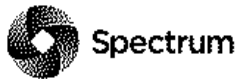
Turning Movement Count (2 . DANFORTH AVE & MORTON RD)

Start Time	E Approach DANFORTH AVE					S Approach MORTON RD					W Approach DANFORTH AVE					Int. Total (15 min)	Int. Total (1 hr)
	Thru E:W	Left E:S	UTurn E:E	Peds E:	Approach Total	Right S:E	Left S:W	UTurn S:S	Peds S:	Approach Total	Right W:S	Thru W:E	UTurn W:W	Peds W:	Approach Total		
07:30:00	137	0	0	0	137	0	2	0	15	2	0	73	0	0	73	212	
07:45:00	146	1	0	1	147	1	0	0	19	1	1	92	1	1	94	242	
08:00:00	135	2	0	0	137	6	1	0	11	7	3	121	0	2	124	268	
08:15:00	141	1	0	0	142	0	1	0	17	1	2	120	0	3	122	265	987
08:30:00	160	7	0	0	167	5	1	0	34	6	2	111	0	1	113	286	1061
08:45:00	153	4	0	0	157	4	2	0	28	6	2	131	0	0	133	296	1115
09:00:00	142	1	0	2	143	5	1	0	20	6	0	104	0	0	104	253	1100
09:15:00	146	1	0	1	147	3	2	0	19	5	1	118	0	0	119	271	1106
BREAK																	
16:00:00	132	1	0	2	133	3	3	0	51	6	5	118	2	4	125	264	
16:15:00	118	3	0	1	121	2	1	0	20	3	5	114	0	2	119	243	
16:30:00	115	6	0	3	121	3	3	0	23	6	2	116	0	0	118	245	
16:45:00	125	0	0	0	125	1	3	0	28	4	2	120	0	1	122	251	1003
17:00:00	115	4	0	0	119	1	5	0	29	6	7	89	0	2	96	221	960
17:15:00	109	2	0	1	111	0	2	0	40	2	1	99	0	1	100	213	930
17:30:00	111	4	1	3	116	1	2	0	31	3	2	71	0	1	73	192	877
17:45:00	113	2	0	3	115	2	2	0	23	4	2	106	0	4	108	227	853
Grand Total	2098	39	1	17	2138	37	31	0	408	68	37	1703	3	22	1743	3949	-
Approach%	98.1%	1.8%	0%		-	54.4%	45.6%	0%		-	2.1%	97.7%	0.2%		-	-	-
Totals %	53.1%	1%	0%		54.1%	0.9%	0.8%	0%		1.7%	0.9%	43.1%	0.1%		44.1%	-	-
Heavy	62	3	0		-	2	1	0		-	0	79	0		-	-	-
Heavy %	3%	7.7%	0%		-	5.4%	3.2%	0%		-	0%	4.6%	0%		-	-	-
Bicycles	288	0	0		-	2	1	0		-	2	222	0		-	-	-
Bicycle %	13.7%	0%	0%		-	5.4%	3.2%	0%		-	5.4%	13%	0%		-	-	-



Peak Hour: 08:00 AM - 09:00 AM Weather: Clear Sky (13.64 °C)

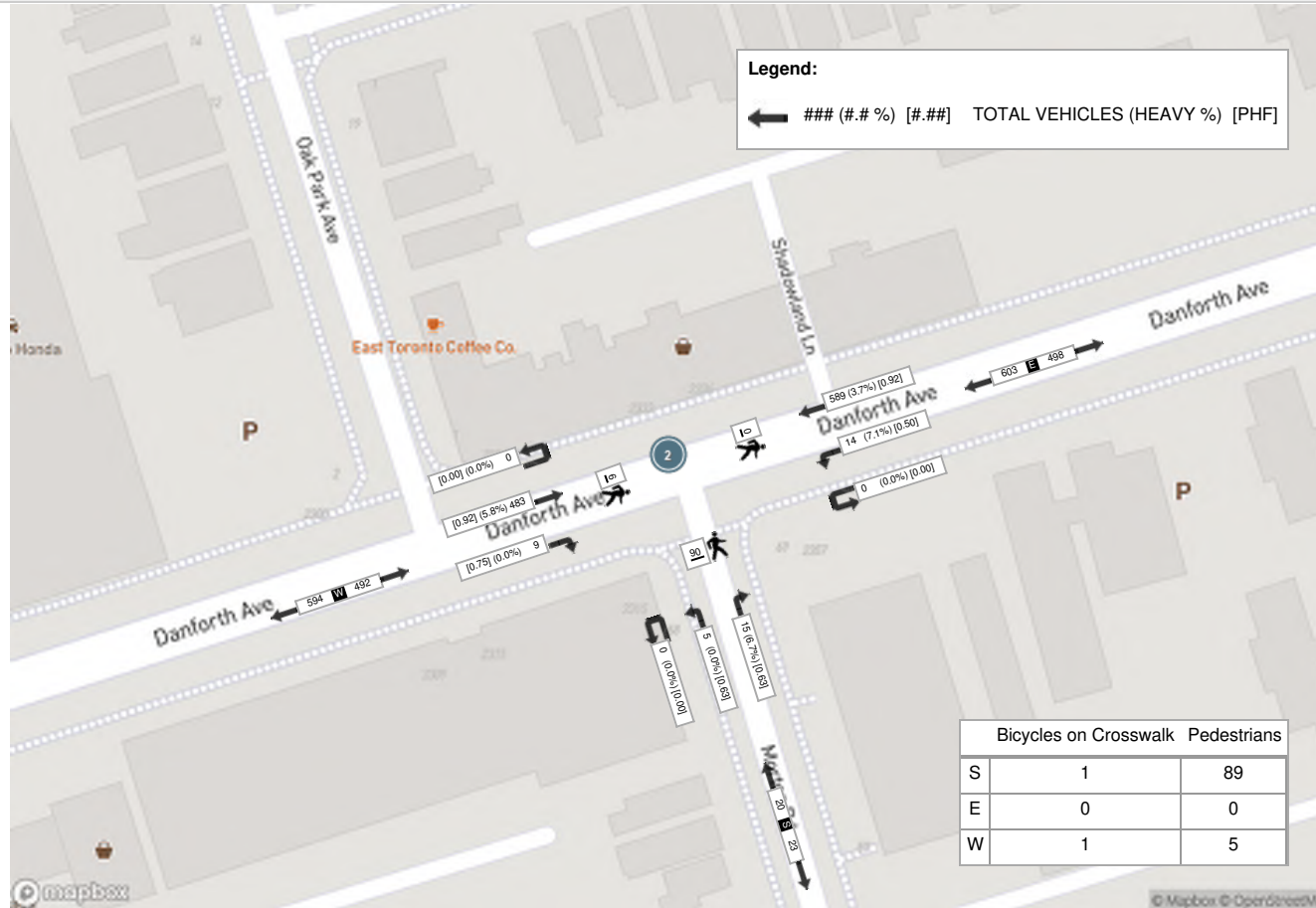
Start Time	E Approach DANFORTH AVE					S Approach MORTON RD					W Approach DANFORTH AVE					Int. Total (15 min)
	Thru	Left	UTurn	Peds	Approach Total	Right	Left	UTurn	Peds	Approach Total	Right	Thru	UTurn	Peds	Approach Total	
08:00:00	135	2	0	0	137	6	1	0	11	7	3	121	0	2	124	268
08:15:00	141	1	0	0	142	0	1	0	17	1	2	120	0	3	122	265
08:30:00	160	7	0	0	167	5	1	0	34	6	2	111	0	1	113	286
08:45:00	153	4	0	0	157	4	2	0	28	6	2	131	0	0	133	296
Grand Total	589	14	0	0	603	15	5	0	90	20	9	483	0	6	492	1115
Approach%	97.7%	2.3%	0%		-	75%	25%	0%		-	1.8%	98.2%	0%		-	-
Totals %	52.8%	1.3%	0%		54.1%	1.3%	0.4%	0%		1.8%	0.8%	43.3%	0%		44.1%	-
PHF	0.92	0.5	0		0.9	0.63	0.63	0		0.71	0.75	0.92	0		0.92	-
Heavy	22	1	0		23	1	0	0		1	0	28	0		28	-
Heavy %	3.7%	7.1%	0%		3.8%	6.7%	0%	0%		5%	0%	5.8%	0%		5.7%	-
Lights	567	13	0		580	14	5	0		19	9	455	0		464	-
Lights %	96.3%	92.9%	0%		96.2%	93.3%	100%	0%		95%	100%	94.2%	0%		94.3%	-
Single-Unit Trucks	11	0	0		11	0	0	0		0	0	14	0		14	-
Single-Unit Trucks %	1.9%	0%	0%		1.8%	0%	0%	0%		0%	0%	2.9%	0%		2.8%	-
Buses	10	1	0		11	1	0	0		1	0	12	0		12	-
Buses %	1.7%	7.1%	0%		1.8%	6.7%	0%	0%		5%	0%	2.5%	0%		2.4%	-
Articulated Trucks	1	0	0		1	0	0	0		0	0	2	0		2	-
Articulated Trucks %	0.2%	0%	0%		0.2%	0%	0%	0%		0%	0%	0.4%	0%		0.4%	-
Pedestrians	-	-	-	0	-	-	-	-	89	-	-	-	-	5	-	-
Pedestrians%	-	-	-	0%	-	-	-	-	92.7%	-	-	-	-	5.2%	-	-
Bicycles on Road	84	0	0	0	-	0	1	0	0	-	0	32	0	0	-	-
Bicycles on Road%	-	-	-	0%	-	-	-	-	0%	-	-	-	-	0%	-	-
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	1	-	-	-	-	1	-	-
Bicycles on Crosswalk%	-	-	-	0%	-	-	-	-	1%	-	-	-	-	1%	-	-



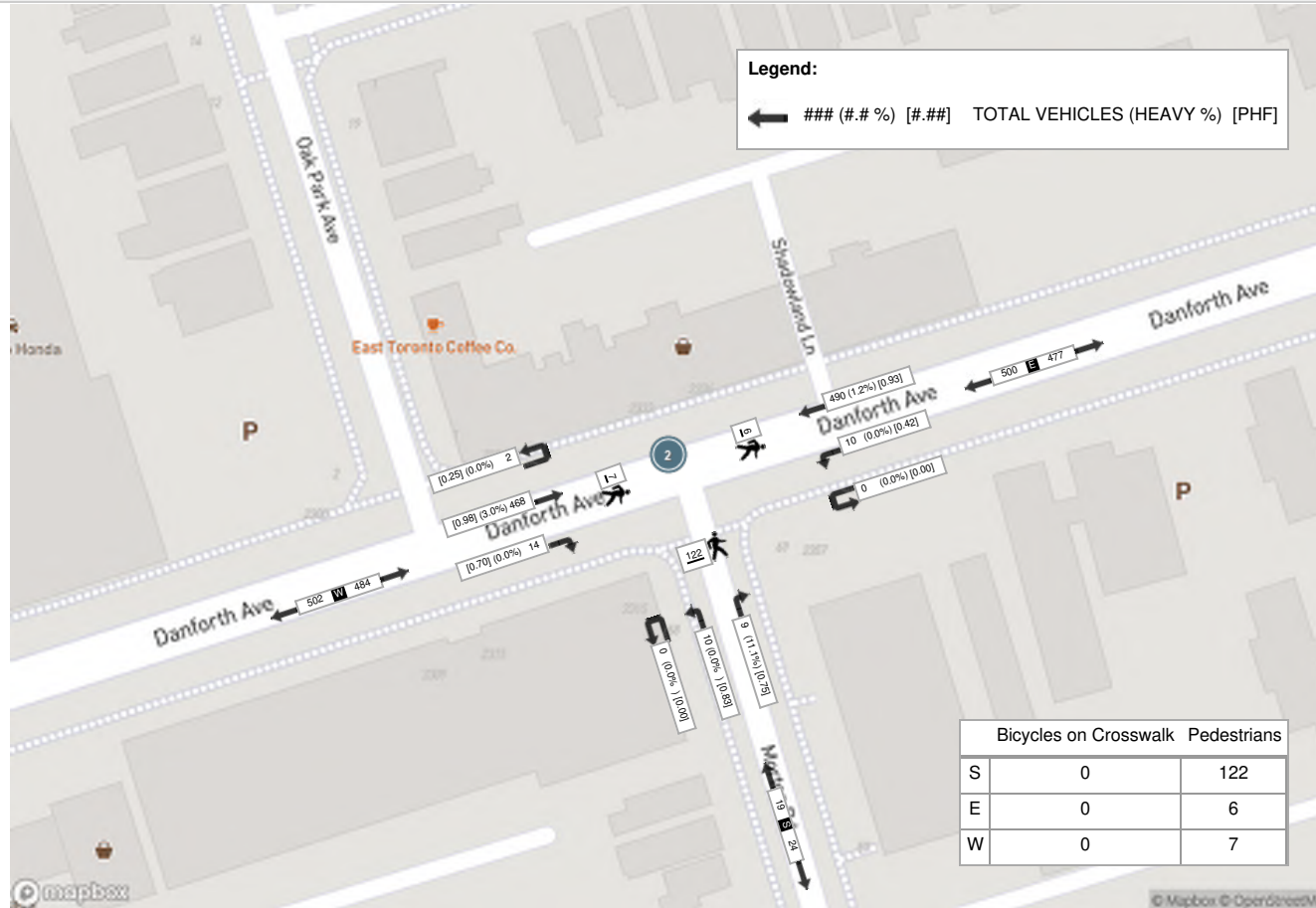
Peak Hour: 04:00 PM - 05:00 PM Weather: Clear Sky (19.95 °C)

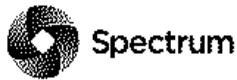
Start Time	E Approach DANFORTH AVE					S Approach MORTON RD					W Approach DANFORTH AVE					Int. Total (15 min)
	Thru	Left	UTurn	Peds	Approach Total	Right	Left	UTurn	Peds	Approach Total	Right	Thru	UTurn	Peds	Approach Total	
16:00:00	132	1	0	2	133	3	3	0	51	6	5	118	2	4	125	264
16:15:00	118	3	0	1	121	2	1	0	20	3	5	114	0	2	119	243
16:30:00	115	6	0	3	121	3	3	0	23	6	2	116	0	0	118	245
16:45:00	125	0	0	0	125	1	3	0	28	4	2	120	0	1	122	251
Grand Total	490	10	0	6	500	9	10	0	122	19	14	468	2	7	484	1003
Approach%	98%	2%	0%		-	47.4%	52.6%	0%		-	2.9%	96.7%	0.4%		-	-
Totals %	48.9%	1%	0%		49.9%	0.9%	1%	0%		1.9%	1.4%	46.7%	0.2%		48.3%	-
PHF	0.93	0.42	0		0.94	0.75	0.83	0		0.79	0.7	0.98	0.25		0.97	-
Heavy	6	0	0		6	1	0	0		1	0	14	0		14	-
Heavy %	1.2%	0%	0%		1.2%	11.1%	0%	0%		5.3%	0%	3%	0%		2.9%	-
Lights	484	10	0		494	8	10	0		18	14	454	2		470	-
Lights %	98.8%	100%	0%		98.8%	88.9%	100%	0%		94.7%	100%	97%	100%		97.1%	-
Single-Unit Trucks	2	0	0		2	1	0	0		1	0	8	0		8	-
Single-Unit Trucks %	0.4%	0%	0%		0.4%	11.1%	0%	0%		5.3%	0%	1.7%	0%		1.7%	-
Buses	4	0	0		4	0	0	0		0	0	6	0		6	-
Buses %	0.8%	0%	0%		0.8%	0%	0%	0%		0%	0%	1.3%	0%		1.2%	-
Articulated Trucks	0	0	0		0	0	0	0		0	0	0	0		0	-
Articulated Trucks %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	6	-	-	-	-	122	-	-	-	-	7	-	-
Pedestrians%	-	-	-	4.4%	-	-	-	-	90.4%	-	-	-	-	5.2%	-	-
Bicycles on Road	65	0	0	0	-	2	0	0	0	-	0	83	0	0	-	-
Bicycles on Road%	-	-	-	0%	-	-	-	-	0%	-	-	-	-	0%	-	-
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
Bicycles on Crosswalk%	-	-	-	0%	-	-	-	-	0%	-	-	-	-	0%	-	-

Peak Hour: 08:00 AM - 09:00 AM Weather: Clear Sky (13.64 °C)



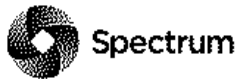
Peak Hour: 04:00 PM - 05:00 PM Weather: Clear Sky (19.95 °C)





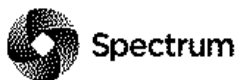
Turning Movement Count (1 . DANFORTH AVE & OAK PARK AVE)

Start Time	N Approach OAK PARK AVE					E Approach DANFORTH AVE					W Approach DANFORTH AVE					Int. Total (15 min)	Int. Total (1 hr)
	Right N:W	Left N:E	UTurn N:N	Peds N:	Approach Total	Right E:N	Thru E:W	UTurn E:E	Peds E:	Approach Total	Thru W:E	Left W:N	UTurn W:W	Peds W:	Approach Total		
07:30:00	8	0	0	14	8	3	135	0	1	138	72	4	0	0	76	222	
07:45:00	5	0	0	23	5	2	146	0	0	148	92	5	1	0	98	251	
08:00:00	11	0	0	21	11	1	135	0	2	136	127	10	0	0	137	284	
08:15:00	16	0	0	33	16	2	140	0	1	142	122	5	0	0	127	285	1042
08:30:00	5	0	0	65	5	4	156	0	0	160	113	9	0	0	122	287	1107
08:45:00	17	0	0	34	17	6	145	0	0	151	132	9	0	1	141	309	1165
09:00:00	12	1	0	15	13	3	142	0	0	145	103	4	0	0	107	265	1146
09:15:00	5	0	0	34	5	2	141	1	1	144	118	6	0	0	124	273	1134
BREAK																	
16:00:00	7	0	0	29	7	6	135	0	1	141	124	14	0	0	138	286	
16:15:00	8	0	0	21	8	8	114	0	4	122	117	13	0	1	130	260	
16:30:00	8	1	0	28	9	10	107	0	0	117	120	10	1	1	131	257	
16:45:00	16	1	0	29	17	8	122	0	4	130	118	22	0	2	140	287	1090
17:00:00	10	3	0	31	13	5	115	0	0	120	96	18	0	1	114	247	1051
17:15:00	9	1	0	34	10	6	103	0	0	109	98	17	0	0	115	234	1025
17:30:00	10	0	0	44	10	5	113	0	0	118	74	15	0	3	89	217	985
17:45:00	11	1	0	39	12	5	116	0	1	121	102	14	0	0	116	249	947
Grand Total	158	8	0	494	166	76	2065	1	15	2142	1728	175	2	9	1905	4213	-
Approach%	95.2%	4.8%	0%	-	-	3.5%	96.4%	0%	-	-	90.7%	9.2%	0.1%	-	-	-	-
Totals %	3.8%	0.2%	0%	3.9%	1.8%	49%	0%	50.8%	41%	4.2%	0%	45.2%	-	-	-	-	-
Heavy	3	0	0	-	1	63	0	-	86	4	0	-	-	-	-	-	-
Heavy %	1.9%	0%	0%	-	1.3%	3.1%	0%	-	5%	2.3%	0%	-	-	-	-	-	-
Bicycles	36	3	0	-	3	270	0	-	223	8	0	-	-	-	-	-	-
Bicycle %	22.8%	37.5%	0%	-	3.9%	13.1%	0%	-	12.9%	4.6%	0%	-	-	-	-	-	-



Peak Hour: 08:00 AM - 09:00 AM Weather: Clear Sky (13.64 °C)

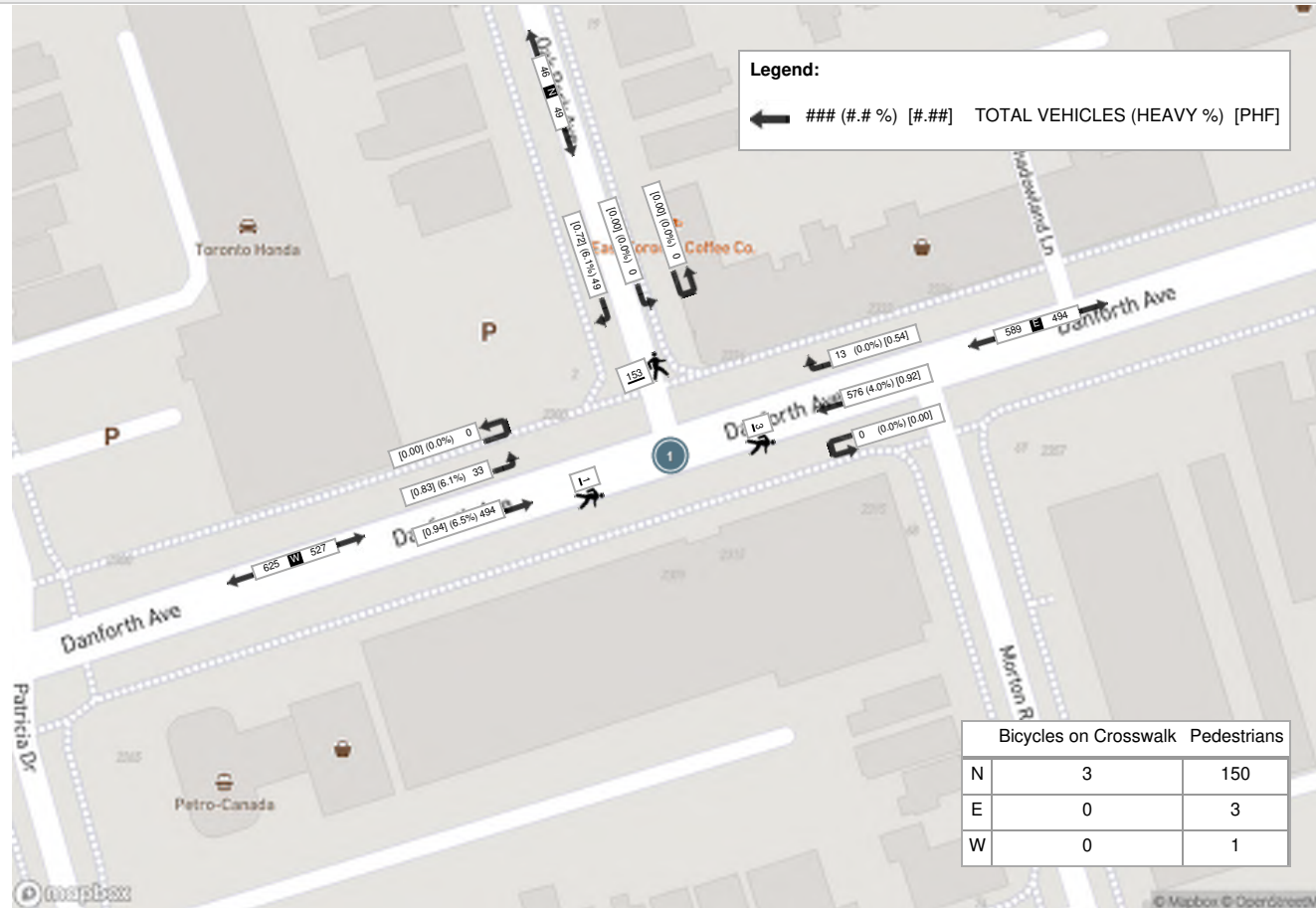
Start Time	N Approach OAK PARK AVE					E Approach DANFORTH AVE					W Approach DANFORTH AVE					Int. Total (15 min)
	Right	Left	UTurn	Peds	Approach Total	Right	Thru	UTurn	Peds	Approach Total	Thru	Left	UTurn	Peds	Approach Total	
08:00:00	11	0	0	21	11	1	135	0	2	136	127	10	0	0	137	284
08:15:00	16	0	0	33	16	2	140	0	1	142	122	5	0	0	127	285
08:30:00	5	0	0	65	5	4	156	0	0	160	113	9	0	0	122	287
08:45:00	17	0	0	34	17	6	145	0	0	151	132	9	0	1	141	309
Grand Total	49	0	0	153	49	13	576	0	3	589	494	33	0	1	527	1165
Approach%	100%	0%	0%	-	-	2.2%	97.8%	0%	-	-	93.7%	6.3%	0%	-	-	-
Totals %	4.2%	0%	0%	4.2%	4.2%	1.1%	49.4%	0%	50.6%	42.4%	2.8%	0%	45.2%	-	-	-
PHF	0.72	0	0	0.72	0.72	0.54	0.92	0	0.92	0.94	0.83	0	0.93	-	-	-
Heavy	3	0	0	3	3	0	23	0	23	32	2	0	34	-	-	-
Heavy %	6.1%	0%	0%	6.1%	6.1%	0%	4%	0%	3.9%	6.5%	6.1%	0%	6.5%	-	-	-
Lights	46	0	0	46	46	13	553	0	566	462	31	0	493	-	-	-
Lights %	93.9%	0%	0%	93.9%	93.9%	100%	96%	0%	96.1%	93.5%	93.9%	0%	93.5%	-	-	-
Single-Unit Trucks	1	0	0	1	1	0	12	0	12	16	0	0	16	-	-	-
Single-Unit Trucks %	2%	0%	0%	2%	2%	0%	2.1%	0%	2%	3.2%	0%	0%	3%	-	-	-
Buses	2	0	0	2	2	0	10	0	10	14	2	0	16	-	-	-
Buses %	4.1%	0%	0%	4.1%	4.1%	0%	1.7%	0%	1.7%	2.8%	6.1%	0%	3%	-	-	-
Articulated Trucks	0	0	0	0	0	0	1	0	1	2	0	0	2	-	-	-
Articulated Trucks %	0%	0%	0%	0%	0%	0%	0.2%	0%	0.2%	0.4%	0%	0%	0.4%	-	-	-
Pedestrians	-	-	-	150	-	-	-	-	3	-	-	-	-	1	-	-
Pedestrians%	-	-	-	95.5%	-	-	-	-	1.9%	-	-	-	-	0.6%	-	-
Bicycles on Road	17	1	0	0	-	0	81	0	0	-	28	1	0	0	-	-
Bicycles on Road%	-	-	-	0%	-	-	-	-	0%	-	-	-	-	0%	-	-
Bicycles on Crosswalk	-	-	-	3	-	-	-	-	0	-	-	-	-	0	-	-
Bicycles on Crosswalk%	-	-	-	1.9%	-	-	-	-	0%	-	-	-	-	0%	-	-



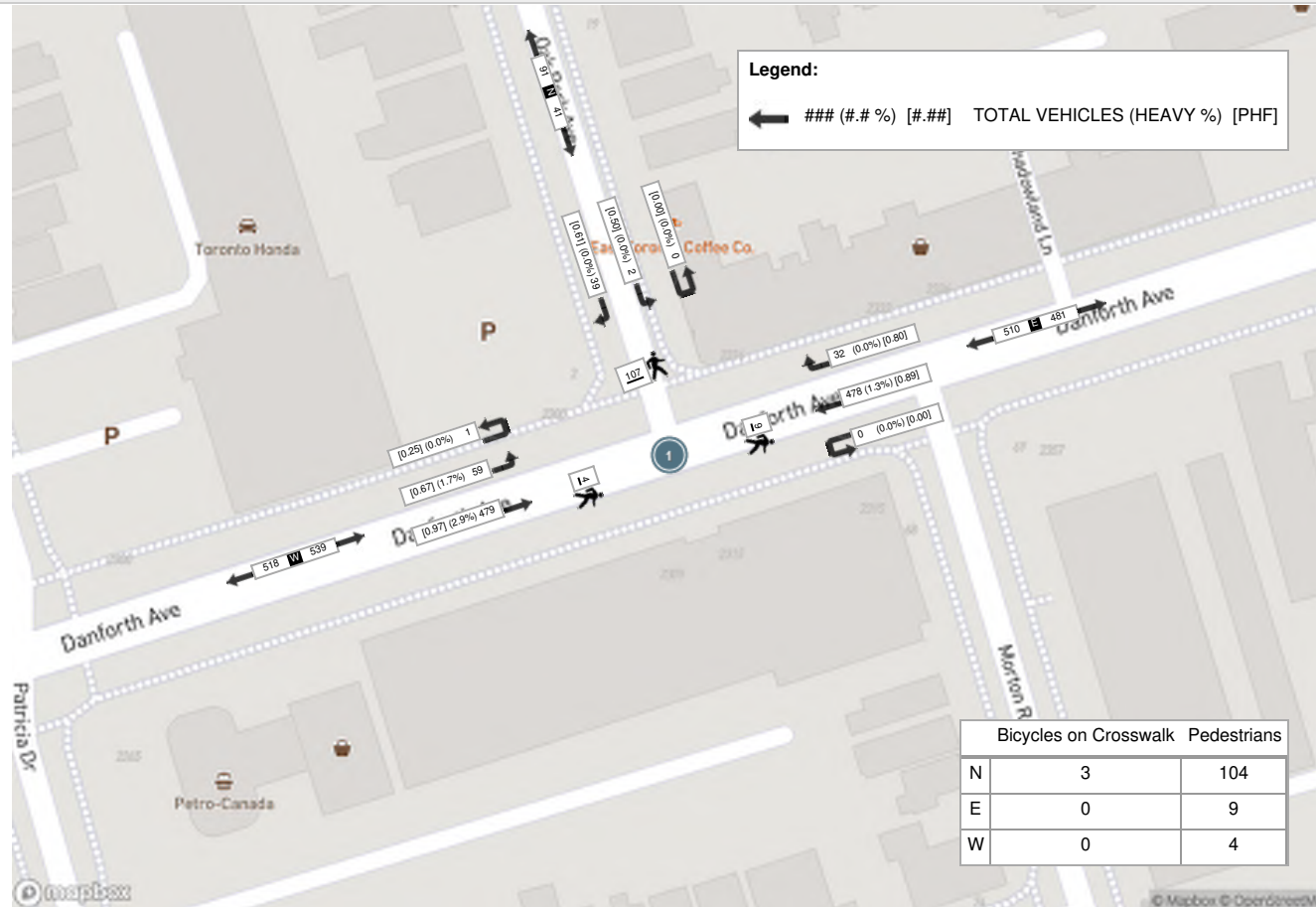
Peak Hour: 04:00 PM - 05:00 PM Weather: Clear Sky (19.95 °C)

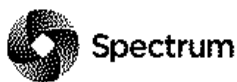
Start Time	N Approach OAK PARK AVE					E Approach DANFORTH AVE					W Approach DANFORTH AVE					Int. Total (15 min)
	Right	Left	UTurn	Peds	Approach Total	Right	Thru	UTurn	Peds	Approach Total	Thru	Left	UTurn	Peds	Approach Total	
16:00:00	7	0	0	29	7	6	135	0	1	141	124	14	0	0	138	286
16:15:00	8	0	0	21	8	8	114	0	4	122	117	13	0	1	130	260
16:30:00	8	1	0	28	9	10	107	0	0	117	120	10	1	1	131	257
16:45:00	16	1	0	29	17	8	122	0	4	130	118	22	0	2	140	287
Grand Total	39	2	0	107	41	32	478	0	9	510	479	59	1	4	539	1090
Approach%	95.1%	4.9%	0%	-	-	6.3%	93.7%	0%	-	-	88.9%	10.9%	0.2%	-	-	-
Totals %	3.6%	0.2%	0%	3.8%	3.8%	2.9%	43.9%	0%	46.8%	46.8%	43.9%	5.4%	0.1%	49.4%	49.4%	-
PHF	0.61	0.5	0	0.6	0.6	0.8	0.89	0	0.9	0.9	0.97	0.67	0.25	0.96	0.96	-
Heavy	0	0	0	0	0	0	6	0	6	6	14	1	0	15	15	-
Heavy %	0%	0%	0%	0%	0%	0%	1.3%	0%	1.2%	1.2%	2.9%	1.7%	0%	2.8%	2.8%	-
Lights	39	2	0	41	41	32	472	0	504	504	465	58	1	524	524	-
Lights %	100%	100%	0%	100%	100%	100%	98.7%	0%	98.8%	98.8%	97.1%	98.3%	100%	97.2%	97.2%	-
Single-Unit Trucks	0	0	0	0	0	0	2	0	2	2	8	0	0	8	8	-
Single-Unit Trucks %	0%	0%	0%	0%	0%	0%	0.4%	0%	0.4%	0.4%	1.7%	0%	0%	1.5%	1.5%	-
Buses	0	0	0	0	0	0	4	0	4	4	6	1	0	7	7	-
Buses %	0%	0%	0%	0%	0%	0%	0.8%	0%	0.8%	0.8%	1.3%	1.7%	0%	1.3%	1.3%	-
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	-
Articulated Trucks %	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	-
Pedestrians	-	-	-	104	-	-	-	-	9	-	-	-	-	4	-	-
Pedestrians%	-	-	-	86.7%	-	-	-	-	7.5%	-	-	-	-	3.3%	-	-
Bicycles on Road	2	1	0	0	-	1	55	0	0	-	85	2	0	0	-	-
Bicycles on Road%	-	-	-	0%	-	-	-	-	0%	-	-	-	-	0%	-	-
Bicycles on Crosswalk	-	-	-	3	-	-	-	-	0	-	-	-	-	0	-	-
Bicycles on Crosswalk%	-	-	-	2.5%	-	-	-	-	0%	-	-	-	-	0%	-	-

Peak Hour: 08:00 AM - 09:00 AM Weather: Clear Sky (13.64 °C)



Peak Hour: 04:00 PM - 05:00 PM Weather: Clear Sky (19.95 °C)





Turning Movement Count (3 . DANFORTH AVE & WESTLAKE AVE / COMMERCIAL DRIVEWAY)

Start Time	N Approach WESTLAKE AVE							E Approach DANFORTH AVE							S Approach WESTLAKE AVE							Hard Right Westlake Ave
	Right N:W	Thru N:S	Bear Left N:SW	Left N:E	UTurn N:N	Peds N:	Approach Total	Right E:N	Thru E:W	Bear Left E:SW	Left E:S	UTurn E:E	Peds E:	Approach Total	Right S:E	Thru S:N	Left S:W	Hard Left S:SW	UTurn S:S	Peds S:	Approach Total	
07:30:00	5	0	0	0	0	12	5	6	140	0	1	0	3	147	3	1	1	0	0	19	5	0
07:45:00	3	3	0	6	0	13	12	1	135	0	1	0	5	137	4	0	2	0	0	17	6	0
08:00:00	5	1	0	5	0	19	11	4	124	0	1	0	4	129	3	1	5	0	0	12	9	0
08:15:00	6	0	0	4	0	21	10	3	140	0	2	0	5	145	2	4	3	0	0	18	9	0
08:30:00	9	1	0	5	0	27	15	4	158	0	7	0	6	169	1	2	3	0	0	35	6	0
08:45:00	6	1	0	6	0	23	13	7	136	0	3	0	13	146	3	5	2	0	0	22	10	0
09:00:00	3	3	0	3	0	14	9	2	136	0	4	0	4	142	3	0	1	0	0	12	4	0
09:15:00	4	3	0	6	0	26	13	2	142	0	2	0	6	146	1	1	3	0	0	22	5	0
BREAK																						
16:00:00	7	2	0	4	0	35	13	4	121	0	3	0	14	128	3	2	5	0	0	46	10	0
16:15:00	10	0	1	3	0	22	14	8	103	0	3	0	22	114	1	1	4	0	0	31	6	0
16:30:00	7	2	0	2	0	25	11	6	107	0	2	0	19	115	4	4	4	0	0	25	12	0
16:45:00	5	1	0	4	0	31	10	4	112	0	1	0	15	117	2	2	7	0	0	29	11	1
17:00:00	2	0	0	7	0	34	9	5	104	0	4	0	17	113	4	1	4	0	0	28	9	0
17:15:00	11	4	0	6	0	45	21	8	96	0	5	0	22	109	4	1	2	0	0	44	7	0
17:30:00	4	3	0	5	0	44	12	8	102	0	3	0	17	113	3	7	5	0	0	32	15	0
17:45:00	6	2	0	6	0	37	14	1	106	0	1	0	11	108	0	1	2	0	0	26	3	0
Grand Total	93	26	1	72	0	428	192	73	1962	0	43	0	183	2078	41	33	53	0	0	418	127	1
Approach%	48.4%	13.5%	0.5%	37.5%	0%		-	3.5%	94.4%	0%	2.1%	0%		-	32.3%	26%	41.7%	0%	0%		-	0.1%
Totals %	2.3%	0.6%	0%	1.8%	0%		4.7%	1.8%	47.8%	0%	1%	0%		50.6%	1%	0.8%	1.3%	0%	0%		3.1%	0%
Heavy	3	1	0	1	0		-	3	62	0	3	0		-	0	0	1	0	0		-	0
Heavy %	3.2%	3.8%	0%	1.4%	0%		-	4.1%	3.2%	0%	7%	0%		-	0%	0%	1.9%	0%	0%		-	0%
Bicycles	11	0	0	2	0		-	7	254	0	1	0		-	2	1	1	0	0		-	3
Bicycle %	11.8%	0%	0%	2.8%	0%		-	9.6%	12.9%	0%	2.3%	0%		-	4.9%	3%	1.9%	0%	0%		-	300%



Peak Hour: 08:00 AM - 09:00 AM Weather: Clear Sky (13.64 °C)

Start Time	N Approach WESTLAKE AVE							E Approach DANFORTH AVE							S Approach WESTLAKE AVE						
	Right	Thru	Bear Left	Left	UTurn	Peds	Approach Total	Right	Thru	Bear Left	Left	UTurn	Peds	Approach Total	Right	Thru	Left	Hard Left	UTurn	Peds	Approach Total
08:00:00	5	1	0	5	0	19	11	4	124	0	1	0	4	129	3	1	5	0	0	12	9
08:15:00	6	0	0	4	0	21	10	3	140	0	2	0	5	145	2	4	3	0	0	18	9
08:30:00	9	1	0	5	0	27	15	4	158	0	7	0	6	169	1	2	3	0	0	35	6
08:45:00	6	1	0	6	0	23	13	7	136	0	3	0	13	146	3	5	2	0	0	22	10
Grand Total	26	3	0	20	0	90	49	18	558	0	13	0	28	589	9	12	13	0	0	87	34
Approach%	53.1%	6.1%	0%	40.8%	0%		-	3.1%	94.7%	0%	2.2%	0%		-	26.5%	35.3%	38.2%	0%	0%		-
Totals %	2.2%	0.3%	0%	1.7%	0%		4.2%	1.5%	47.9%	0%	1.1%	0%		50.6%	0.8%	1%	1.1%	0%	0%		2.9%
PHF	0.72	0.75	0	0.83	0		0.82	0.64	0.88	0	0.46	0		0.87	0.75	0.6	0.65	0	0		0.85
Heavy	1	0	0	1	0		2	1	22	0	2	0		25	0	0	1	0	0		1
Heavy %	3.8%	0%	0%	5%	0%		4.1%	5.6%	3.9%	0%	15.4%	0%		4.2%	0%	0%	7.7%	0%	0%		2.9%
Lights	25	3	0	19	0		47	17	536	0	11	0		564	9	12	12	0	0		33
Lights %	96.2%	100%	0%	95%	0%		95.9%	94.4%	96.1%	0%	84.6%	0%		95.8%	100%	100%	92.3%	0%	0%		97.1%
Single-Unit Trucks	0	0	0	0	0		0	1	12	0	0	0		13	0	0	0	0	0		0
Single-Unit Trucks %	0%	0%	0%	0%	0%		0%	5.6%	2.2%	0%	0%	0%		2.2%	0%	0%	0%	0%	0%		0%
Buses	1	0	0	1	0		2	0	9	0	2	0		11	0	0	1	0	0		1
Buses %	3.8%	0%	0%	5%	0%		4.1%	0%	1.6%	0%	15.4%	0%		1.9%	0%	0%	7.7%	0%	0%		2.9%
Articulated Trucks	0	0	0	0	0		0	0	1	0	0	0		1	0	0	0	0	0		0
Articulated Trucks %	0%	0%	0%	0%	0%		0%	0%	0.2%	0%	0%	0%		0.2%	0%	0%	0%	0%	0%		0%
Pedestrians	-	-	-	-	-	87	-	-	-	-	-	-	28	-	-	-	-	-	-	86	-
Pedestrians%	-	-	-	-	-	35.8%	-	-	-	-	-	-	11.5%	-	-	-	-	-	-	35.4%	-
Bicycles on Road	8	0	0	0	0	0	-	1	71	0	0	0	0	-	0	0	0	0	0	0	-
Bicycles on Road%	-	-	-	-	-	0%	-	-	-	-	-	-	0%	-	-	-	-	-	-	0%	-
Bicycles on Crosswalk	-	-	-	-	-	3	-	-	-	-	-	-	0	-	-	-	-	-	-	1	-
Bicycles on Crosswalk%	-	-	-	-	-	1.2%	-	-	-	-	-	-	0%	-	-	-	-	-	-	0.4%	-



Peak Hour: 04:00 PM - 05:00 PM Weather: Clear Sky (19.95 °C)

Start Time	N Approach WESTLAKE AVE							E Approach DANFORTH AVE							S Approach WESTLAKE AVE						
	Right	Thru	Bear Left	Left	UTurn	Peds	Approach Total	Right	Thru	Bear Left	Left	UTurn	Peds	Approach Total	Right	Thru	Left	Hard Left	UTurn	Peds	Approach Total
16:00:00	7	2	0	4	0	35	13	4	121	0	3	0	14	128	3	2	5	0	0	46	10
16:15:00	10	0	1	3	0	22	14	8	103	0	3	0	22	114	1	1	4	0	0	31	6
16:30:00	7	2	0	2	0	25	11	6	107	0	2	0	19	115	4	4	4	0	0	25	12
16:45:00	5	1	0	4	0	31	10	4	112	0	1	0	15	117	2	2	7	0	0	29	11
Grand Total	29	5	1	13	0	113	48	22	443	0	9	0	70	474	10	9	20	0	0	131	39
Approach%	60.4%	10.4%	2.1%	27.1%	0%	-	-	4.6%	93.5%	0%	1.9%	0%	-	-	25.6%	23.1%	51.3%	0%	0%	-	-
Totals %	2.8%	0.5%	0.1%	1.3%	0%	4.7%	4.7%	2.1%	43%	0%	0.9%	0%	46%	46%	1%	0.9%	1.9%	0%	0%	3.8%	3.8%
PHF	0.73	0.63	0.25	0.81	0	0.86	0.86	0.69	0.92	0	0.75	0	0.93	0.93	0.63	0.56	0.71	0	0	0.81	0.81
Heavy	0	1	0	0	0	1	1	0	6	0	0	0	6	6	0	0	0	0	0	0	0
Heavy %	0%	20%	0%	0%	0%	2.1%	2.1%	0%	1.4%	0%	0%	0%	1.3%	1.3%	0%	0%	0%	0%	0%	0%	0%
Lights	29	4	1	13	0	47	47	22	437	0	9	0	468	468	10	9	20	0	0	39	39
Lights %	100%	80%	100%	100%	0%	97.9%	97.9%	100%	98.6%	0%	100%	0%	98.7%	98.7%	100%	100%	100%	0%	0%	100%	100%
Single-Unit Trucks	0	1	0	0	0	1	1	0	2	0	0	0	2	2	0	0	0	0	0	0	0
Single-Unit Trucks %	0%	20%	0%	0%	0%	2.1%	2.1%	0%	0.5%	0%	0%	0%	0.4%	0.4%	0%	0%	0%	0%	0%	0%	0%
Buses	0	0	0	0	0	0	0	0	4	0	0	0	4	4	0	0	0	0	0	0	0
Buses %	0%	0%	0%	0%	0%	0%	0%	0%	0.9%	0%	0%	0%	0.8%	0.8%	0%	0%	0%	0%	0%	0%	0%
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Articulated Trucks %	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Pedestrians	-	-	-	-	-	110	-	-	-	-	-	-	70	-	-	-	-	-	-	129	-
Pedestrians%	-	-	-	-	-	31.4%	-	-	-	-	-	-	20%	-	-	-	-	-	-	36.9%	-
Bicycles on Road	0	0	0	0	0	0	-	0	55	0	1	0	0	-	0	0	0	0	0	0	-
Bicycles on Road%	-	-	-	-	-	0%	-	-	-	-	-	-	0%	-	-	-	-	-	-	0%	-
Bicycles on Crosswalk	-	-	-	-	-	3	-	-	-	-	-	-	0	-	-	-	-	-	-	2	-
Bicycles on Crosswalk%	-	-	-	-	-	0.9%	-	-	-	-	-	-	0%	-	-	-	-	-	-	0.6%	-

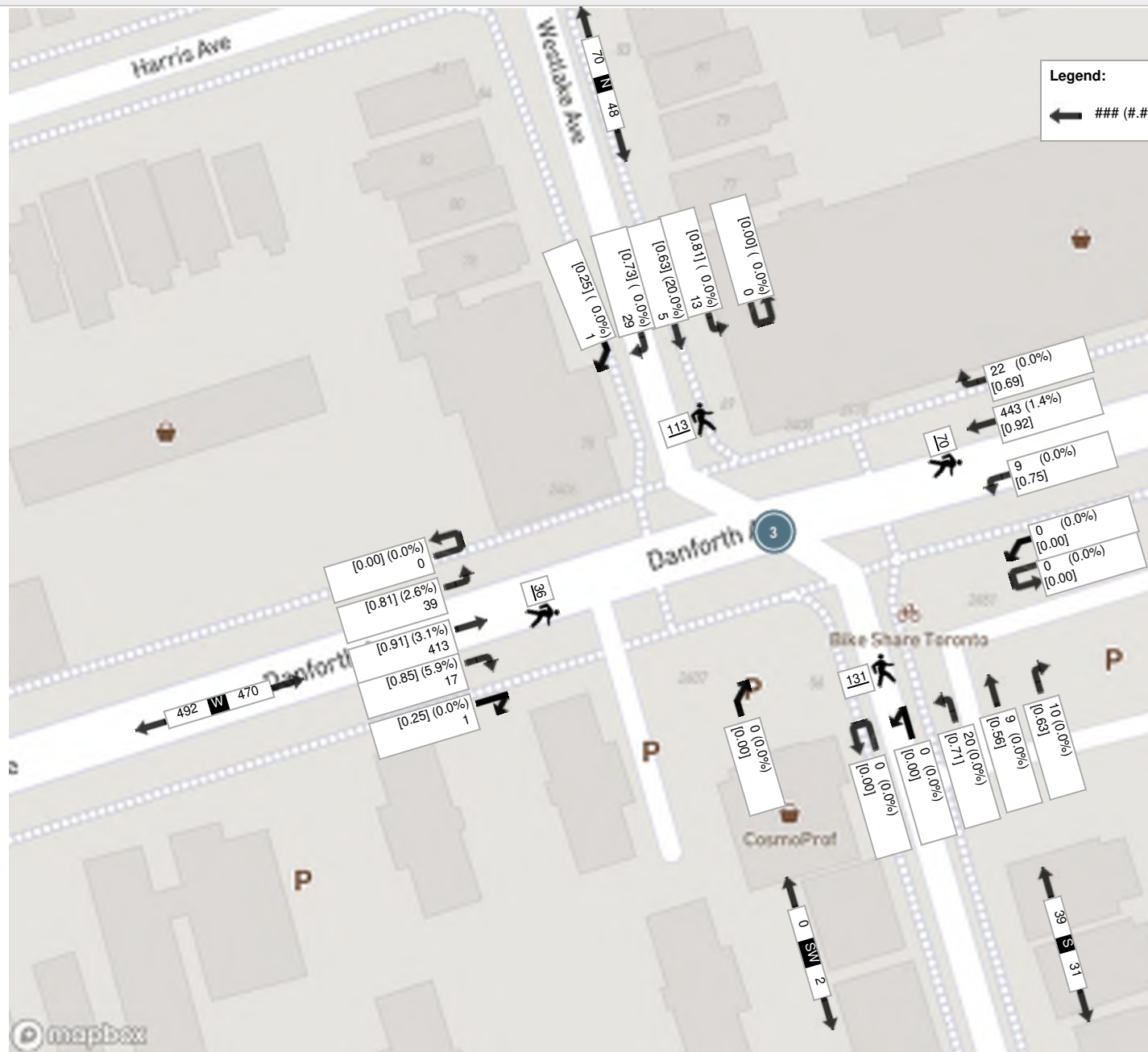
Peak Hour: 08:00 AM - 09:00 AM Weather: Clear Sky (13.64 °C)

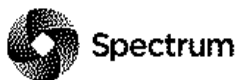




Turning Movement Count
Location Name: DANFORTH AVE & WESTLAKE AVE / COMMERCIAL DRIVEWAY
Date: Thu, May 16, 2024 Deployment Lead: David Chu

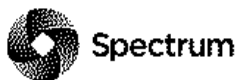
Peak Hour: 04:00 PM - 05:00 PM Weather: Clear Sky (19.95 °C)





Turning Movement Count (9 . WESTLAKE AVE & 2451 DANFORTH AVE)

Start Time	N Approach WESTLAKE AVE					E Approach 2451 DANFORTH AVE (SOBEYS ACCESS)					S Approach WESTLAKE AVE					Int. Total (15 min)	Int. Total (1 hr)
	Thru N:S	Left N:E	UTurn N:N	Peds N:	Approach Total	Right E:N	Left E:S	UTurn E:E	Peds E:	Approach Total	Right S:E	Thru S:N	UTurn S:S	Peds S:	Approach Total		
07:30:00	2	0	0	0	2	0	0	0	5	0	0	5	0	3	5	7	
07:45:00	4	1	0	0	5	0	0	0	3	0	0	6	0	1	6	11	
08:00:00	4	1	0	0	5	0	0	0	3	0	0	9	0	2	9	14	
08:15:00	4	0	0	1	4	0	0	0	3	0	1	10	0	1	11	15	47
08:30:00	8	2	0	0	10	1	1	0	7	2	0	5	0	2	5	17	57
08:45:00	6	2	0	1	8	2	0	0	7	2	0	4	0	2	4	14	60
09:00:00	5	4	0	0	9	1	0	0	6	1	0	3	0	1	3	13	59
09:15:00	7	0	0	0	7	1	1	0	1	2	0	4	0	0	4	13	57
BREAK																	
16:00:00	7	3	0	5	10	1	0	0	7	1	2	5	0	0	7	18	
16:15:00	6	3	0	2	9	3	1	0	7	4	2	4	0	2	6	19	
16:30:00	8	2	0	0	10	4	2	0	6	6	1	7	0	3	8	24	
16:45:00	2	1	0	0	3	3	1	0	6	4	0	9	0	3	9	16	77
17:00:00	8	0	0	1	8	2	0	0	6	2	2	7	0	0	9	19	78
17:15:00	7	5	0	3	12	1	3	0	14	4	2	7	0	1	9	25	84
17:30:00	8	4	0	2	12	3	1	0	8	4	1	9	0	2	10	26	86
17:45:00	4	3	0	1	7	1	2	0	6	3	1	2	0	0	3	13	83
Grand Total	90	31	0	16	121	23	12	0	95	35	12	96	0	23	108	264	-
Approach%	74.4%	25.6%	0%	-	-	65.7%	34.3%	0%	-	-	11.1%	88.9%	0%	-	-	-	-
Totals %	34.1%	11.7%	0%	-	45.8%	8.7%	4.5%	0%	-	13.3%	4.5%	36.4%	0%	-	40.9%	-	-
Heavy	4	2	0	-	-	0	0	0	-	-	0	1	0	-	-	-	-
Heavy %	4.4%	6.5%	0%	-	-	0%	0%	0%	-	-	0%	1%	0%	-	-	-	-
Bicycles	6	3	0	-	-	3	1	0	-	-	3	5	0	-	-	-	-
Bicycle %	6.7%	9.7%	0%	-	-	13%	8.3%	0%	-	-	25%	5.2%	0%	-	-	-	-



Peak Hour: 08:00 AM - 09:00 AM Weather: Clear Sky (13.64 °C)

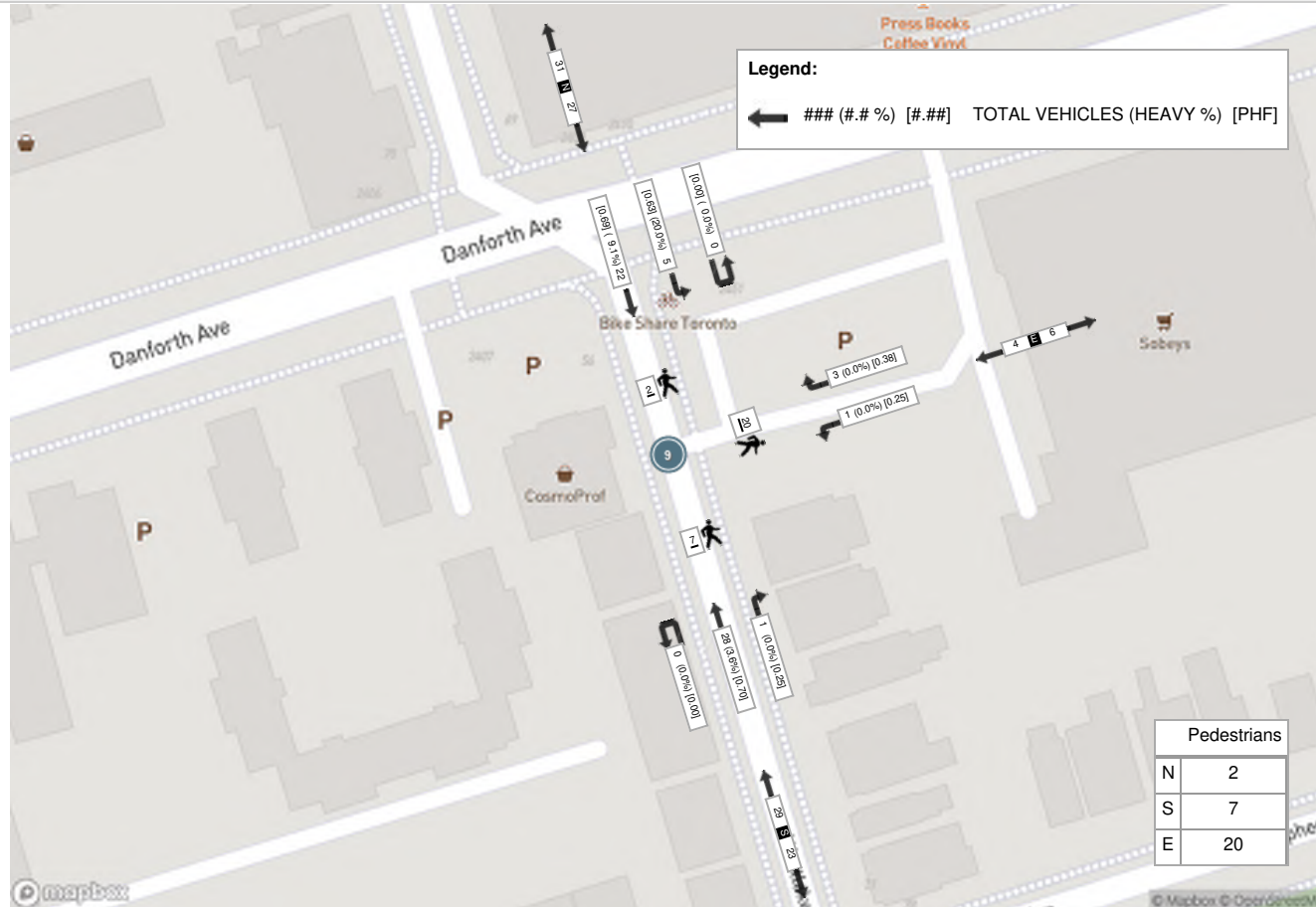
Start Time	N Approach WESTLAKE AVE					E Approach 2451 DANFORTH AVE (SOBEYS ACCESS)					S Approach WESTLAKE AVE					Int. Total (15 min)
	Thru	Left	UTurn	Peds	Approach Total	Right	Left	UTurn	Peds	Approach Total	Right	Thru	UTurn	Peds	Approach Total	
08:00:00	4	1	0	0	5	0	0	0	3	0	0	9	0	2	9	14
08:15:00	4	0	0	1	4	0	0	0	3	0	1	10	0	1	11	15
08:30:00	8	2	0	0	10	1	1	0	7	2	0	5	0	2	5	17
08:45:00	6	2	0	1	8	2	0	0	7	2	0	4	0	2	4	14
Grand Total	22	5	0	2	27	3	1	0	20	4	1	28	0	7	29	60
Approach%	81.5%	18.5%	0%		-	75%	25%	0%		-	3.4%	96.6%	0%		-	-
Totals %	36.7%	8.3%	0%		45%	5%	1.7%	0%		6.7%	1.7%	46.7%	0%		48.3%	-
PHF	0.69	0.63	0		0.68	0.38	0.25	0		0.5	0.25	0.7	0		0.66	-
Heavy	2	1	0		3	0	0	0		0	0	1	0		1	-
Heavy %	9.1%	20%	0%		11.1%	0%	0%	0%		0%	0%	3.6%	0%		3.4%	-
Lights	20	4	0		24	3	1	0		4	1	27	0		28	-
Lights %	90.9%	80%	0%		88.9%	100%	100%	0%		100%	100%	96.4%	0%		96.6%	-
Single-Unit Trucks	0	0	0		0	0	0	0		0	0	0	0		0	-
Single-Unit Trucks %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Buses	2	0	0		2	0	0	0		0	0	1	0		1	-
Buses %	9.1%	0%	0%		7.4%	0%	0%	0%		0%	0%	3.6%	0%		3.4%	-
Articulated Trucks	0	1	0		1	0	0	0		0	0	0	0		0	-
Articulated Trucks %	0%	20%	0%		3.7%	0%	0%	0%		0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	2	-	-	-	-	20	-	-	-	-	7	-	-
Pedestrians%	-	-	-	6.9%		-	-	-	69%		-	-	-	24.1%		-
Bicycles on Road	3	0	0	0	-	0	0	0	0	-	1	0	0	0	-	-
Bicycles on Road%	-	-	-	0%		-	-	-	0%		-	-	-	0%		-



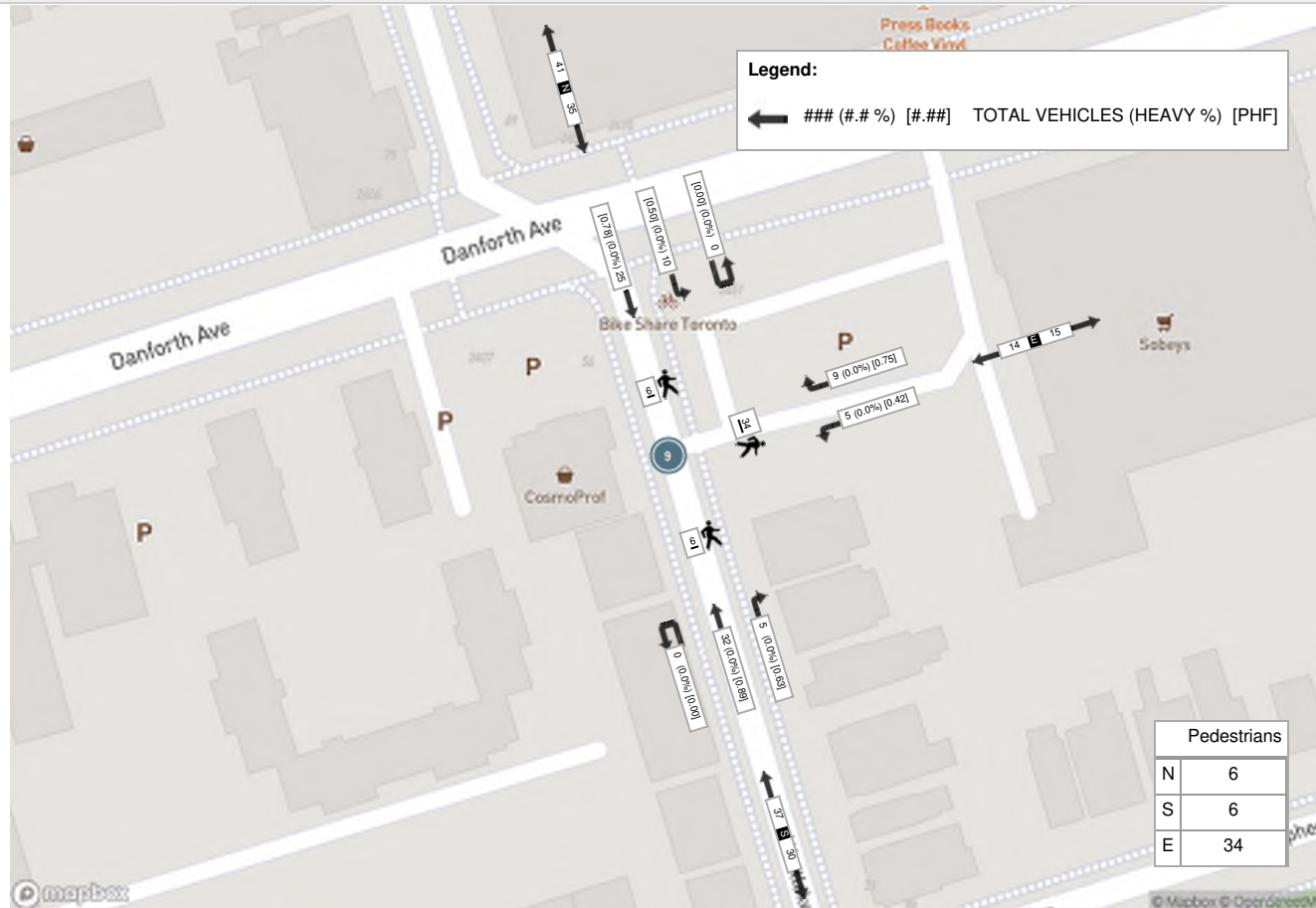
Peak Hour: 04:45 PM - 05:45 PM Weather: Clear Sky (19.95 °C)

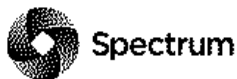
Start Time	N Approach WESTLAKE AVE					E Approach 2451 DANFORTH AVE (SOBEYS ACCESS)					S Approach WESTLAKE AVE					Int. Total (15 min)
	Thru	Left	UTurn	Peds	Approach Total	Right	Left	UTurn	Peds	Approach Total	Right	Thru	UTurn	Peds	Approach Total	
16:45:00	2	1	0	0	3	3	1	0	6	4	0	9	0	3	9	16
17:00:00	8	0	0	1	8	2	0	0	6	2	2	7	0	0	9	19
17:15:00	7	5	0	3	12	1	3	0	14	4	2	7	0	1	9	25
17:30:00	8	4	0	2	12	3	1	0	8	4	1	9	0	2	10	26
Grand Total	25	10	0	6	35	9	5	0	34	14	5	32	0	6	37	86
Approach%	71.4%	28.6%	0%		-	64.3%	35.7%	0%		-	13.5%	86.5%	0%		-	-
Totals %	29.1%	11.6%	0%		40.7%	10.5%	5.8%	0%		16.3%	5.8%	37.2%	0%		43%	-
PHF	0.78	0.5	0		0.73	0.75	0.42	0		0.88	0.63	0.89	0		0.93	-
Heavy	0	0	0		0	0	0	0		0	0	0	0		0	-
Heavy %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Lights	25	10	0		35	9	5	0		14	5	32	0		37	-
Lights %	100%	100%	0%		100%	100%	100%	0%		100%	100%	100%	0%		100%	-
Single-Unit Trucks	0	0	0		0	0	0	0		0	0	0	0		0	-
Single-Unit Trucks %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Buses	0	0	0		0	0	0	0		0	0	0	0		0	-
Buses %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Articulated Trucks	0	0	0		0	0	0	0		0	0	0	0		0	-
Articulated Trucks %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	6	-	-	-	-	34	-	-	-	-	6	-	-
Pedestrians%	-	-	-	13%		-	-	-	73.9%		-	-	-	13%		-
Bicycles on Road	0	2	0	0	-	0	0	0	0	-	0	2	0	0	-	-
Bicycles on Road%	-	-	-	0%		-	-	-	0%		-	-	-	0%		-

Peak Hour: 08:00 AM - 09:00 AM Weather: Clear Sky (13.64 °C)



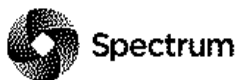
Peak Hour: 04:45 PM - 05:45 PM Weather: Clear Sky (19.95 °C)





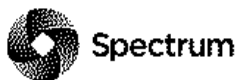
Turning Movement Count (8 . WESTLAKE AVE & COSMO PROF ACCESS)

Start Time	N Approach WESTLAKE AVE					S Approach WESTLAKE AVE					W Approach 2407 DANFORTH AVE (COSMO PROF ACCESS)					Int. Total (15 min)	Int. Total (1 hr)
	Right N:W	Thru N:S	UTurn N:N	Peds N:	Approach Total	Thru S:N	Left S:W	UTurn S:S	Peds S:	Approach Total	Right W:S	Left W:N	UTurn W:W	Peds W:	Approach Total		
07:30:00	0	2	0	0	2	5	0	0	0	5	0	0	0	0	0	7	
07:45:00	0	5	0	2	5	6	0	0	0	6	0	0	0	0	0	11	
08:00:00	0	5	0	0	5	9	0	0	0	9	0	0	0	1	0	14	
08:15:00	0	4	0	0	4	9	0	0	1	9	0	0	0	14	0	13	45
08:30:00	1	10	0	0	11	7	0	0	0	7	0	0	0	9	0	18	56
08:45:00	1	8	0	0	9	7	0	0	1	7	0	2	0	5	2	18	63
09:00:00	2	9	0	0	11	4	0	0	0	4	0	0	0	2	0	15	64
09:15:00	0	7	0	1	7	5	0	0	0	5	0	1	0	1	1	13	64
BREAK																	
16:00:00	0	10	0	0	10	7	0	0	5	7	0	2	0	5	2	19	
16:15:00	0	8	0	1	8	6	0	0	1	6	1	0	0	2	1	15	
16:30:00	0	10	0	1	10	12	0	0	1	12	0	0	0	4	0	22	
16:45:00	0	3	0	0	3	12	0	0	0	12	0	0	0	4	0	15	71
17:00:00	0	7	0	1	7	9	0	0	1	9	1	0	0	12	1	17	69
17:15:00	1	12	0	0	13	8	0	0	3	8	0	0	0	9	0	21	75
17:30:00	0	11	0	2	11	12	0	0	2	12	0	1	0	9	1	24	77
17:45:00	1	7	0	0	8	3	0	0	0	3	0	0	0	11	0	11	73
Grand Total	6	118	0	8	124	121	0	0	15	121	2	6	0	88	8	253	-
Approach%	4.8%	95.2%	0%		-	100%	0%	0%		-	25%	75%	0%		-	-	-
Totals %	2.4%	46.6%	0%		49%	47.8%	0%	0%		47.8%	0.8%	2.4%	0%		3.2%	-	-
Heavy	0	6	0		-	1	0	0		-	0	0	0		-	-	-
Heavy %	0%	5.1%	0%		-	0.8%	0%	0%		-	0%	0%	0%		-	-	-
Bicycles	0	5	1		-	4	4	0		-	4	0	0		-	-	-
Bicycle %	0%	4.2%	0%		-	3.3%	0%	0%		-	200%	0%	0%		-	-	-



Peak Hour: 08:30 AM - 09:30 AM Weather: Clear Sky (13.64 °C)

Start Time	N Approach WESTLAKE AVE					S Approach WESTLAKE AVE					W Approach 2407 DANFORTH AVE (COSMO PROF ACCESS)					Int. Total (15 min)
	Right	Thru	UTurn	Peds	Approach Total	Thru	Left	UTurn	Peds	Approach Total	Right	Left	UTurn	Peds	Approach Total	
08:30:00	1	10	0	0	11	7	0	0	0	7	0	0	0	9	0	18
08:45:00	1	8	0	0	9	7	0	0	1	7	0	2	0	5	2	18
09:00:00	2	9	0	0	11	4	0	0	0	4	0	0	0	2	0	15
09:15:00	0	7	0	1	7	5	0	0	0	5	0	1	0	1	1	13
Grand Total	4	34	0	1	38	23	0	0	1	23	0	3	0	17	3	64
Approach%	10.5%	89.5%	0%		-	100%	0%	0%		-	0%	100%	0%		-	-
Totals %	6.3%	53.1%	0%		59.4%	35.9%	0%	0%		35.9%	0%	4.7%	0%		4.7%	-
PHF	0.5	0.85	0		0.86	0.82	0	0		0.82	0	0.38	0		0.38	-
Heavy	0	1	0		1	0	0	0		0	0	0	0		0	-
Heavy %	0%	2.9%	0%		2.6%	0%	0%	0%		0%	0%	0%	0%		0%	-
Lights	4	33	0		37	23	0	0		23	0	3	0		3	-
Lights %	100%	97.1%	0%		97.4%	100%	0%	0%		100%	0%	100%	0%		100%	-
Single-Unit Trucks	0	0	0		0	0	0	0		0	0	0	0		0	-
Single-Unit Trucks %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Buses	0	0	0		0	0	0	0		0	0	0	0		0	-
Buses %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Articulated Trucks	0	1	0		1	0	0	0		0	0	0	0		0	-
Articulated Trucks %	0%	2.9%	0%		2.6%	0%	0%	0%		0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	1	-	-	-	-	1	-	-	-	-	17	-	-
Pedestrians%	-	-	-	5.3%	-	-	-	-	5.3%	-	-	-	-	89.5%	-	-
Bicycles on Road	0	1	1	0	-	0	2	0	0	-	0	0	0	0	-	-
Bicycles on Road%	-	-	-	0%	-	-	-	-	0%	-	-	-	-	0%	-	-



Peak Hour: 04:45 PM - 05:45 PM Weather: Clear Sky (19.95 °C)

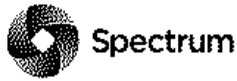
Start Time	N Approach WESTLAKE AVE					S Approach WESTLAKE AVE					W Approach 2407 DANFORTH AVE (COSMO PROF ACCESS)					Int. Total (15 min)
	Right	Thru	UTurn	Peds	Approach Total	Thru	Left	UTurn	Peds	Approach Total	Right	Left	UTurn	Peds	Approach Total	
16:45:00	0	3	0	0	3	12	0	0	0	12	0	0	0	4	0	15
17:00:00	0	7	0	1	7	9	0	0	1	9	1	0	0	12	1	17
17:15:00	1	12	0	0	13	8	0	0	3	8	0	0	0	9	0	21
17:30:00	0	11	0	2	11	12	0	0	2	12	0	1	0	9	1	24
Grand Total	1	33	0	3	34	41	0	0	6	41	1	1	0	34	2	77
Approach%	2.9%	97.1%	0%		-	100%	0%	0%		-	50%	50%	0%		-	-
Totals %	1.3%	42.9%	0%		44.2%	53.2%	0%	0%		53.2%	1.3%	1.3%	0%		2.6%	-
PHF	0.25	0.69	0		0.65	0.85	0	0		0.85	0.25	0.25	0		0.5	-
Heavy	0	0	0		0	0	0	0		0	0	0	0		0	-
Heavy %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Lights	1	33	0		34	41	0	0		41	1	1	0		2	-
Lights %	100%	100%	0%		100%	100%	0%	0%		100%	100%	100%	0%		100%	-
Single-Unit Trucks	0	0	0		0	0	0	0		0	0	0	0		0	-
Single-Unit Trucks %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Buses	0	0	0		0	0	0	0		0	0	0	0		0	-
Buses %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Articulated Trucks	0	0	0		0	0	0	0		0	0	0	0		0	-
Articulated Trucks %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	3	-	-	-	-	6	-	-	-	-	34	-	-
Pedestrians%	-	-	-	7%		-	-	-	14%		-	-	-	79.1%		-
Bicycles on Road	0	1	0	0	-	2	0	0	0	-	1	0	0	0	-	-
Bicycles on Road%	-	-	-	0%		-	-	-	0%		-	-	-	0%		-

Peak Hour: 08:30 AM - 09:30 AM Weather: Clear Sky (13.64 °C)



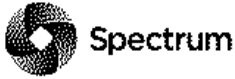
Peak Hour: 04:45 PM - 05:45 PM Weather: Clear Sky (19.95 °C)





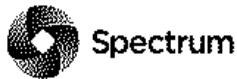
Turning Movement Count (10 . WESTLAKE AVE & STEPHENSON AVE)

Start Time	N Approach WESTLAKE AVE						E Approach STEPHENSON AVE						S Approach WESTLAKE AVE						W Approach STEPHENSON AVE						Int. Total (15 min)	Int. Total (1 hr)	
	Right N:W	Thru N:S	Left N:E	UTurn N:N	Peds N:	Approach Total	Right E:N	Thru E:W	Left E:S	UTurn E:E	Peds E:	Approach Total	Right S:E	Thru S:N	Left S:W	UTurn S:S	Peds S:	Approach Total	Right W:S	Thru W:E	Left W:N	UTurn W:W	Peds W:	Approach Total			
07:30:00	0	1	1	0	2	2	2	1	0	0	0	3	1	2	0	0	7	3	0	0	1	0	6	1	9		
07:45:00	0	3	1	0	1	4	4	1	1	0	3	6	2	1	0	0	2	3	0	0	1	0	1	1	14		
08:00:00	1	1	2	0	5	4	3	2	1	0	0	6	0	5	0	0	8	5	0	3	0	0	5	3	18		
08:15:00	0	0	3	0	16	3	6	0	1	1	7	8	3	3	1	0	7	7	0	3	1	0	6	4	22	63	
08:30:00	3	3	3	1	14	10	3	3	1	0	3	7	0	1	0	0	6	1	0	5	0	0	5	5	23	77	
08:45:00	1	2	2	0	12	5	3	5	1	0	5	9	0	1	0	0	8	1	0	3	0	0	10	3	18	81	
09:00:00	1	2	2	0	2	5	2	1	0	0	4	3	0	1	0	0	5	1	1	1	0	0	3	2	11	74	
09:15:00	2	1	4	0	5	7	3	1	1	0	6	5	1	1	0	0	1	2	0	0	0	0	0	0	14	66	
BREAK																											
16:00:00	2	2	3	0	12	7	6	4	1	0	7	11	4	1	0	0	8	5	0	6	1	0	7	7	30		
16:15:00	0	2	4	0	9	6	4	6	1	0	5	11	5	2	0	0	3	7	1	6	1	0	2	8	32		
16:30:00	1	3	6	0	4	10	4	2	3	0	5	9	5	3	0	0	5	8	1	2	1	0	4	4	31		
16:45:00	1	1	2	0	4	4	8	2	3	0	5	13	3	1	0	0	6	4	0	10	0	0	1	10	31	124	
17:00:00	1	3	4	0	11	8	3	4	3	0	12	10	4	2	0	0	5	6	1	8	3	0	5	12	36	130	
17:15:00	4	3	4	0	9	11	5	9	2	0	8	16	5	2	0	0	16	7	1	4	2	0	9	7	41	139	
17:30:00	0	2	6	0	9	8	4	4	4	0	16	12	3	4	1	0	17	8	0	7	1	0	8	8	36	144	
17:45:00	1	1	4	0	7	6	1	3	0	0	7	4	4	2	4	0	4	10	0	11	0	1	13	12	32	145	
Grand Total	18	30	51	1	122	100	61	48	23	1	93	133	40	32	6	0	108	78	5	69	12	1	85	87	398	-	
Approach%	18%	30%	51%	1%	-	-	45.9%	36.1%	17.3%	0.8%	-	-	51.3%	41%	7.7%	0%	-	-	5.7%	79.3%	13.8%	1.1%	-	-	-	-	
Totals %	4.5%	7.5%	12.8%	0.3%	-	25.1%	15.3%	12.1%	5.8%	0.3%	-	33.4%	10.1%	8%	1.5%	0%	-	19.6%	1.3%	17.3%	3%	0.3%	-	21.9%	-	-	
Heavy	1	2	1	0	-	-	0	1	1	0	-	-	0	1	0	0	-	-	0	0	0	0	-	-	-	-	
Heavy %	5.6%	6.7%	2%	0%	-	-	0%	2.1%	4.3%	0%	-	-	0%	3.1%	0%	0%	-	-	0%	0%	0%	0%	-	-	-	-	
Bicycles	2	0	4	0	-	-	3	3	3	0	-	-	3	4	0	0	-	-	0	2	0	0	-	-	-	-	
Bicycle %	11.1%	0%	7.8%	0%	-	-	4.9%	6.3%	13%	0%	-	-	7.5%	12.5%	0%	0%	-	-	0%	2.9%	0%	0%	-	-	-	-	



Peak Hour: 08:00 AM - 09:00 AM Weather: Clear Sky (13.64 °C)

Start Time	N Approach WESTLAKE AVE						E Approach STEPHENSON AVE						S Approach WESTLAKE AVE						W Approach STEPHENSON AVE						Int. Total (15 min)
	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	
08:00:00	1	1	2	0	5	4	3	2	1	0	0	6	0	5	0	0	8	5	0	3	0	0	5	3	18
08:15:00	0	0	3	0	16	3	6	0	1	1	7	8	3	3	1	0	7	7	0	3	1	0	6	4	22
08:30:00	3	3	3	1	14	10	3	3	1	0	3	7	0	1	0	0	6	1	0	5	0	0	5	5	23
08:45:00	1	2	2	0	12	5	3	5	1	0	5	9	0	1	0	0	8	1	0	3	0	0	10	3	18
Grand Total	5	6	10	1	47	22	15	10	4	1	15	30	3	10	1	0	29	14	0	14	1	0	26	15	81
Approach%	22.7%	27.3%	45.5%	4.5%	-	-	50%	33.3%	13.3%	3.3%	-	-	21.4%	71.4%	7.1%	0%	-	-	0%	93.3%	6.7%	0%	-	-	-
Totals %	6.2%	7.4%	12.3%	1.2%	27.2%	27.2%	18.5%	12.3%	4.9%	1.2%	37%	37%	3.7%	12.3%	1.2%	0%	17.3%	0%	17.3%	1.2%	0%	-	18.5%	-	-
PHF	0.42	0.5	0.83	0.25	0.55	0.55	0.63	0.5	1	0.25	0.83	0.83	0.25	0.5	0.25	0	0.5	0.5	0	0.7	0.25	0	0.75	-	-
Heavy	1	0	1	0	-	2	0	1	1	0	-	2	0	1	0	0	-	1	0	0	0	0	-	0	-
Heavy %	20%	0%	10%	0%	-	9.1%	0%	10%	25%	0%	-	6.7%	0%	10%	0%	0%	-	7.1%	0%	0%	0%	0%	-	0%	-
Lights	4	6	9	1	-	20	15	9	3	1	-	28	3	9	1	0	-	13	0	14	1	0	-	15	-
Lights %	80%	100%	90%	100%	-	90.9%	100%	90%	75%	100%	-	93.3%	100%	90%	100%	0%	-	92.9%	0%	100%	100%	0%	-	100%	-
Single-Unit Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	-
Single-Unit Trucks %	0%	0%	0%	0%	-	0%	0%	0%	0%	0%	-	0%	0%	0%	0%	0%	-	0%	0%	0%	0%	0%	-	0%	-
Buses	1	0	1	0	-	2	0	1	1	0	-	2	0	1	0	0	-	1	0	0	0	0	-	0	-
Buses %	20%	0%	10%	0%	-	9.1%	0%	10%	25%	0%	-	6.7%	0%	10%	0%	0%	-	7.1%	0%	0%	0%	0%	-	0%	-
Pedestrians	-	-	-	-	47	-	-	-	-	-	15	-	-	-	-	-	28	-	-	-	-	-	26	-	-
Pedestrians%	-	-	-	-	40.2%	-	-	-	-	-	12.8%	-	-	-	-	-	23.9%	-	-	-	-	-	22.2%	-	-
Bicycles on Road	0	0	2	0	0	-	0	2	1	0	0	-	0	1	0	0	0	-	0	0	0	0	0	-	-
Bicycles on Road%	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	0	-	-
Bicycles on Crosswalk%	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	0.9%	-	-	-	-	-	0%	-	-



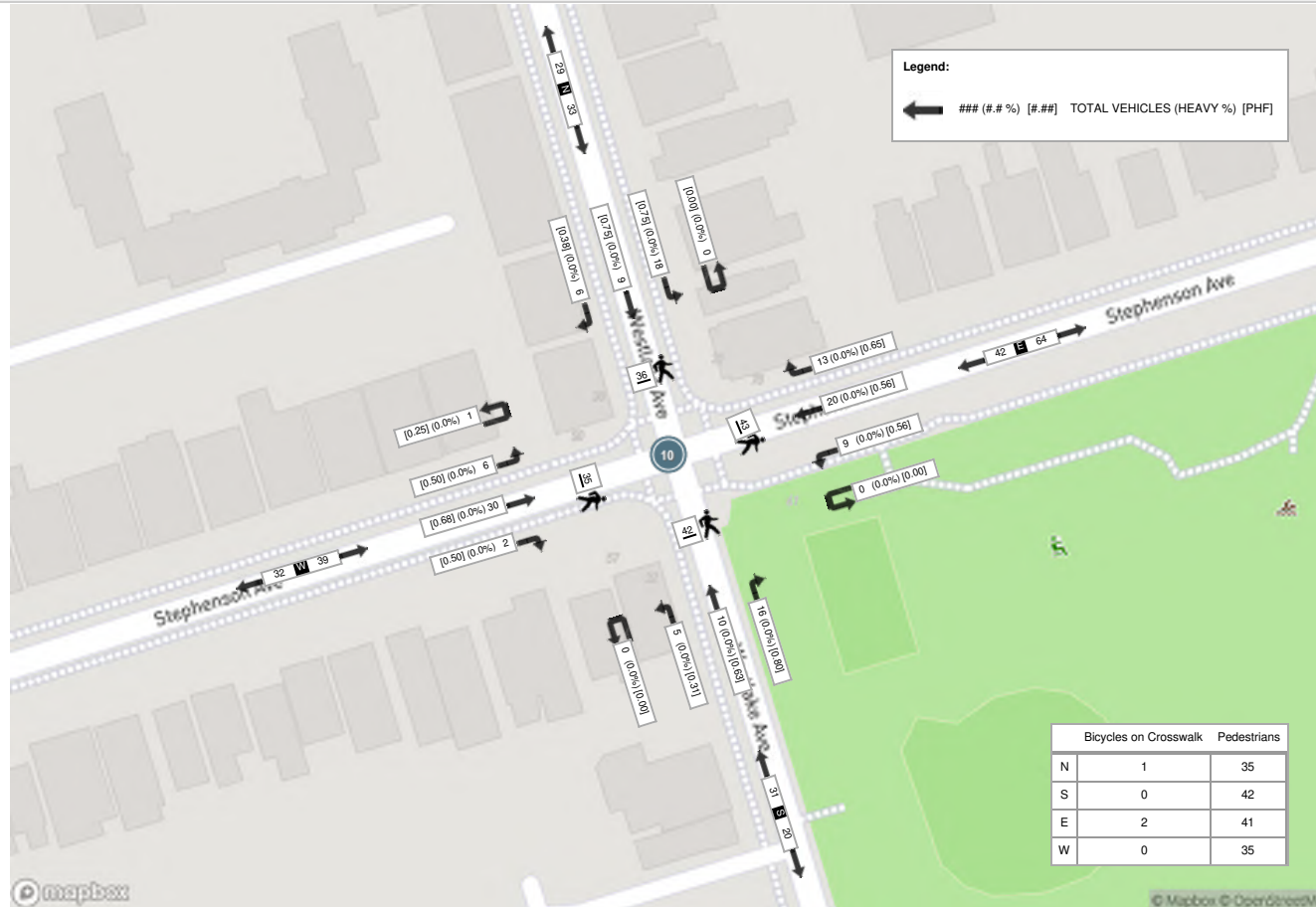
Peak Hour: 05:00 PM - 06:00 PM Weather: Clear Sky (19.95 °C)

Start Time	N Approach WESTLAKE AVE						E Approach STEPHENSON AVE						S Approach WESTLAKE AVE						W Approach STEPHENSON AVE						Int. Total (15 min)
	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	Right	Thru	Left	UTurn	Peds	Approach Total	
17:00:00	1	3	4	0	11	8	3	4	3	0	12	10	4	2	0	0	5	6	1	8	3	0	5	12	36
17:15:00	4	3	4	0	9	11	5	9	2	0	8	16	5	2	0	0	16	7	1	4	2	0	9	7	41
17:30:00	0	2	6	0	9	8	4	4	4	0	16	12	3	4	1	0	17	8	0	7	1	0	8	8	36
17:45:00	1	1	4	0	7	6	1	3	0	0	7	4	4	2	4	0	4	10	0	11	0	1	13	12	32
Grand Total	6	9	18	0	36	33	13	20	9	0	43	42	16	10	5	0	42	31	2	30	6	1	35	39	145
Approach%	18.2%	27.3%	54.5%	0%		-	31%	47.6%	21.4%	0%		-	51.6%	32.3%	16.1%	0%		-	5.1%	76.9%	15.4%	2.6%		-	-
Totals %	4.1%	6.2%	12.4%	0%		22.8%	9%	13.8%	6.2%	0%		29%	11%	6.9%	3.4%	0%		21.4%	1.4%	20.7%	4.1%	0.7%		26.9%	-
PHF	0.38	0.75	0.75	0		0.75	0.65	0.56	0.56	0		0.66	0.8	0.63	0.31	0		0.78	0.5	0.68	0.5	0.25		0.81	-
Heavy	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	-
Heavy %	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	-
Lights	6	9	18	0		33	13	20	9	0		42	16	10	5	0		31	2	30	6	1		39	-
Lights %	100%	100%	100%	0%		100%	100%	100%	100%	0%		100%	100%	100%	100%	0%		100%	100%	100%	100%	100%		100%	-
Single-Unit Trucks	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	-
Single-Unit Trucks %	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	-
Buses	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	-
Buses %	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	-	35	-	-	-	-	-	41	-	-	-	-	-	42	-	-	-	-	-	35	-	-
Pedestrians%	-	-	-	-	22.4%	-	-	-	-	-	26.3%	-	-	-	-	-	26.9%	-	-	-	-	-	22.4%	-	-
Bicycles on Road	0	0	0	0	0	-	0	0	0	0	0	-	0	1	0	0	0	-	0	1	0	0	0	-	-
Bicycles on Road%	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-
Bicycles on Crosswalk	-	-	-	-	1	-	-	-	-	-	2	-	-	-	-	-	0	-	-	-	-	-	0	-	-
Bicycles on Crosswalk%	-	-	-	-	0.6%	-	-	-	-	-	1.3%	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-

Peak Hour: 08:00 AM - 09:00 AM Weather: Clear Sky (13.64 °C)



Peak Hour: 05:00 PM - 06:00 PM Weather: Clear Sky (19.95 °C)

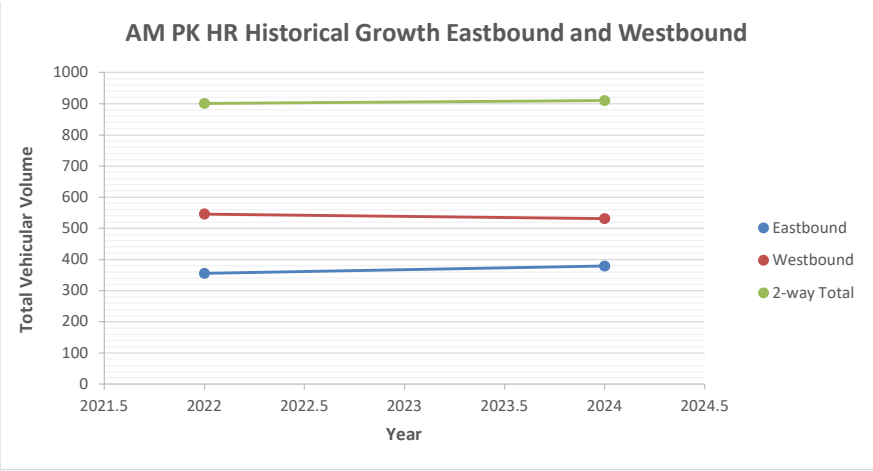


Appendix F:

Corridor Growth Rate

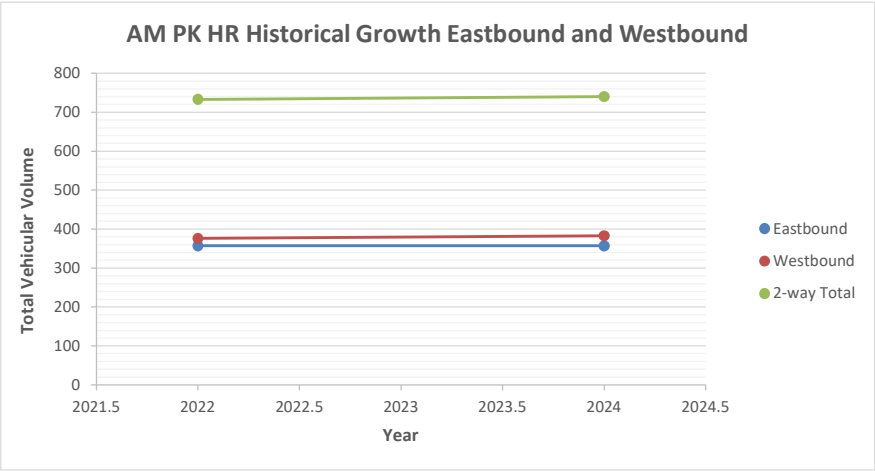
Project:	2451 Danforth Ave
Project ID:	7036-35
Intersection:	Danforth Ave / Main St
Peak Hour:	AM

Count Information		East of Danforth Ave / Main St		
Date	Year	Eastbound	Westbound	2-way
September 1, 2022	2022	355	546	901
May 25, 2024	2024	379	531	910
Existing	2024	379	531	910
Slope		12.00	-7.50	4.50
Annual Growth		3.17%	-1.41%	0.49%



Project:	2451 Danforth Ave
Project ID:	7036-35
Intersection:	Danforth Ave / Main Street
Peak Hour:	PM

Count Information		East of Danforth Ave / Main Street		
Date	Year	Eastbound	Westbound	2-way
September 1, 2022	2022	357	376	733
May 25, 2024	2024	357	383	740
Existing	2024	357	383	740
Slope		0.00	3.50	3.50
Annual Growth		0.00%	0.91%	0.47%



Appendix G:

Signal Timing Plans (STPs)

List of Abbreviations for Traffic Signal Operations

January 25, 2021

System		Signal Mode of Operation		Intersection Features	
TransSuite TCS		FA	Fully Actuated	APS	Accessible Pedestrian Signal
SCOOT		FT	Fixed Time	AAWB	Active Advanced Warning Beacons
SCATS		SA1	Semi Actuated 1 (Pedestrian Fixed Time)	LBI	Leading Bicycle Interval
		SA2	Semi-Actuated 2 (with vehicle extensions)	LBO	LED Blank-Out Sign
		SAP	Semi-Actuated Pedestrian (without vehicle extensions)	LPI	Leading Pedestrian Interval
		SAV	Semi-Actuated Vehicle (no side street pedestrians phase)	PE	Pre-emption
		PA-IPS	Pedestrian Actuated at Intersection Pedestrian Signals	PPP	Pedestrian Priority Phase
		PA-MPS	Pedestrian Actuated at Midblock Pedestrian Signals	PR	Pedestrian Recycle
				TSP	Transit Signal Priority
				WRM	Walk Rest Modifier
Intersection Identification					
TCS#	Intersection Number				
SCN	UTC/SCOOT Number				
Special Facility (SCOOT)		Communication			
SF#1	Special Facility #1	AC	Awaiting Communication		
SF#2	Special Facility #2	LOCAL	Not On Computer Control		
SF#3	Special Facility #3				
SF#4	Special Facility #4				

Phase Aspect		All Directions	North/South	East/West	Northbound	Southbound	Eastbound	Westbound
1. Normal Phase								
	Green		NSG	EWG	NBG	SBG	EBG	WBG
	Amber (Yellow)		NSY	EWY	NBY	SBY	EBY	WBY
	Red		NSR	EWR	NBR	SBR	EBR	WBR
	All	ALR						
2. Priority Phase (Arrow)								
	Straight-Through Green		NSSA	EWSA	NBSA	SBSA	EBSA	WBSA
	Left-Turn Green		NSLA	EWLA	NBLA	SBLA	EBLA	WBLA
	Left-Turn Amber		NSYA	EWYA	NBYA	SBYA	EBYA	WBYA
	Left-Turn Amber Ball		NSLY	EWLY	NBLY	SBLY	EBLY	WBLY
	Left-Turn Red Ball		NSLR	EWLR	NBLR	SBLR	EBLR	WBLR
	Right-Turn Green				NBRA	SBRA	EBRA	WBRA
	Right-Turn Amber				NRYA	SBRYA	EBRYA	WBRYA
	Right-Turn Amber Ball				NBRY	SBRY	EBRY	WBRY
	Right-Turn Red Ball				NBRR	SBRR	EBRR	WBRR
3. Pedestrian								
	Walk	WK	NSWK	EWWK	NBWK	SBWK	EBWK	WBWK
	Flashing Don't Walk	FDW	NSFDW	EWFDW	NBFDW	SBFDW	EBFDW	WBFDW
	Don't Walk	DW	NSDW	EWDW	NBDW	SBDW	EBDW	WBDW
4. Transit								
	Green		NSTG	EWTG	NBTG	SBTG	EBTG	WBTG
	Straight-Through Green Arrow		NSTGA	EWTGA	NBTGA	SBTGA	EBTGA	WBTGA
	Amber (Yellow)		NSTY	EWTY	NBTY	SBTY	EBTY	WBTY
	Bar		NSTB	EWTB	NBTB	SBTB	EBTB	WBTB
	Red		NSTR	EWTR	NBTR	SBTR	EBTR	WBTR
5. Bike								
	Green		NSBG	EWBG	NBBG	SBBG	EBBG	WBBG
	Amber (Yellow)		NSBY	EWBY	NBBY	SBBY	EBBY	WBBY
	Red		NSBR	EWBR	NBBR	SBBR	EBBR	WBBR

LOCATION: Danforth Ave & Gledhill Ave/Patricia Dr MODE/COMMENT: SAP with PR & LPI TCS: 352 PREPARED BY/DATE: HDR / Jacky Chan / January 27, 2023 CHECKED BY/DATE: HDR / Dan Lu / January 31, 2023 CITY STAFF: Syed Qasim IMPLEMENTATION DATE: March 20, 2023								ATO (DISTRICT) / WARD: 1 (Toronto & East York) / 19 COMPUTER SYSTEM: TransSuite CONTROLLER / CABINET TYPE: Peek ATC-1000 / TS2T1 CONFLICT FLASH: Red & Red DESIGN WALK SPEED: 1.0 m/s (FDW based on full crossing at 1.2 m/s) CHANNEL/DROP: 4008/30 CONTROLLER FIRMWARE: 3.018.1.2976	N
NEMA Phase		OFF	AM	PM	NIGHT	WKND	DVP Closure	Phase Mode (Fixed/Demanded/Callable)	Remarks
		All Other Times	06:30-09:30 M-F	15:00-19:00 M-F	23:00-06:30 Daily	10:00-19:00 Sat & Sun			
		Pattern 1	Pattern 2	Pattern 3	Pattern 4	Pattern 5	Pattern 16		
	Local Plan Split Table	Split 1	Split 2	Split 3	Split 4	Split 5	Split 16		
1		WLK FDW MIN MAX1 AMB ALR SPLIT							Pedestrian Minimums: EWWK = 12 sec, EWFD = 12 sec NSWK = 12 sec, NSFD = 16 sec NS phase is callable by vehicle or pedestrian actuation. The NSWK & NSFD are displayed on the pedestrian signal heads if a vehicle and/or pedestrian call is received. Side street passage time = 3 sec
2	Danforth Ave 	DLY GRN 5 WLK 12 FDW 12 MIN 19 MAX1 37 AMB 3.0 ALR 2.9 SPLIT						Fixed Split shown includes 5 sec EW LPI	EW Leading Pedestrian Interval - EWWK comes up 5 sec before EW vehicle green NS Leading Pedestrian Interval - NSWK comes up 5 sec before NS vehicle green The following grades were used to calculate the AMB intervals: East Leg = -1.0% West Leg = -1.1% North Leg = -1.1% South Leg = -1.8%
3		WLK FDW MIN MAX1 AMB ALR SPLIT							
4	Gledhill Ave 	DLY GRN 5 WLK 12 FDW 16 MIN 23 MAX1 23 AMB 3.0 ALR 4.1 SPLIT						Callable by Stopbar Loop and/or Push Button Split shown includes 5 sec NS LPI	
5		WLK FDW MIN MAX1 AMB ALR SPLIT							
6	Danforth Ave 	DLY GRN 5 WLK 12 FDW 12 MIN 19 MAX1 37 AMB 3.0 ALR 2.9 SPLIT						Fixed Split shown includes 5 sec EW LPI	
7		WLK FDW MIN MAX1 AMB ALR SPLIT							
8	Patricia Dr 	DLY GRN 5 WLK 12 FDW 16 MIN 23 MAX1 23 AMB 3.0 ALR 4.1 SPLIT						Callable by Stopbar Loop and/or Push Button Split shown includes 5 sec NS LPI	
	CL OF	84 79	90 63	90 43	72 26	84 26	94 44		

Notes: NS Amber and All-red values updated to reflect NS speed limit reduction to 30 km/h.

LOCATION: Main St & Danforth Ave MODE/COMMENT: SA1 with 2 wire Polara APS & TSP* & LPI TCS: 353 PREPARED BY/DATE: Lina Elmorshedy / April 17, 2024 CHECKED BY/DATE: Ihtesham Ahmed / April 17, 2024 CITY STAFF: IMPLEMENTATION DATE:										ATO (DISTRICT) / WARD: 1 (Toronto & East York) / 19 COMPUTER SYSTEM: TransSuite CONTROLLER/CABINET TYPE: Peek ATC-1000 / TS2T1 CONFLICT FLASH: Red & Red DESIGN WALK SPEED: 0.9 m/s (FDW based on full crossing at 1.1 m/s) CHANNEL/DROP: 4008/24 CONTROLLER FIRMWARE: 3.018.1.2976										N ↑
NEMA Phase		OFF All Other Times	AM 06:30-09:30 M-F	PM 15:00-19:00 M-F	NIGHT 23:00-06:30 Daily	WKND 10:00-19:00 Sat & Sun	Special Event TBD	DVP Closure	Phase Mode (Fixed/Demanded/Callable)	Remarks										
	Local Plan Split Table	Pattern 1 Split 1	Pattern 2 Split 2	Pattern 3 Split 3	Pattern 4 Split 4	Pattern 5 Split 5	Pattern 15 Split 15	Pattern 16 Split 16												
1	NOT USED	WLK FDW MIN MAX1 AMB ALR SPLIT								Pedestrian Minimums: EWWK = 8 sec, EWFD = 17 sec NSWK = 8 sec, NSFD = 22 sec Left Turn Passage = 2 sec *See back for TSP instructions TSP enabled for NB on November 7, 2017. The SB enabled on September 5, 2019. Actuated APS on during EWWK & NSWK when no left turn arrows are displayed.										
2	Main St ↑ ↕ ↓	WLK DLY 5 WLK 8 FDW 17 MIN 20 MAX1 20 AMB 3.0 ALR 3.7 SPLIT							Fixed Split shown includes 5 sec of NS LPI POZ activated by Request Loop (max extension of 16 secs in Green/SDW)	Extended APS Push Activation = 3 sec NS Leading Pedestrian Interval - NSWK comes up 5 seconds before NS vehicle green. The following grades were used to calculate the AMB intervals: East Leg = -2.0% West Leg = 0.0% North Leg = -0.4% South Leg = -2.3%										
3	↙	WLK FDW MIN 6 MAX1 6 AMB 3.0 ALR 4.4 SPLIT							Callable by 9m Setback Loop											
4	Danforth Ave → ↔	WLK 8 FDW 22 MIN 30 MAX1 31 AMB 3.0 ALR 3.7 SPLIT							Fixed (Truncation allowable)											
5	NOT USED	WLK FDW MIN MAX1 AMB ALR SPLIT																		
6	Main St ↕ ↓	WLK DLY 5 WLK 8 FDW 17 MIN 20 MAX1 20 AMB 3.0 ALR 3.7 SPLIT							Fixed Split shown includes 5 sec of NS LPI POZ activated by Request Loop (max extension of 16 secs in Green/SDW)											
7	NOT USED	WLK FDW MIN MAX1 AMB ALR SPLIT																		
8	Danforth Ave ↔ ←	WLK 8 FDW 22 MIN 30 MAX1 45 AMB 3.0 ALR 3.7 SPLIT							Fixed (Truncation allowable)											
		CL OF	84 31	90 89	90 70	72 53	84 48	94 55	94 31											

Notes:

LOC: Main St & Danforth Ave
MODE: FT with 2 wire Polara APS & TSP* & LPI
TCS: 353 PREPARATION DATE (TIMING CARD): December 23, 2022

OFFSET CORRECTION PARAMETERS

2.3.4 O.C. Extend / Reduce (Max. time added & subtracted in sec.)									
Ø 1 Ø 2 Ø 3 Ø 4 Ø 5 Ø 6 Ø 7 Ø 8									
[Cycle] [Skip]									
OFF									
Split 1	Ext.	--	21	--	21	--	21	--	21
	Rdc.	--	--	1	--	--	--	--	1
AM									
Split 2	Ext.	--	19	--	20	--	19	--	20
	Rdc.	--	--	4	--	--	--	--	4
PM									
Split 3	Ext.	--	18	--	19	--	18	--	19
	Rdc.	--	--	1	5	--	--	--	6
EVE									
Split 4	Ext.	--	17	--	18	--	17	--	18
	Rdc.	--	--	--	1	--	--	--	1
WKND									
Split 5	Ext.	--	21	--	21	--	21	--	21
	Rdc.	--	--	1	--	--	--	--	1
Special Events									
Split 15	Ext.	--	19	--	20	--	19	--	20
	Rdc.	--	--	--	7	--	--	--	7
DVP									
Split 16	Ext.	--	19	--	20	--	19	--	20
	Rdc.	--	--	2	4	--	--	--	6

OC Thresholds set to 3x OC Rdc due to limited slop. The controller could take up to three cycles to get back in sync from -TSP Recovery.
OC Rdc set to reduce to fractions of a second due to limit time available.
Phase 2/6 cannot be reduced due to firmware 3.18.1.2976 issue with LPIs and TSP.
Phase 4/8 cannot be reduced to the minimum due to a firmware 3.18.1.2976 issue with long left turn clearances.

T.S.P. PARAMETERS

PREPARED: JC

TSP RUN	TSP RUN
# 2	# 6
NB Thru	SB Thru

2.8.2 Transit Run Parameters

ATC Green Extend Mode (Equivalent TTC Algorithm)	Mode 0	Mode 0
	B-2	B-2

2.8.3 Transit Action Plan 1 (used for pattern 1, 4, 5, 15, 16)

Run Enable (X = Yes)	X	X
Run Config = 1	Recovery = 2 (O.C. with delay)	

2.8.3 Transit Action Plan 2 (used for pattern 2&3)

Run Enable (X = Yes)	X	X
Run Config = 2	Recovery = 2 (O.C. with delay)	

2.8.4 Transit Run Configuration 1

Delay / Extend / Fail	13 / -- / 235	24 / -- / 235
Max Req During Offset Corr	1	1
CALLS (and Extends)	Ø 2/6	Ø 2/6
Skips	--	--
Reduces (Truncates)	Ø 4/8	Ø 4/8

2.8.4 Transit Run Configuration 2

Delay / Extend / Fail	13 / -- / 235	5 / -- / 235
Max Req During Offset Corr	1	1
CALLS (and Extends)	Ø 2/6	Ø 2/6
Skips	--	--
Reduces (Truncates)	Ø 4/8	Ø 4/8

2.8.6 TSP Split Tables: 1, 2 & 5

	Ø 1	Ø 2	Ø 3	Ø 4	Ø 5	Ø 6	Ø 7	Ø 8
GRN EXT (SDW Extension)	--	16	--	--	--	16	--	--
GRN RDC (Reduction)	--	--	--	--	--	--	--	--
WLK EXT (Walk Extension)	--	--	--	--	--	--	--	--

2.8.6 TSP Split Tables: 3

GRN EXT (SDW Extension)	--	16	--	--	--	16	--	--
GRN RDC (Reduction)	--	--	--	5	--	--	--	5
WLK EXT (Walk Extension)	--	--	--	--	--	--	--	--

2.8.6 TSP Split Tables: 4

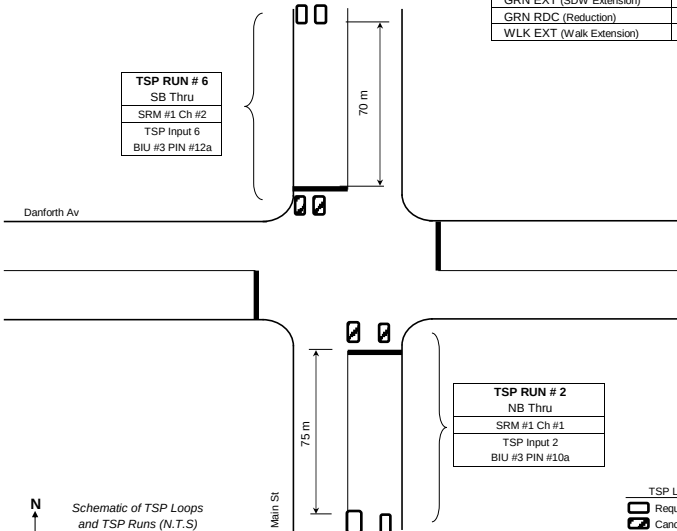
GRN EXT (SDW Extension)	--	16	--	--	--	16	--	--
GRN RDC (Reduction)	--	--	--	1	--	--	--	1
WLK EXT (Walk Extension)	--	--	--	--	--	--	--	--

2.8.6 TSP Split Tables: 15

GRN EXT (SDW Extension)	--	16	--	--	--	16	--	--
GRN RDC (Reduction)	--	--	--	7	--	--	--	7
WLK EXT (Walk Extension)	--	--	--	--	--	--	--	--

2.8.6 TSP Split Tables: 16

GRN EXT (SDW Extension)	--	16	--	--	--	16	--	--
GRN RDC (Reduction)	--	--	--	4	--	--	--	4
WLK EXT (Walk Extension)	--	--	--	--	--	--	--	--



ATC Mode	0	2	3	4
TTC Algorhm	B-2	A	C	D
Extensions	SDW	Walk	W/SDW	W/SDW

TSP SUMMARY

Maximum Green Extensions:
NSG:16 s Green/SDW
EW Truncation allowable to PED min by TOD

LOCATION: Danforth Ave & Westlake Ave MODE/COMMENT: SAP with PR and 2-Wire Polara APS & LPI TCS: 1267 PREPARED BY/DATE: Lina Elmorshedy / April 15, 2024 CHECKED BY/DATE: Ihtesham Ahmad / April 16, 2024 CITY STAFF: IMPLEMENTATION DATE: April 16, 2024							ATO (DISTRICT) / WARD: 1 (Toronto & East York) / 19 COMPUTER SYSTEM: TransSuite CONTROLLER/CABINET TYPE: Econolite ASC/3-2100 / TS2T1 CONFLICT FLASH: Red & Red DESIGN WALK SPEED: 0.9 m/s (FDW based on full crossing at 1.1 m/s) CHANNEL/DROP: 4008/8 CONTROLLER FIRMWARE: 2.47.10			N ↑
NEMA Phase		OFF All Other Times	AM 06:30-09:30 M-F	PM 15:00-19:00 M-F	NIGHT 23:00-06:30 M-F	WKND 10:00-19:00 Sat & Sun	TTC Closure	DVP Closure	Phase Mode (Fixed/Demanded/Callable)	Remarks
	Local Plan System Plan	Pattern 1 Plan 1	Pattern 2 Plan 2	Pattern 3 Plan 3	Pattern 4 Plan 4	Pattern 5 Plan 5	Pattern 6 Plan 6	Pattern 16 Plan 16		
1		WLK FDW MIN MAX1 AMB ALR SPLIT								Pedestrian Minimums: EWWK = 8 sec, EWFD = 11 sec NSWK = 8 sec, NSFDD = 17 sec NS phase is callable by vehicle and/or pedestrian actuation. If a vehicle and/or pedestrian call is received, the maximum NSG would be served. The NSWK & NSFDD are only displayed on the pedestrian signal heads if a vehicle and/or pedestrian call is received.
2		DLY GRN 5 WLK 8 FDW 11 MIN 14 MAX1 39 AMB 3.0 ALR 4.0 SPLIT	51	57	57	39	51	61	58	Fixed Split shown includes 5 sec of EW LPI Side Street Passage Time = 3 sec APS on during 7 secs of EWWK & NSWK when activated by pushbuttons. Leading Pedestrian Interval in all directions - EWWK comes up 5 seconds before EW vehicle green. NSWK comes up 5 seconds before NS vehicle green.
3		WLK FDW MIN MAX1 AMB ALR SPLIT								The following grades were used to calculate the AMB intervals: East Leg = 0.6% West Leg = 0.0% North Leg = -0.6% South Leg = -2.0%
4		DLY GRN 5 WLK 8 FDW 17 MIN 20 MAX1 20 AMB 3.0 ALR 4.7 SPLIT	33	33	33	33	33	33	36	Callable by Stopbar Loop and/or Push Button Split shown includes 5 sec of NS LPI
5		WLK FDW MIN MAX1 AMB ALR SPLIT								
6		DLY GRN 5 WLK 8 FDW 11 MIN 14 MAX1 39 AMB 3.0 ALR 4.0 SPLIT	51	57	57	39	51	61	58	Fixed Split shown includes 5 sec of EW LPI
7		WLK FDW MIN MAX1 AMB ALR SPLIT								
8		DLY GRN 5 WLK 8 FDW 17 MIN 20 MAX1 20 AMB 3.0 ALR 4.7 SPLIT	33	33	33	33	33	33	36	Callable by Stopbar Loop and/or Push Button Split shown includes 5 sec of NS LPI
	CL OF		84 37	90 28	90 85	72 57	84 66	94 1	94 66	

NOTES: NS Amber and All-red values updated to reflect NS speed limit reduction to 30 km/h.

Appendix H:

Synchro Analysis Reports

Timings

1: Main Street & Danforth Avenue

EX AM
Existing AM

									
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations									
Traffic Volume (vph)	10	380	180	540	65	80	240	45	205
Future Volume (vph)	10	380	180	540	65	80	240	45	205
Turn Type	Perm	NA	pm+pt	NA	Perm	Perm	NA	Perm	NA
Protected Phases		4	3	8			2		6
Permitted Phases	4		8		8	2		6	
Detector Phase	4	4	3	8	8	2	2	6	6
Switch Phase									
Minimum Initial (s)	30.0	30.0	6.0	30.0	30.0	20.0	20.0	20.0	20.0
Minimum Split (s)	36.7	36.7	13.4	36.7	36.7	31.7	31.7	31.7	31.7
Total Split (s)	38.0	38.0	18.0	56.0	56.0	34.0	34.0	34.0	34.0
Total Split (%)	42.2%	42.2%	20.0%	62.2%	62.2%	37.8%	37.8%	37.8%	37.8%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.7	3.7	4.4	3.7	3.7	3.7	3.7	3.7	3.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	6.7	6.7	7.4	6.7	6.7		6.7		6.7
Lead/Lag	Lag	Lag	Lead						
Lead-Lag Optimize?	Yes	Yes	Yes						
Recall Mode	Min	Min	Min	None	None	C-Min	C-Min	C-Min	C-Min
Act Effect Green (s)	31.1	31.1	47.8	48.5	48.5		28.1		28.1
Actuated g/C Ratio	0.35	0.35	0.53	0.54	0.54		0.31		0.31
v/c Ratio	0.04	0.82	0.56	0.57	0.11		0.64		0.38
Control Delay	15.8	40.8	17.8	16.3	3.0		26.7		25.5
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	15.8	40.8	17.8	16.3	3.0		26.7		25.5
LOS	B	D	B	B	A		C		C
Approach Delay		40.3		15.5			26.7		25.5
Approach LOS		D		B			C		C

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 89 (99%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.82

Intersection Signal Delay: 25.5

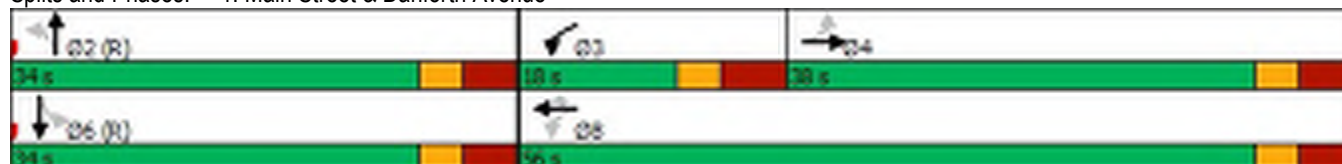
Intersection LOS: C

Intersection Capacity Utilization 117.4%

ICU Level of Service H

Analysis Period (min) 15

Splits and Phases: 1: Main Street & Danforth Avenue

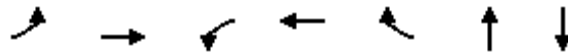


Queues

1: Main Street & Danforth Avenue

EX AM

Existing AM



Lane Group	EBL	EBT	WBL	WBT	WBR	NBT	SBT
Lane Group Flow (vph)	10	490	186	557	67	478	278
v/c Ratio	0.04	0.82	0.56	0.57	0.11	0.64	0.38
Control Delay	15.8	40.8	17.8	16.3	3.0	26.7	25.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	15.8	40.8	17.8	16.3	3.0	26.7	25.5
Queue Length 50th (m)	1.7	93.0	16.5	62.0	0.0	32.5	20.0
Queue Length 95th (m)	m3.3	#135.9	28.1	92.3	5.8	50.4	31.6
Internal Link Dist (m)		68.6		134.2		339.1	295.4
Turn Bay Length (m)	30.0						
Base Capacity (vph)	253	604	342	1011	599	763	754
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.04	0.81	0.54	0.55	0.11	0.63	0.37

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

1: Main Street & Danforth Avenue

EX AM

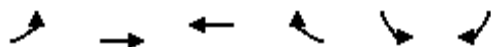
Existing AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	10	380	95	180	540	65	80	240	145	45	205	20
Future Volume (vph)	10	380	95	180	540	65	80	240	145	45	205	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	6.7	6.7		7.4	6.7	6.7		6.7			6.7	
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00		0.95			0.95	
Frpb, ped/bikes	1.00	0.97		1.00	1.00	0.82		0.92			0.95	
Flpb, ped/bikes	0.92	1.00		0.99	1.00	1.00		0.92			0.98	
Frt	1.00	0.97		1.00	1.00	0.85		0.95			0.99	
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.99			0.99	
Satd. Flow (prot)	1498	1690		1687	1827	1028		2682			2896	
Flt Permitted	0.46	1.00		0.20	1.00	1.00		0.82			0.80	
Satd. Flow (perm)	718	1690		355	1827	1028		2221			2347	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	10	392	98	186	557	67	82	247	149	46	211	21
RTOR Reduction (vph)	0	10	0	0	0	31	0	57	0	0	7	0
Lane Group Flow (vph)	10	480	0	186	557	36	0	421	0	0	271	0
Confl. Peds. (#/hr)	185		173	173		185	768		139	139		768
Confl. Bikes (#/hr)			6			2						6
Heavy Vehicles (%)	10%	6%	3%	5%	4%	28%	3%	8%	5%	40%	7%	4%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	6	6	0	6	6
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4			8		8	2			6		
Actuated Green, G (s)	31.2	31.2		48.5	48.5	48.5		28.1			28.1	
Effective Green, g (s)	31.2	31.2		48.5	48.5	48.5		28.1			28.1	
Actuated g/C Ratio	0.35	0.35		0.54	0.54	0.54		0.31			0.31	
Clearance Time (s)	6.7	6.7		7.4	6.7	6.7		6.7			6.7	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	248	585		337	984	553		693			732	
v/s Ratio Prot		c0.28		0.06	c0.30							
v/s Ratio Perm	0.01			0.24		0.04		c0.19			0.12	
v/c Ratio	0.04	0.82		0.55	0.57	0.07		0.61			0.37	
Uniform Delay, d1	19.5	26.8		14.4	13.8	9.9		26.3			24.1	
Progression Factor	0.80	1.07		1.00	1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.1	8.7		2.0	0.8	0.1		3.9			1.4	
Delay (s)	15.6	37.3		16.4	14.5	10.0		30.2			25.5	
Level of Service	B	D		B	B	A		C			C	
Approach Delay (s)		36.9			14.6			30.2			25.5	
Approach LOS		D			B			C			C	
Intersection Summary												
HCM 2000 Control Delay			25.1				HCM 2000 Level of Service			C		
HCM 2000 Volume to Capacity ratio			0.74									
Actuated Cycle Length (s)			90.0				Sum of lost time (s)		20.8			
Intersection Capacity Utilization			117.4%				ICU Level of Service		H			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis

2: Danforth Avenue & Chisholm Avenue

EX AM
Existing AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	15	475	600	40	10	30
Future Volume (Veh/h)	15	475	600	40	10	30
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	16	500	632	42	11	32
Pedestrians		6	2		125	
Lane Width (m)		3.6	3.5		3.6	
Walking Speed (m/s)		1.2	1.2		1.2	
Percent Blockage		1	0		10	
Right turn flare (veh)						
Median type		None	None			
Median storage veh						
Upstream signal (m)		172	93			
pX, platoon unblocked	0.80				0.85	0.80
vC, conflicting volume	799				1291	763
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	620				982	575
tC, single (s)	4.1				6.5	6.3
tC, 2 stage (s)						
tF (s)	2.2				3.6	3.4
p0 queue free %	98				94	91
cM capacity (veh/h)	692				199	362
Direction, Lane #	EB 1	WB 1	WB 2	SB 1		
Volume Total	516	632	42	43		
Volume Left	16	0	0	11		
Volume Right	0	0	42	32		
cSH	692	1700	1700	299		
Volume to Capacity	0.02	0.37	0.02	0.14		
Queue Length 95th (m)	0.6	0.0	0.0	4.0		
Control Delay (s)	0.6	0.0	0.0	19.0		
Lane LOS	A			C		
Approach Delay (s)	0.6	0.0		19.0		
Approach LOS				C		
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utilization			48.9%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

3: Danforth East Site Access & Danforth Avenue

EX AM
Existing AM

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (veh/h)	470	15	20	610	10	20
Future Volume (Veh/h)	470	15	20	610	10	20
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	505	16	22	656	11	22
Pedestrians	7			9	149	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	1			1	12	
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (m)	136			128		
pX, platoon unblocked				0.89	0.85	0.89
vC, conflicting volume				670	1369	671
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				564	1048	565
tC, single (s)				4.1	6.4	6.2
tC, 2 stage (s)						
tF (s)				2.2	3.5	3.3
p0 queue free %				97	94	95
cM capacity (veh/h)				790	184	407
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	521	678	33			
Volume Left	0	22	11			
Volume Right	16	0	22			
cSH	1700	790	290			
Volume to Capacity	0.31	0.03	0.11			
Queue Length 95th (m)	0.0	0.7	3.0			
Control Delay (s)	0.0	0.7	19.0			
Lane LOS	A		C			
Approach Delay (s)	0.0	0.7	19.0			
Approach LOS	C					
Intersection Summary						
Average Delay	0.9					
Intersection Capacity Utilization	60.8%			ICU Level of Service	B	
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

4: Danforth West Site Access & Danforth Avenue

EX AM
Existing AM

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (veh/h)	480	5	0	620	0	5
Future Volume (Veh/h)	480	5	0	620	0	5
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	516	5	0	667	0	5
Pedestrians	2					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (m)	49			215		
pX, platoon unblocked			0.88		0.86	0.88
vC, conflicting volume			521		1188	518
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			392		833	389
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	99
cM capacity (veh/h)			1040		294	586
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	521	667	5			
Volume Left	0	0	0			
Volume Right	5	0	5			
cSH	1700	1040	586			
Volume to Capacity	0.31	0.00	0.01			
Queue Length 95th (m)	0.0	0.0	0.2			
Control Delay (s)	0.0	0.0	11.2			
Lane LOS				B		
Approach Delay (s)	0.0	0.0	11.2			
Approach LOS				B		
Intersection Summary						
Average Delay				0.0		
Intersection Capacity Utilization	42.6%			ICU Level of Service		A
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

5: Westlake Avenue & Westlake Site Access

EX AM
Existing AM

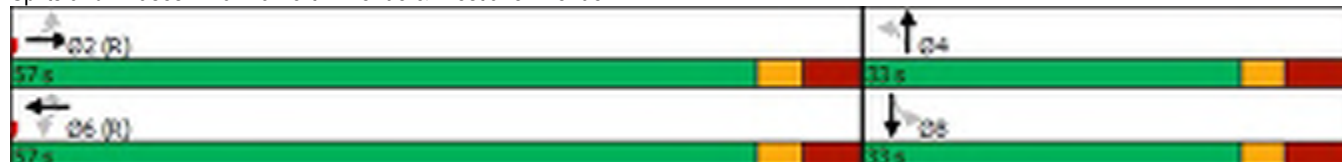
						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	0	5	30	0	5	25
Future Volume (Veh/h)	0	5	30	0	5	25
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	5	33	0	5	27
Pedestrians	20		7			2
Lane Width (m)	3.6		3.6			3.6
Walking Speed (m/s)	1.2		1.2			1.2
Percent Blockage	2		1			0
Right turn flare (veh)						
Median type			None			None
Median storage veh						
Upstream signal (m)						43
pX, platoon unblocked						
vC, conflicting volume	97	55			53	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	97	55			53	
tC, single (s)	6.4	6.2			4.3	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.4	
p0 queue free %	100	99			100	
cM capacity (veh/h)	884	999			1421	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	5	33	32			
Volume Left	0	0	5			
Volume Right	5	0	0			
cSH	999	1700	1421			
Volume to Capacity	0.01	0.02	0.00			
Queue Length 95th (m)	0.1	0.0	0.1			
Control Delay (s)	8.6	0.0	1.2			
Lane LOS	A		A			
Approach Delay (s)	8.6	0.0	1.2			
Approach LOS	A					
Intersection Summary						
Average Delay			1.2			
Intersection Capacity Utilization			18.8%	ICU Level of Service		A
Analysis Period (min)			15			

Timings 6: Danforth Avenue & Westlake Avenue

EX AM
Existing AM

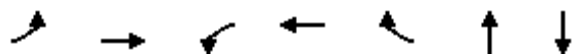
									
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations									
Traffic Volume (vph)	20	455	15	585	20	20	10	20	5
Future Volume (vph)	20	455	15	585	20	20	10	20	5
Turn Type	Perm	NA	Perm	NA	Perm	Perm	NA	Perm	NA
Protected Phases		2		6			4		8
Permitted Phases	2		6		6	4		8	
Detector Phase	2	2	6	6	6	4	4	8	8
Switch Phase									
Minimum Initial (s)	14.0	14.0	14.0	14.0	14.0	20.0	20.0	20.0	20.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	32.7	32.7	32.7	32.7
Total Split (s)	57.0	57.0	57.0	57.0	57.0	33.0	33.0	33.0	33.0
Total Split (%)	63.3%	63.3%	63.3%	63.3%	63.3%	36.7%	36.7%	36.7%	36.7%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	4.0	4.0	4.0	4.0	4.0	4.7	4.7	4.7	4.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0		7.7		7.7
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	C-Min	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	69.2	69.2	69.2	69.2	69.2		20.0		20.0
Actuated g/C Ratio	0.77	0.77	0.77	0.77	0.77		0.22		0.22
v/c Ratio	0.04	0.37	0.03	0.44	0.02		0.14		0.16
Control Delay	15.4	17.5	11.6	12.9	11.1		23.9		18.3
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	15.4	17.5	11.6	12.9	11.1		23.9		18.3
LOS	B	B	B	B	B		C		B
Approach Delay		17.4		12.8			23.8		18.3
Approach LOS		B		B			C		B
Intersection Summary									
Cycle Length: 90									
Actuated Cycle Length: 90									
Offset: 28 (31%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green									
Natural Cycle: 65									
Control Type: Actuated-Coordinated									
Maximum v/c Ratio: 0.44									
Intersection Signal Delay: 15.3					Intersection LOS: B				
Intersection Capacity Utilization 64.8%					ICU Level of Service C				
Analysis Period (min) 15									

Splits and Phases: 6: Danforth Avenue & Westlake Avenue



Queues
6: Danforth Avenue & Westlake Avenue

EX AM
Existing AM



Lane Group	EBL	EBT	WBL	WBT	WBR	NBT	SBT
Lane Group Flow (vph)	21	500	16	622	21	43	53
v/c Ratio	0.04	0.37	0.03	0.44	0.02	0.14	0.16
Control Delay	15.4	17.5	11.6	12.9	11.1	23.9	18.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	15.4	17.5	11.6	12.9	11.1	23.9	18.3
Queue Length 50th (m)	2.9	87.0	1.5	75.2	2.0	4.7	3.8
Queue Length 95th (m)	m6.7	109.3	m2.9	95.1	m3.8	13.6	13.6
Internal Link Dist (m)		139.2		25.3		0.7	152.8
Turn Bay Length (m)	35.0		20.0		20.0		
Base Capacity (vph)	468	1361	526	1404	893	393	401
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.04	0.37	0.03	0.44	0.02	0.11	0.13

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

6: Danforth Avenue & Westlake Avenue

EX AM
Existing AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	20	455	15	15	585	20	20	10	10	20	5	25
Future Volume (vph)	20	455	15	15	585	20	20	10	10	20	5	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	7.0	7.0		7.0	7.0	7.0		7.7			7.7	
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	0.99		1.00	1.00	0.77		0.98			0.95	
Flpb, ped/bikes	0.93	1.00		0.91	1.00	1.00		0.97			0.98	
Frt	1.00	1.00		1.00	1.00	0.85		0.97			0.93	
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.98			0.98	
Satd. Flow (prot)	1491	1770		1406	1827	1162		1637			1562	
Flt Permitted	0.38	1.00		0.46	1.00	1.00		0.82			0.85	
Satd. Flow (perm)	598	1770		674	1827	1162		1372			1356	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	21	484	16	16	622	21	21	11	11	21	5	27
RTOR Reduction (vph)	0	1	0	0	0	0	0	10	0	0	23	0
Lane Group Flow (vph)	21	499	0	16	622	21	0	33	0	0	30	0
Confl. Peds. (#/hr)	90		87	87		90	38		28	28		38
Confl. Bikes (#/hr)									3			1
Heavy Vehicles (%)	11%	6%	8%	15%	4%	6%	8%	0%	0%	5%	0%	4%
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6		6	4			8		
Actuated Green, G (s)	63.3	63.3		63.3	63.3	63.3		12.0			12.0	
Effective Green, g (s)	63.3	63.3		63.3	63.3	63.3		12.0			12.0	
Actuated g/C Ratio	0.70	0.70		0.70	0.70	0.70		0.13			0.13	
Clearance Time (s)	7.0	7.0		7.0	7.0	7.0		7.7			7.7	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	420	1244		474	1284	817		182			180	
v/s Ratio Prot		0.28			c0.34							
v/s Ratio Perm	0.04			0.02		0.02		c0.02			0.02	
v/c Ratio	0.05	0.40		0.03	0.48	0.03		0.18			0.16	
Uniform Delay, d1	4.1	5.5		4.1	6.0	4.0		34.6			34.6	
Progression Factor	2.23	2.39		1.68	1.55	1.67		1.00			1.00	
Incremental Delay, d2	0.2	0.9		0.1	1.2	0.1		0.5			0.4	
Delay (s)	9.4	14.1		7.0	10.5	6.8		35.1			35.0	
Level of Service	A	B		A	B	A		D			C	
Approach Delay (s)		13.9			10.3			35.1			35.0	
Approach LOS		B			B			D			C	
Intersection Summary												
HCM 2000 Control Delay			13.6				HCM 2000 Level of Service			B		
HCM 2000 Volume to Capacity ratio			0.44									
Actuated Cycle Length (s)			90.0				Sum of lost time (s)			14.7		
Intersection Capacity Utilization			64.8%				ICU Level of Service			C		
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis 7: Westlake Avenue & Stephenson Avenue

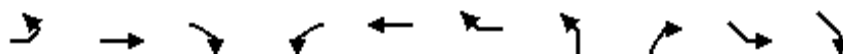
EX AM
Existing AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	0	15	0	5	10	15	0	15	5	10	10	5
Future Volume (vph)	0	15	0	5	10	15	0	15	5	10	10	5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	16	0	5	11	16	0	16	5	11	11	5
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	16	32	21	27								
Volume Left (vph)	0	5	0	11								
Volume Right (vph)	0	16	5	5								
Hadj (s)	0.00	-0.14	-0.01	0.10								
Departure Headway (s)	4.0	3.9	4.0	4.1								
Degree Utilization, x	0.02	0.03	0.02	0.03								
Capacity (veh/h)	875	912	873	858								
Control Delay (s)	7.1	7.0	7.1	7.3								
Approach Delay (s)	7.1	7.0	7.1	7.3								
Approach LOS	A	A	A	A								
Intersection Summary												
Delay				7.1								
Level of Service				A								
Intersection Capacity Utilization				30.2%	ICU Level of Service	A						
Analysis Period (min)				15								

HCM Unsignalized Intersection Capacity Analysis

8: Morton Road & Danforth Avenue & Oak Park Avenue

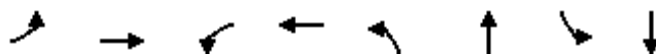
EX AM
Existing AM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBR	SEL	SER
Lane Configurations		↔			↔		↔		↔	
Traffic Volume (veh/h)	0	475	10	15	600	15	5	15	0	0
Future Volume (Veh/h)	0	475	10	15	600	15	5	15	0	0
Sign Control		Free			Free		Stop		Stop	
Grade		0%			0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	516	11	16	652	16	5	16	0	0
Pedestrians							90		150	
Lane Width (m)							3.6		3.6	
Walking Speed (m/s)							1.2		1.2	
Percent Blockage							8		12	
Right turn flare (veh)										
Median type		None			None					
Median storage (veh)										
Upstream signal (m)		143			163					
pX, platoon unblocked	0.83			0.86			0.90	0.86	0.90	0.90
vC, conflicting volume	818			617			1462	612	1382	1459
vC1, stage 1 conf vol										
vC2, stage 2 conf vol										
vCu, unblocked vol	681			476			1115	470	1027	1112
tC, single (s)	4.1			4.2			6.5	6.3	7.1	6.5
tC, 2 stage (s)										
tF (s)	2.2			2.3			4.0	3.4	3.5	4.0
p0 queue free %	100			98			97	97	100	100
cM capacity (veh/h)	671			846			150	465	133	149
Direction, Lane #	EB 1	WB 1	NB 1	SE 1						
Volume Total	527	684	21	0						
Volume Left	0	16	0	0						
Volume Right	11	16	16	0						
cSH	671	846	310	1700						
Volume to Capacity	0.00	0.02	0.07	0.00						
Queue Length 95th (m)	0.0	0.5	1.7	0.0						
Control Delay (s)	0.0	0.5	17.4	0.0						
Lane LOS		A	C	A						
Approach Delay (s)	0.0	0.5	17.4	0.0						
Approach LOS			C	A						
Intersection Summary										
Average Delay			0.6							
Intersection Capacity Utilization			54.8%		ICU Level of Service				A	
Analysis Period (min)			15							

Timings 9: Patricia Drive/Gledhill Avenue & Danforth Avenue

EX AM
Existing AM



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations								
Traffic Volume (vph)	30	440	10	615	60	20	30	10
Future Volume (vph)	30	440	10	615	60	20	30	10
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases		2		6		4		8
Permitted Phases	2		6		4		8	
Detector Phase	2	2	6	6	4	4	8	8
Switch Phase								
Minimum Initial (s)	19.0	19.0	19.0	19.0	23.0	23.0	23.0	23.0
Minimum Split (s)	35.1	35.1	35.1	35.1	35.1	35.1	35.1	35.1
Total Split (s)	54.0	54.0	54.0	54.0	36.0	36.0	36.0	36.0
Total Split (%)	60.0%	60.0%	60.0%	60.0%	40.0%	40.0%	40.0%	40.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.9	2.9	2.9	2.9	4.1	4.1	4.1	4.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	5.9	5.9	5.9	5.9		7.1		7.1
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effect Green (s)	61.2	61.2	61.2	61.2		23.0		23.0
Actuated g/C Ratio	0.68	0.68	0.68	0.68		0.26		0.26
v/c Ratio	0.09	0.43	0.02	0.31		0.33		0.25
Control Delay	8.5	10.4	3.4	5.1		27.4		18.0
Queue Delay	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	8.5	10.4	3.4	5.1		27.4		18.0
LOS	A	B	A	A		C		B
Approach Delay		10.3		5.0		27.4		18.0
Approach LOS		B		A		C		B

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 63 (70%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 75

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.43

Intersection Signal Delay: 9.3

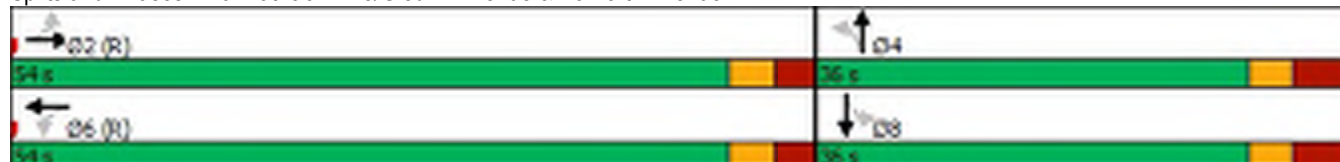
Intersection LOS: A

Intersection Capacity Utilization 59.1%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 9: Patricia Drive/Gledhill Avenue & Danforth Avenue



Queues

9: Patricia Drive/Gledhill Avenue & Danforth Avenue

EX AM

Existing AM



Lane Group	EBL	EBT	WBL	WBT	NBT	SBT
Lane Group Flow (vph)	33	511	11	716	101	83
v/c Ratio	0.09	0.43	0.02	0.31	0.33	0.25
Control Delay	8.5	10.4	3.4	5.1	27.4	18.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	8.5	10.4	3.4	5.1	27.4	18.0
Queue Length 50th (m)	2.4	47.7	0.4	28.7	13.1	6.2
Queue Length 95th (m)	6.6	72.2	m0.8	25.2	27.7	18.4
Internal Link Dist (m)		219.0		119.1	113.2	112.4
Turn Bay Length (m)	30.0		20.0			
Base Capacity (vph)	377	1189	461	2322	384	410
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.43	0.02	0.31	0.26	0.20

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

9: Patricia Drive/Gledhill Avenue & Danforth Avenue

EX AM
Existing AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	30	440	20	10	615	30	60	20	15	30	10	35
Future Volume (vph)	30	440	20	10	615	30	60	20	15	30	10	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	5.9	5.9		5.9	5.9			7.1			7.1	
Lane Util. Factor	1.00	1.00		1.00	0.95			1.00			1.00	
Frpb, ped/bikes	1.00	0.99		1.00	0.98			0.98			0.91	
Flpb, ped/bikes	0.86	1.00		0.94	1.00			0.90			0.97	
Frt	1.00	0.99		1.00	0.99			0.98			0.94	
Flt Protected	0.95	1.00		0.95	1.00			0.97			0.98	
Satd. Flow (prot)	1433	1746		1518	3413			1489			1398	
Flt Permitted	0.36	1.00		0.42	1.00			0.76			0.84	
Satd. Flow (perm)	549	1746		675	3413			1173			1197	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.97	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	33	489	22	11	683	33	62	22	17	33	11	39
RTOR Reduction (vph)	0	1	0	0	3	0	0	10	0	0	31	0
Lane Group Flow (vph)	33	510	0	11	713	0	0	91	0	0	52	0
Confl. Peds. (#/hr)	214		81	81		214	192		56	56		192
Confl. Bikes (#/hr)			2			3						
Heavy Vehicles (%)	7%	7%	14%	10%	3%	4%	3%	11%	15%	6%	22%	10%
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	58.6	58.6		58.6	58.6			18.4			18.4	
Effective Green, g (s)	58.6	58.6		58.6	58.6			18.4			18.4	
Actuated g/C Ratio	0.65	0.65		0.65	0.65			0.20			0.20	
Clearance Time (s)	5.9	5.9		5.9	5.9			7.1			7.1	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0			3.0	
Lane Grp Cap (vph)	357	1136		439	2222			239			244	
v/s Ratio Prot		c0.29			0.21							
v/s Ratio Perm	0.06			0.02				c0.08			0.04	
v/c Ratio	0.09	0.45		0.03	0.32			0.38			0.21	
Uniform Delay, d1	5.8	7.7		5.6	6.9			30.9			29.8	
Progression Factor	1.00	1.00		0.44	0.60			1.00			1.00	
Incremental Delay, d2	0.5	1.3		0.1	0.4			1.0			0.4	
Delay (s)	6.3	9.0		2.5	4.5			31.9			30.2	
Level of Service	A	A		A	A			C			C	
Approach Delay (s)		8.9			4.5			31.9			30.2	
Approach LOS		A			A			C			C	
Intersection Summary												
HCM 2000 Control Delay			9.5			HCM 2000 Level of Service				A		
HCM 2000 Volume to Capacity ratio			0.43									
Actuated Cycle Length (s)			90.0			Sum of lost time (s)			13.0			
Intersection Capacity Utilization			59.1%			ICU Level of Service			B			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis

10:

EX AM

Existing AM

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	5	0	0	35	30	5
Future Volume (Veh/h)	5	0	0	35	30	5
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	0	0	38	33	5
Pedestrians	1			1	17	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	0			0	1	
Right turn flare (veh)						
Median type				None	None	
Median storage veh						
Upstream signal (m)					25	
pX, platoon unblocked						
vC, conflicting volume	92	38	39			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	92	38	39			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	100	100			
cM capacity (veh/h)	900	1039	1583			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	5	38	38			
Volume Left	5	0	0			
Volume Right	0	0	5			
cSH	900	1583	1700			
Volume to Capacity	0.01	0.00	0.02			
Queue Length 95th (m)	0.1	0.0	0.0			
Control Delay (s)	9.0	0.0	0.0			
Lane LOS	A					
Approach Delay (s)	9.0	0.0	0.0			
Approach LOS	A					
Intersection Summary						
Average Delay			0.6			
Intersection Capacity Utilization		14.0%		ICU Level of Service		A
Analysis Period (min)		15				

Timings 1: Main Street & Danforth Avenue

EX PM
Existing PM

									
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations									
Traffic Volume (vph)	25	375	155	375	75	90	225	50	245
Future Volume (vph)	25	375	155	375	75	90	225	50	245
Turn Type	Perm	NA	pm+pt	NA	Perm	Perm	NA	Perm	NA
Protected Phases		4	3	8			2		6
Permitted Phases	4		8		8	2		6	
Detector Phase	4	4	3	8	8	2	2	6	6
Switch Phase									
Minimum Initial (s)	30.0	30.0	6.0	30.0	30.0	20.0	20.0	20.0	20.0
Minimum Split (s)	36.7	36.7	12.7	36.7	36.7	28.7	28.7	28.7	28.7
Total Split (s)	43.0	43.0	14.0	57.0	57.0	33.0	33.0	33.0	33.0
Total Split (%)	47.8%	47.8%	15.6%	63.3%	63.3%	36.7%	36.7%	36.7%	36.7%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	6.7	6.7	6.7	6.7	6.7		6.7		6.7
Lead/Lag	Lag	Lag	Lead						
Lead-Lag Optimize?	Yes	Yes	Yes						
Recall Mode	None	None	Min	None	None	C-Min	C-Min	C-Min	C-Min
Act Effect Green (s)	31.9	31.9	46.8	46.8	46.8		29.8		29.8
Actuated g/C Ratio	0.35	0.35	0.52	0.52	0.52		0.33		0.33
v/c Ratio	0.09	0.77	0.52	0.41	0.12		0.70		0.43
Control Delay	28.8	44.4	17.2	14.4	4.9		24.6		25.6
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	28.8	44.4	17.2	14.4	4.9		24.6		25.6
LOS	C	D	B	B	A		C		C
Approach Delay		43.6		13.9			24.6		25.6
Approach LOS		D		B			C		C

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 70 (78%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.77

Intersection Signal Delay: 26.1

Intersection LOS: C

Intersection Capacity Utilization 109.0%

ICU Level of Service H

Analysis Period (min) 15

Splits and Phases: 1: Main Street & Danforth Avenue

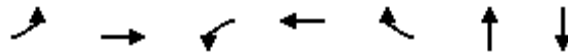


Queues

1: Main Street & Danforth Avenue

EX PM

Existing PM



Lane Group	EBL	EBT	WBL	WBT	WBR	NBT	SBT
Lane Group Flow (vph)	26	469	161	391	78	536	328
v/c Ratio	0.09	0.77	0.52	0.41	0.12	0.70	0.43
Control Delay	28.8	44.4	17.2	14.4	4.9	24.6	25.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	28.8	44.4	17.2	14.4	4.9	24.6	25.6
Queue Length 50th (m)	4.4	89.2	14.8	41.1	2.1	31.1	23.5
Queue Length 95th (m)	11.8	119.7	23.5	56.7	8.2	#55.5	38.4
Internal Link Dist (m)		68.6		134.2		339.1	295.4
Turn Bay Length (m)	30.0						
Base Capacity (vph)	346	696	312	1031	700	764	769
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.08	0.67	0.52	0.38	0.11	0.70	0.43

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: Main Street & Danforth Avenue

EX PM

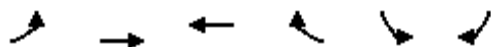
Existing PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	25	375	75	155	375	75	90	225	200	50	245	20
Future Volume (vph)	25	375	75	155	375	75	90	225	200	50	245	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	6.7	6.7		6.7	6.7	6.7		6.7			6.7	
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00		0.95			0.95	
Frpb, ped/bikes	1.00	0.96		1.00	1.00	0.77		0.83			0.96	
Flpb, ped/bikes	0.87	1.00		0.98	1.00	1.00		0.92			0.97	
Frt	1.00	0.98		1.00	1.00	0.85		0.94			0.99	
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.99			0.99	
Satd. Flow (prot)	1535	1707		1642	1845	1211		2450			2940	
Flt Permitted	0.53	1.00		0.23	1.00	1.00		0.80			0.78	
Satd. Flow (perm)	858	1707		401	1845	1211		1987			2309	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	26	391	78	161	391	78	94	234	208	52	255	21
RTOR Reduction (vph)	0	8	0	0	0	25	0	106	0	0	5	0
Lane Group Flow (vph)	26	461	0	161	391	53	0	430	0	0	323	0
Confl. Peds. (#/hr)	251		260	260		251	689		212	212		689
Confl. Bikes (#/hr)			1			2			3			2
Heavy Vehicles (%)	1%	1%	20%	7%	3%	1%	2%	8%	0%	31%	7%	4%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	6	6	0	6	6
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4			8		8	2			6		
Actuated Green, G (s)	31.9	31.9		46.8	46.8	46.8		29.8			29.8	
Effective Green, g (s)	31.9	31.9		46.8	46.8	46.8		29.8			29.8	
Actuated g/C Ratio	0.35	0.35		0.52	0.52	0.52		0.33			0.33	
Clearance Time (s)	6.7	6.7		6.7	6.7	6.7		6.7			6.7	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	304	605		321	959	629		657			764	
v/s Ratio Prot		c0.27		0.05	c0.21							
v/s Ratio Perm	0.03			0.21		0.04		c0.22			0.14	
v/c Ratio	0.09	0.76		0.50	0.41	0.08		0.65			0.42	
Uniform Delay, d1	19.3	25.7		14.4	13.2	10.8		25.7			23.4	
Progression Factor	1.53	1.42		1.00	1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.1	5.5		1.2	0.3	0.1		5.0			1.7	
Delay (s)	29.6	42.1		15.6	13.4	10.9		30.7			25.1	
Level of Service	C	D		B	B	B		C			C	
Approach Delay (s)		41.4			13.7			30.7			25.1	
Approach LOS		D			B			C			C	
Intersection Summary												
HCM 2000 Control Delay			27.1				HCM 2000 Level of Service			C		
HCM 2000 Volume to Capacity ratio			0.70									
Actuated Cycle Length (s)			90.0				Sum of lost time (s)		20.1			
Intersection Capacity Utilization			109.0%				ICU Level of Service			H		
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis

2: Danforth Avenue & Chisholm Avenue

EX PM
Existing PM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	15	455	455	30	20	25
Future Volume (Veh/h)	15	455	455	30	20	25
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	495	495	33	22	27
Pedestrians		4	6		192	
Lane Width (m)		3.6	3.5		3.6	
Walking Speed (m/s)		1.2	1.2		1.2	
Percent Blockage		0	0		16	
Right turn flare (veh)						
Median type		None	None			
Median storage veh						
Upstream signal (m)		172	93			
pX, platoon unblocked	0.87				0.92	0.87
vC, conflicting volume	720				1220	691
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	607				989	574
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	98				89	93
cM capacity (veh/h)	720				206	376
Direction, Lane #	EB 1	WB 1	WB 2	SB 1		
Volume Total	511	495	33	49		
Volume Left	16	0	0	22		
Volume Right	0	0	33	27		
cSH	720	1700	1700	275		
Volume to Capacity	0.02	0.29	0.02	0.18		
Queue Length 95th (m)	0.5	0.0	0.0	5.1		
Control Delay (s)	0.6	0.0	0.0	20.9		
Lane LOS	A			C		
Approach Delay (s)	0.6	0.0		20.9		
Approach LOS				C		
Intersection Summary						
Average Delay			1.2			
Intersection Capacity Utilization			47.3%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

3: Danforth East Site Access & Danforth Avenue

EX PM
Existing PM

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↱			↱	↘↗	
Traffic Volume (veh/h)	445	10	10	470	10	25
Future Volume (Veh/h)	445	10	10	470	10	25
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	468	11	11	495	11	26
Pedestrians	3			13	295	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	0			1	25	
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (m)	136			128		
pX, platoon unblocked			0.91		0.92	0.91
vC, conflicting volume			774		1288	782
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			701		1050	710
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			98		94	91
cM capacity (veh/h)			621		173	296
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	479	506	37			
Volume Left	0	11	11			
Volume Right	11	0	26			
cSH	1700	621	245			
Volume to Capacity	0.28	0.02	0.15			
Queue Length 95th (m)	0.0	0.4	4.2			
Control Delay (s)	0.0	0.5	22.3			
Lane LOS		A	C			
Approach Delay (s)	0.0	0.5	22.3			
Approach LOS			C			
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utilization			46.3%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

4: Danforth West Site Access & Danforth Avenue

EX PM
Existing PM

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↱			↱	↘↗	
Traffic Volume (veh/h)	450	5	5	475	0	5
Future Volume (Veh/h)	450	5	5	475	0	5
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Hourly flow rate (vph)	469	5	5	495	0	5
Pedestrians	4			2		
Lane Width (m)	3.6			3.6		
Walking Speed (m/s)	1.2			1.2		
Percent Blockage	0			0		
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (m)	49			215		
pX, platoon unblocked			0.90		0.94	0.90
vC, conflicting volume			474		980	474
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			365		720	365
tC, single (s)			4.1		6.4	6.4
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.5
p0 queue free %			100		100	99
cM capacity (veh/h)			1089		372	585
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	474	500	5			
Volume Left	0	5	0			
Volume Right	5	0	5			
cSH	1700	1089	585			
Volume to Capacity	0.28	0.00	0.01			
Queue Length 95th (m)	0.0	0.1	0.2			
Control Delay (s)	0.0	0.1	11.2			
Lane LOS		A	B			
Approach Delay (s)	0.0	0.1	11.2			
Approach LOS			B			
Intersection Summary						
Average Delay		0.1				
Intersection Capacity Utilization		39.6%	ICU Level of Service	A		
Analysis Period (min)		15				

HCM Unsignalized Intersection Capacity Analysis

5: Westlake Avenue & Westlake Site Access

EX PM
Existing PM

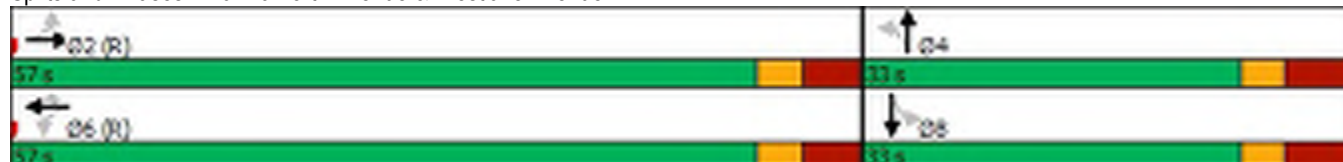
						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	5	10	35	5	10	20
Future Volume (Veh/h)	5	10	35	5	10	20
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	11	38	5	11	22
Pedestrians	34		6			6
Lane Width (m)	3.6		3.6			3.6
Walking Speed (m/s)	1.2		1.2			1.2
Percent Blockage	3		1			1
Right turn flare (veh)						
Median type			None			None
Median storage veh						
Upstream signal (m)						43
pX, platoon unblocked						
vC, conflicting volume	124	80			77	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	124	80			77	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	99	99			99	
cM capacity (veh/h)	840	952			1491	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	16	43	33			
Volume Left	5	0	11			
Volume Right	11	5	0			
cSH	914	1700	1491			
Volume to Capacity	0.02	0.03	0.01			
Queue Length 95th (m)	0.4	0.0	0.2			
Control Delay (s)	9.0	0.0	2.5			
Lane LOS	A		A			
Approach Delay (s)	9.0	0.0	2.5			
Approach LOS	A					
Intersection Summary						
Average Delay			2.5			
Intersection Capacity Utilization			21.9%	ICU Level of Service		A
Analysis Period (min)			15			

Timings 6: Danforth Avenue & Westlake Avenue

EX PM
Existing PM

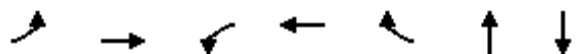
									
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations									
Traffic Volume (vph)	40	415	10	445	20	25	10	30	5
Future Volume (vph)	40	415	10	445	20	25	10	30	5
Turn Type	Perm	NA	Perm	NA	Perm	Perm	NA	Perm	NA
Protected Phases		2		6			4		8
Permitted Phases	2		6		6	4		8	
Detector Phase	2	2	6	6	6	4	4	8	8
Switch Phase									
Minimum Initial (s)	14.0	14.0	14.0	14.0	14.0	20.0	20.0	20.0	20.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	32.7	32.7	32.7	32.7
Total Split (s)	57.0	57.0	57.0	57.0	57.0	33.0	33.0	33.0	33.0
Total Split (%)	63.3%	63.3%	63.3%	63.3%	63.3%	36.7%	36.7%	36.7%	36.7%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	4.0	4.0	4.0	4.0	4.0	4.7	4.7	4.7	4.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0		7.7		7.7
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	C-Min	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	69.2	69.2	69.2	69.2	69.2		20.0		20.0
Actuated g/C Ratio	0.77	0.77	0.77	0.77	0.77		0.22		0.22
v/c Ratio	0.07	0.32	0.02	0.32	0.02		0.15		0.19
Control Delay	5.1	7.2	8.4	9.7	7.9		24.9		23.8
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	5.1	7.2	8.4	9.7	7.9		24.9		23.8
LOS	A	A	A	A	A		C		C
Approach Delay		7.0		9.6			24.9		23.8
Approach LOS		A		A			C		C
Intersection Summary									
Cycle Length: 90									
Actuated Cycle Length: 90									
Offset: 85 (94%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green									
Natural Cycle: 60									
Control Type: Actuated-Coordinated									
Maximum v/c Ratio: 0.32									
Intersection Signal Delay: 9.8					Intersection LOS: A				
Intersection Capacity Utilization 65.9%					ICU Level of Service C				
Analysis Period (min) 15									

Splits and Phases: 6: Danforth Avenue & Westlake Avenue



Queues
6: Danforth Avenue & Westlake Avenue

EX PM
Existing PM



Lane Group	EBL	EBT	WBL	WBT	WBR	NBT	SBT
Lane Group Flow (vph)	41	443	10	459	21	46	51
v/c Ratio	0.07	0.32	0.02	0.32	0.02	0.15	0.19
Control Delay	5.1	7.2	8.4	9.7	7.9	24.9	23.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	5.1	7.2	8.4	9.7	7.9	24.9	23.8
Queue Length 50th (m)	3.9	54.8	1.0	57.9	2.1	5.3	5.3
Queue Length 95th (m)	7.0	79.6	m2.7	86.1	m5.5	14.5	15.2
Internal Link Dist (m)		139.2		25.3		0.7	152.8
Turn Bay Length (m)	35.0		20.0		20.0		
Base Capacity (vph)	597	1395	610	1432	880	385	342
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.07	0.32	0.02	0.32	0.02	0.12	0.15

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

6: Danforth Avenue & Westlake Avenue

EX PM
Existing PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	40	415	15	10	445	20	25	10	10	30	5	15
Future Volume (vph)	40	415	15	10	445	20	25	10	10	30	5	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	7.0	7.0		7.0	7.0	7.0		7.7			7.7	
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	0.99		1.00	1.00	0.72		0.98			0.96	
Flpb, ped/bikes	0.87	1.00		0.84	1.00	1.00		0.93			0.96	
Frt	1.00	0.99		1.00	1.00	0.85		0.97			0.96	
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.97			0.97	
Satd. Flow (prot)	1502	1813		1499	1863	1145		1641			1454	
Flt Permitted	0.48	1.00		0.49	1.00	1.00		0.80			0.79	
Satd. Flow (perm)	762	1813		777	1863	1145		1344			1180	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	41	428	15	10	459	21	26	10	10	31	5	15
RTOR Reduction (vph)	0	1	0	0	0	0	0	9	0	0	13	0
Lane Group Flow (vph)	41	442	0	10	459	21	0	37	0	0	38	0
Confl. Peds. (#/hr)	113		131	131		113	70		36	36		70
Confl. Bikes (#/hr)						2			3			2
Heavy Vehicles (%)	3%	3%	6%	0%	2%	0%	0%	0%	0%	20%	0%	0%
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6		6	4			8		
Actuated Green, G (s)	63.3	63.3		63.3	63.3	63.3		12.0			12.0	
Effective Green, g (s)	63.3	63.3		63.3	63.3	63.3		12.0			12.0	
Actuated g/C Ratio	0.70	0.70		0.70	0.70	0.70		0.13			0.13	
Clearance Time (s)	7.0	7.0		7.0	7.0	7.0		7.7			7.7	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	535	1275		546	1310	805		179			157	
v/s Ratio Prot		0.24			c0.25							
v/s Ratio Perm	0.05			0.01		0.02		0.03			c0.03	
v/c Ratio	0.08	0.35		0.02	0.35	0.03		0.21			0.24	
Uniform Delay, d1	4.2	5.2		4.0	5.3	4.0		34.8			34.9	
Progression Factor	0.73	0.99		1.20	1.37	1.19		1.00			1.00	
Incremental Delay, d2	0.3	0.7		0.1	0.7	0.1		0.6			0.8	
Delay (s)	3.3	5.9		4.9	7.9	4.8		35.3			35.7	
Level of Service	A	A		A	A	A		D			D	
Approach Delay (s)		5.7			7.7			35.3			35.7	
Approach LOS		A			A			D			D	
Intersection Summary												
HCM 2000 Control Delay			9.3				HCM 2000 Level of Service			A		
HCM 2000 Volume to Capacity ratio			0.33									
Actuated Cycle Length (s)			90.0				Sum of lost time (s)			14.7		
Intersection Capacity Utilization			65.9%				ICU Level of Service			C		
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis 7: Westlake Avenue & Stephenson Avenue

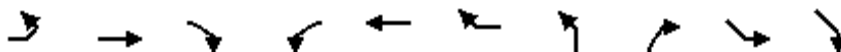
EX PM
Existing PM

																				
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR								
Lane Configurations																				
Sign Control		Stop			Stop			Stop			Stop									
Traffic Volume (vph)	5	30	0	10	20	15	5	20	15	20	0	5								
Future Volume (vph)	5	30	0	10	20	15	5	20	15	20	0	5								
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92								
Hourly flow rate (vph)	5	33	0	11	22	16	5	22	16	22	0	5								
Direction, Lane #	EB 1	WB 1	NB 1	SB 1																
Volume Total (vph)	38	49	43	27																
Volume Left (vph)	5	11	5	22																
Volume Right (vph)	0	16	16	5																
Hadj (s)	0.03	-0.15	-0.20	0.05																
Departure Headway (s)	4.1	3.9	3.9	4.2																
Degree Utilization, x	0.04	0.05	0.05	0.03																
Capacity (veh/h)	852	893	889	838																
Control Delay (s)	7.3	7.2	7.1	7.3																
Approach Delay (s)	7.3	7.2	7.1	7.3																
Approach LOS	A	A	A	A																
Intersection Summary																				
Delay			7.2																	
Level of Service			A																	
Intersection Capacity Utilization			29.2%	ICU Level of Service						A										
Analysis Period (min)			15																	

HCM Unsignalized Intersection Capacity Analysis

8: Morton Road & Danforth Avenue & Oak Park Avenue

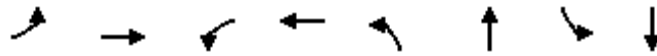
EX PM
Existing PM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBR	SEL	SER
Lane Configurations		↔			↔		↔		↔	
Traffic Volume (veh/h)	0	460	15	10	445	30	10	10	0	0
Future Volume (Veh/h)	0	460	15	10	445	30	10	10	0	0
Sign Control		Free			Free		Stop		Stop	
Grade		0%			0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	500	16	11	484	33	11	11	0	0
Pedestrians					9		122		107	
Lane Width (m)					3.6		3.6		3.6	
Walking Speed (m/s)					1.2		1.2		1.2	
Percent Blockage					1		10		9	
Right turn flare (veh)										
Median type		None			None					
Median storage (veh)										
Upstream signal (m)		143			163					
pX, platoon unblocked	0.90			0.87			0.92	0.87	0.92	0.92
vC, conflicting volume	624			638			1276	639	1163	1268
vC1, stage 1 conf vol										
vC2, stage 2 conf vol										
vCu, unblocked vol	528			513			1017	514	894	1008
tC, single (s)	4.1			4.1			6.5	6.3	7.1	6.5
tC, 2 stage (s)										
tF (s)	2.2			2.2			4.0	3.4	3.5	4.0
p0 queue free %	100			99			94	97	100	100
cM capacity (veh/h)	861			826			178	423	173	179
Direction, Lane #	EB 1	WB 1	NB 1	SE 1						
Volume Total	516	528	22	0						
Volume Left	0	11	0	0						
Volume Right	16	33	11	0						
cSH	861	826	251	1700						
Volume to Capacity	0.00	0.01	0.09	0.00						
Queue Length 95th (m)	0.0	0.3	2.3	0.0						
Control Delay (s)	0.0	0.4	20.7	0.0						
Lane LOS		A	C	A						
Approach Delay (s)	0.0	0.4	20.7	0.0						
Approach LOS			C	A						
Intersection Summary										
Average Delay			0.6							
Intersection Capacity Utilization			46.3%		ICU Level of Service				A	
Analysis Period (min)			15							

Timings 9: Patricia Drive/Gledhill Avenue & Danforth Avenue

EX PM
Existing PM



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations								
Traffic Volume (vph)	35	440	15	465	50	10	30	15
Future Volume (vph)	35	440	15	465	50	10	30	15
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases		2		6		4		8
Permitted Phases	2		6		4		8	
Detector Phase	2	2	6	6	4	4	8	8
Switch Phase								
Minimum Initial (s)	19.0	19.0	19.0	19.0	23.0	23.0	23.0	23.0
Minimum Split (s)	35.1	35.1	35.1	35.1	35.1	35.1	35.1	35.1
Total Split (s)	54.0	54.0	54.0	54.0	36.0	36.0	36.0	36.0
Total Split (%)	60.0%	60.0%	60.0%	60.0%	40.0%	40.0%	40.0%	40.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.9	2.9	2.9	2.9	4.1	4.1	4.1	4.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	5.9	5.9	5.9	5.9		7.1		7.1
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effect Green (s)	61.2	61.2	61.2	61.2		23.0		23.0
Actuated g/C Ratio	0.68	0.68	0.68	0.68		0.26		0.26
v/c Ratio	0.07	0.41	0.03	0.21		0.21		0.16
Control Delay	8.1	10.0	15.0	13.9		26.8		23.8
Queue Delay	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	8.1	10.0	15.0	13.9		26.8		23.8
LOS	A	B	B	B		C		C
Approach Delay		9.9		13.9		26.8		23.8
Approach LOS		A		B		C		C

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 43 (48%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 75

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.41

Intersection Signal Delay: 13.3

Intersection LOS: B

Intersection Capacity Utilization 62.7%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 9: Patricia Drive/Gledhill Avenue & Danforth Avenue





Lane Group	EBL	EBT	WBL	WBT	NBT	SBT
Lane Group Flow (vph)	36	500	16	500	67	57
v/c Ratio	0.07	0.41	0.03	0.21	0.21	0.16
Control Delay	8.1	10.0	15.0	13.9	26.8	23.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	8.1	10.0	15.0	13.9	26.8	23.8
Queue Length 50th (m)	2.5	45.3	2.2	38.0	8.9	6.6
Queue Length 95th (m)	6.7	68.5	m7.1	52.5	20.0	16.6
Internal Link Dist (m)		219.0		119.1	113.2	112.4
Turn Bay Length (m)	30.0		20.0			
Base Capacity (vph)	500	1228	504	2374	403	440
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.07	0.41	0.03	0.21	0.17	0.13

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

9: Patricia Drive/Gledhill Avenue & Danforth Avenue

EX PM
Existing PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	35	440	40	15	465	15	50	10	5	30	15	10
Future Volume (vph)	35	440	40	15	465	15	50	10	5	30	15	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	5.9	5.9		5.9	5.9			7.1			7.1	
Lane Util. Factor	1.00	1.00		1.00	0.95			1.00			1.00	
Frpb, ped/bikes	1.00	0.98		1.00	0.99			0.99			0.99	
Flpb, ped/bikes	0.86	1.00		0.91	1.00			0.96			0.98	
Frt	1.00	0.99		1.00	1.00			0.99			0.98	
Flt Protected	0.95	1.00		0.95	1.00			0.96			0.97	
Satd. Flow (prot)	1483	1803		1623	3490			1629			1618	
Flt Permitted	0.47	1.00		0.43	1.00			0.74			0.81	
Satd. Flow (perm)	732	1803		734	3490			1246			1352	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	36	458	42	16	484	16	52	10	5	31	16	10
RTOR Reduction (vph)	0	3	0	0	2	0	0	4	0	0	8	0
Lane Group Flow (vph)	36	497	0	16	498	0	0	63	0	0	49	0
Confl. Peds. (#/hr)	154		111	111		154	63		30	30		63
Confl. Bikes (#/hr)						3			1			
Heavy Vehicles (%)	3%	2%	3%	0%	2%	0%	2%	18%	20%	13%	0%	3%
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	58.6	58.6		58.6	58.6			18.4			18.4	
Effective Green, g (s)	58.6	58.6		58.6	58.6			18.4			18.4	
Actuated g/C Ratio	0.65	0.65		0.65	0.65			0.20			0.20	
Clearance Time (s)	5.9	5.9		5.9	5.9			7.1			7.1	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0			3.0	
Lane Grp Cap (vph)	476	1173		477	2272			254			276	
v/s Ratio Prot		c0.28			0.14							
v/s Ratio Perm	0.05			0.02				c0.05			0.04	
v/c Ratio	0.08	0.42		0.03	0.22			0.25			0.18	
Uniform Delay, d1	5.8	7.6		5.6	6.4			30.0			29.6	
Progression Factor	1.00	1.00		1.97	1.85			1.00			1.00	
Incremental Delay, d2	0.3	1.1		0.1	0.2			0.5			0.3	
Delay (s)	6.1	8.7		11.1	12.0			30.5			29.9	
Level of Service	A	A		B	B			C			C	
Approach Delay (s)		8.5			12.0			30.5			29.9	
Approach LOS		A			B			C			C	
Intersection Summary												
HCM 2000 Control Delay			12.3			HCM 2000 Level of Service				B		
HCM 2000 Volume to Capacity ratio			0.38									
Actuated Cycle Length (s)			90.0			Sum of lost time (s)			13.0			
Intersection Capacity Utilization			62.7%			ICU Level of Service			B			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis

10:

EX PM

Existing PM

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	0	0	0	45	30	0
Future Volume (Veh/h)	0	0	0	45	30	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	49	33	0
Pedestrians	6			3	34	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	1			0	3	
Right turn flare (veh)						
Median type				None	None	
Median storage veh						
Upstream signal (m)					25	
pX, platoon unblocked						
vC, conflicting volume	122	42	39			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	122	42	39			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	849	1027	1576			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	0	49	33			
Volume Left	0	0	0			
Volume Right	0	0	0			
cSH	1700	1576	1700			
Volume to Capacity	0.01	0.00	0.02			
Queue Length 95th (m)	0.0	0.0	0.0			
Control Delay (s)	0.0	0.0	0.0			
Lane LOS	A					
Approach Delay (s)	0.0	0.0	0.0			
Approach LOS	A					
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization		16.1%		ICU Level of Service		A
Analysis Period (min)		15				

Timings

1: Main Street & Danforth Avenue

FB AM
Future Background AM

									
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations									
Traffic Volume (vph)	20	410	245	710	80	80	265	45	225
Future Volume (vph)	20	410	245	710	80	80	265	45	225
Turn Type	Perm	NA	pm+pt	NA	Perm	Perm	NA	Perm	NA
Protected Phases		4	3	8			2		6
Permitted Phases	4		8		8	2		6	
Detector Phase	4	4	3	8	8	2	2	6	6
Switch Phase									
Minimum Initial (s)	30.0	30.0	6.0	30.0	30.0	20.0	20.0	20.0	20.0
Minimum Split (s)	36.7	36.7	13.4	36.7	36.7	31.7	31.7	31.7	31.7
Total Split (s)	38.0	38.0	18.0	56.0	56.0	34.0	34.0	34.0	34.0
Total Split (%)	42.2%	42.2%	20.0%	62.2%	62.2%	37.8%	37.8%	37.8%	37.8%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.7	3.7	4.4	3.7	3.7	3.7	3.7	3.7	3.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	6.7	6.7	7.4	6.7	6.7		6.7		6.7
Lead/Lag	Lag	Lag	Lead						
Lead-Lag Optimize?	Yes	Yes	Yes						
Recall Mode	Min	Min	Min	None	None	C-Min	C-Min	C-Min	C-Min
Act Effect Green (s)	31.2	31.2	49.2	49.9	49.9		26.7		26.7
Actuated g/C Ratio	0.35	0.35	0.55	0.55	0.55		0.30		0.30
v/c Ratio	0.10	0.87	0.75	0.72	0.14		0.72		0.43
Control Delay	19.1	43.7	28.6	20.0	2.9		29.9		27.4
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	19.1	43.7	28.6	20.0	2.9		29.9		27.4
LOS	B	D	C	B	A		C		C
Approach Delay		42.8		20.7			29.9		27.4
Approach LOS		D		C			C		C

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 89 (99%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.87

Intersection Signal Delay: 28.4

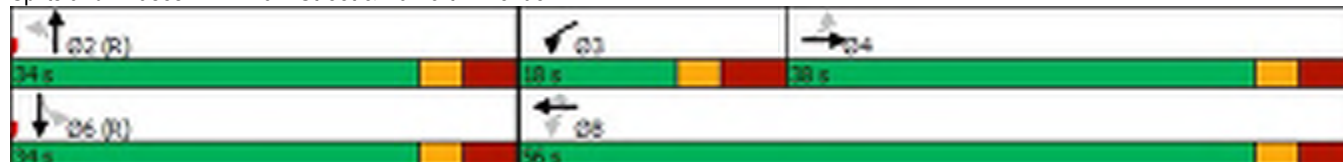
Intersection LOS: C

Intersection Capacity Utilization 126.4%

ICU Level of Service H

Analysis Period (min) 15

Splits and Phases: 1: Main Street & Danforth Avenue

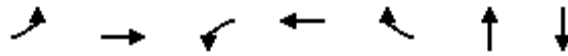


Queues

FB AM

1: Main Street & Danforth Avenue

Future Background AM



Lane Group	EBL	EBT	WBL	WBT	WBR	NBT	SBT
Lane Group Flow (vph)	21	521	253	732	82	525	299
v/c Ratio	0.10	0.87	0.75	0.72	0.14	0.72	0.43
Control Delay	19.1	43.7	28.6	20.0	2.9	29.9	27.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	19.1	43.7	28.6	20.0	2.9	29.9	27.4
Queue Length 50th (m)	2.8	75.1	20.8	83.7	0.0	39.3	23.5
Queue Length 95th (m)	m5.3	#150.7	#57.9	140.6	6.3	56.0	34.3
Internal Link Dist (m)		68.6		134.2		339.1	295.4
Turn Bay Length (m)	30.0						
Base Capacity (vph)	204	605	339	1021	611	753	715
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.10	0.86	0.75	0.72	0.13	0.70	0.42

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

FB AM

1: Main Street & Danforth Avenue

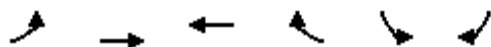
Future Background AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	20	410	95	245	710	80	80	265	165	45	225	20
Future Volume (vph)	20	410	95	245	710	80	80	265	165	45	225	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	6.7	6.7		7.4	6.7	6.7		6.7			6.7	
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00		0.95			0.95	
Frpb, ped/bikes	1.00	0.97		1.00	1.00	0.82		0.91			0.95	
Flpb, ped/bikes	0.94	1.00		0.99	1.00	1.00		0.93			0.98	
Frt	1.00	0.97		1.00	1.00	0.85		0.95			0.99	
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.99			0.99	
Satd. Flow (prot)	1528	1696		1690	1827	1028		2695			2930	
Flt Permitted	0.36	1.00		0.17	1.00	1.00		0.83			0.78	
Satd. Flow (perm)	581	1696		303	1827	1028		2241			2302	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	21	423	98	253	732	82	82	273	170	46	232	21
RTOR Reduction (vph)	0	9	0	0	0	37	0	65	0	0	6	0
Lane Group Flow (vph)	21	512	0	253	732	45	0	460	0	0	293	0
Confl. Peds. (#/hr)	185		173	173		185	768		139	139		768
Confl. Bikes (#/hr)			6			2						6
Heavy Vehicles (%)	10%	6%	3%	5%	4%	28%	3%	8%	5%	40%	7%	4%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	6	6	0	6	6
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4			8		8	2			6		
Actuated Green, G (s)	31.2	31.2		49.9	49.9	49.9		26.7			26.7	
Effective Green, g (s)	31.2	31.2		49.9	49.9	49.9		26.7			26.7	
Actuated g/C Ratio	0.35	0.35		0.55	0.55	0.55		0.30			0.30	
Clearance Time (s)	6.7	6.7		7.4	6.7	6.7		6.7			6.7	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	201	587		342	1012	569		664			682	
v/s Ratio Prot		c0.30		0.09	c0.40							
v/s Ratio Perm	0.04			0.32		0.04		c0.21			0.13	
v/c Ratio	0.10	0.87		0.74	0.72	0.08		0.69			0.43	
Uniform Delay, d1	19.9	27.5		15.3	14.9	9.3		28.0			25.5	
Progression Factor	0.89	1.00		1.00	1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.2	12.8		8.1	2.6	0.1		5.9			2.0	
Delay (s)	17.9	40.3		23.5	17.5	9.4		33.9			27.5	
Level of Service	B	D		C	B	A		C			C	
Approach Delay (s)		39.5			18.3			33.9			27.5	
Approach LOS		D			B			C			C	
Intersection Summary												
HCM 2000 Control Delay			27.5									
HCM 2000 Volume to Capacity ratio			0.83									
Actuated Cycle Length (s)			90.0									
Intersection Capacity Utilization			126.4%									
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis

2: Danforth Avenue & Chisholm Avenue

FB AM
Future Background AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	15	515	770	40	10	30
Future Volume (Veh/h)	15	515	770	40	10	30
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	16	542	811	42	11	32
Pedestrians		6	2		125	
Lane Width (m)		3.6	3.5		3.6	
Walking Speed (m/s)		1.2	1.2		1.2	
Percent Blockage		1	0		10	
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (m)		172	93			
pX, platoon unblocked	0.69				0.75	0.69
vC, conflicting volume	978				1512	942
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	746				1153	694
tC, single (s)	4.1				6.5	6.3
tC, 2 stage (s)						
tF (s)	2.2				3.6	3.4
p0 queue free %	97				92	88
cM capacity (veh/h)	540				138	269
Direction, Lane #	EB 1	WB 1	WB 2	SB 1		
Volume Total	558	811	42	43		
Volume Left	16	0	0	11		
Volume Right	0	0	42	32		
cSH	540	1700	1700	217		
Volume to Capacity	0.03	0.48	0.02	0.20		
Queue Length 95th (m)	0.7	0.0	0.0	5.8		
Control Delay (s)	0.8	0.0	0.0	25.7		
Lane LOS	A			D		
Approach Delay (s)	0.8	0.0		25.7		
Approach LOS				D		
Intersection Summary						
Average Delay		1.1				
Intersection Capacity Utilization		52.3%		ICU Level of Service		A
Analysis Period (min)		15				

HCM Unsignalized Intersection Capacity Analysis

3: Danforth East Site Access & Danforth Avenue

FB AM
Future Background AM

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↰			↱	↰	↱
Traffic Volume (veh/h)	510	15	20	780	10	20
Future Volume (Veh/h)	510	15	20	780	10	20
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	548	16	22	839	11	22
Pedestrians	7			9	149	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	1			1	12	
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (m)	136			128		
pX, platoon unblocked			0.87		0.76	0.87
vC, conflicting volume			713		1595	714
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			594		1233	595
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			97		91	94
cM capacity (veh/h)			755		126	383
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	564	861	33			
Volume Left	0	22	11			
Volume Right	16	0	22			
cSH	1700	755	228			
Volume to Capacity	0.33	0.03	0.14			
Queue Length 95th (m)	0.0	0.7	4.0			
Control Delay (s)	0.0	0.8	23.5			
Lane LOS		A	C			
Approach Delay (s)	0.0	0.8	23.5			
Approach LOS			C			
Intersection Summary						
Average Delay		1.0				
Intersection Capacity Utilization		69.7%	ICU Level of Service		C	
Analysis Period (min)		15				

HCM Unsignalized Intersection Capacity Analysis

4: Danforth West Site Access & Danforth Avenue

FB AM
Future Background AM

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (veh/h)	520	5	0	790	0	5
Future Volume (Veh/h)	520	5	0	790	0	5
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	559	5	0	849	0	5
Pedestrians	2					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (m)	49			215		
pX, platoon unblocked			0.87	0.76		0.87
vC, conflicting volume			564	1412		562
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			419	986		416
tC, single (s)			4.1	6.4		6.2
tC, 2 stage (s)						
tF (s)			2.2	3.5		3.3
p0 queue free %			100	100		99
cM capacity (veh/h)			997	209		555
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	564	849	5			
Volume Left	0	0	0			
Volume Right	5	0	5			
cSH	1700	997	555			
Volume to Capacity	0.33	0.00	0.01			
Queue Length 95th (m)	0.0	0.0	0.2			
Control Delay (s)	0.0	0.0	11.5			
Lane LOS				B		
Approach Delay (s)	0.0	0.0	11.5			
Approach LOS				B		
Intersection Summary						
Average Delay				0.0		
Intersection Capacity Utilization	51.6%			ICU Level of Service		A
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

5: Westlake Avenue & Westlake Site Access

FB AM
Future Background AM

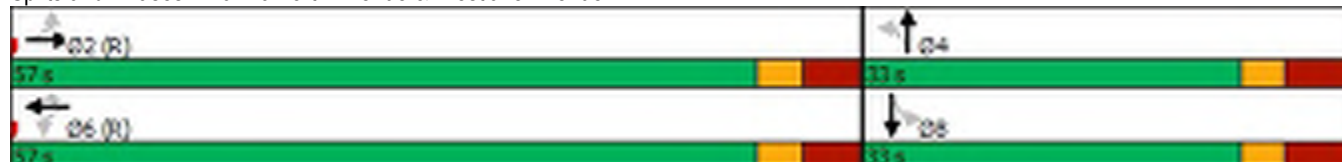
						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	0	5	40	0	5	30
Future Volume (Veh/h)	0	5	40	0	5	30
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	5	43	0	5	33
Pedestrians	20		7			2
Lane Width (m)	3.6		3.6			3.6
Walking Speed (m/s)	1.2		1.2			1.2
Percent Blockage	2		1			0
Right turn flare (veh)						
Median type			None			None
Median storage veh						
Upstream signal (m)						43
pX, platoon unblocked						
vC, conflicting volume	113	65			63	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	113	65			63	
tC, single (s)	6.4	6.2			4.3	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.4	
p0 queue free %	100	99			100	
cM capacity (veh/h)	865	986			1409	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	5	43	38			
Volume Left	0	0	5			
Volume Right	5	0	0			
cSH	986	1700	1409			
Volume to Capacity	0.01	0.03	0.00			
Queue Length 95th (m)	0.1	0.0	0.1			
Control Delay (s)	8.7	0.0	1.0			
Lane LOS	A		A			
Approach Delay (s)	8.7	0.0	1.0			
Approach LOS	A					
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utilization			18.8%	ICU Level of Service		A
Analysis Period (min)			15			

Timings 6: Danforth Avenue & Westlake Avenue

FB AM
Future Background AM

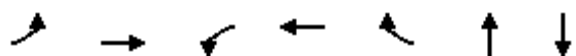
									
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations									
Traffic Volume (vph)	20	495	15	755	20	30	10	20	5
Future Volume (vph)	20	495	15	755	20	30	10	20	5
Turn Type	Perm	NA	Perm	NA	Perm	Perm	NA	Perm	NA
Protected Phases		2		6			4		8
Permitted Phases	2		6		6	4		8	
Detector Phase	2	2	6	6	6	4	4	8	8
Switch Phase									
Minimum Initial (s)	14.0	14.0	14.0	14.0	14.0	20.0	20.0	20.0	20.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	32.7	32.7	32.7	32.7
Total Split (s)	57.0	57.0	57.0	57.0	57.0	33.0	33.0	33.0	33.0
Total Split (%)	63.3%	63.3%	63.3%	63.3%	63.3%	36.7%	36.7%	36.7%	36.7%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	4.0	4.0	4.0	4.0	4.0	4.7	4.7	4.7	4.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0		7.7		7.7
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	C-Min	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	69.2	69.2	69.2	69.2	69.2		20.0		20.0
Actuated g/C Ratio	0.77	0.77	0.77	0.77	0.77		0.22		0.22
v/c Ratio	0.06	0.40	0.03	0.57	0.02		0.18		0.17
Control Delay	12.6	13.0	9.9	11.5	9.5		25.6		18.3
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	12.6	13.0	9.9	11.5	9.5		25.6		18.3
LOS	B	B	A	B	A		C		B
Approach Delay		12.9		11.4			25.6		18.3
Approach LOS		B		B			C		B
Intersection Summary									
Cycle Length: 90									
Actuated Cycle Length: 90									
Offset: 28 (31%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green									
Natural Cycle: 75									
Control Type: Actuated-Coordinated									
Maximum v/c Ratio: 0.57									
Intersection Signal Delay: 12.7					Intersection LOS: B				
Intersection Capacity Utilization 71.6%					ICU Level of Service C				
Analysis Period (min) 15									

Splits and Phases: 6: Danforth Avenue & Westlake Avenue



Queues
6: Danforth Avenue & Westlake Avenue

FB AM
Future Background AM



Lane Group	EBL	EBT	WBL	WBT	WBR	NBT	SBT
Lane Group Flow (vph)	21	548	16	803	21	54	53
v/c Ratio	0.06	0.40	0.03	0.57	0.02	0.18	0.17
Control Delay	12.6	13.0	9.9	11.5	9.5	25.6	18.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	12.6	13.0	9.9	11.5	9.5	25.6	18.3
Queue Length 50th (m)	2.1	68.4	1.3	87.4	1.7	6.3	3.8
Queue Length 95th (m)	m5.2	97.1	m2.0	100.5	m2.6	16.4	13.6
Internal Link Dist (m)		139.2		25.3		0.7	152.8
Turn Bay Length (m)	35.0		20.0		20.0		
Base Capacity (vph)	363	1358	496	1404	893	377	398
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.06	0.40	0.03	0.57	0.02	0.14	0.13

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

6: Danforth Avenue & Westlake Avenue

FB AM
Future Background AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	20	495	20	15	755	20	30	10	10	20	5	25
Future Volume (vph)	20	495	20	15	755	20	30	10	10	20	5	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	7.0	7.0		7.0	7.0	7.0		7.7			7.7	
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	0.99		1.00	1.00	0.77		0.98			0.95	
Flpb, ped/bikes	0.96	1.00		0.92	1.00	1.00		0.96			0.98	
Frt	1.00	0.99		1.00	1.00	0.85		0.97			0.93	
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.97			0.98	
Satd. Flow (prot)	1539	1766		1421	1827	1162		1623			1562	
Flt Permitted	0.28	1.00		0.43	1.00	1.00		0.79			0.85	
Satd. Flow (perm)	452	1766		636	1827	1162		1316			1349	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	21	527	21	16	803	21	32	11	11	21	5	27
RTOR Reduction (vph)	0	1	0	0	0	0	0	10	0	0	23	0
Lane Group Flow (vph)	21	547	0	16	803	21	0	44	0	0	30	0
Confl. Peds. (#/hr)	90		87	87		90	38		28	28		38
Confl. Bikes (#/hr)									3			1
Heavy Vehicles (%)	11%	6%	8%	15%	4%	6%	8%	0%	0%	5%	0%	4%
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6		6	4			8		
Actuated Green, G (s)	63.3	63.3		63.3	63.3	63.3		12.0			12.0	
Effective Green, g (s)	63.3	63.3		63.3	63.3	63.3		12.0			12.0	
Actuated g/C Ratio	0.70	0.70		0.70	0.70	0.70		0.13			0.13	
Clearance Time (s)	7.0	7.0		7.0	7.0	7.0		7.7			7.7	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	317	1242		447	1284	817		175			179	
v/s Ratio Prot		0.31			c0.44							
v/s Ratio Perm	0.05			0.03		0.02		c0.03			0.02	
v/c Ratio	0.07	0.44		0.04	0.63	0.03		0.25			0.17	
Uniform Delay, d1	4.2	5.7		4.1	7.1	4.0		35.0			34.6	
Progression Factor	1.75	1.65		1.42	1.10	1.43		1.00			1.00	
Incremental Delay, d2	0.4	1.0		0.1	1.8	0.0		0.8			0.4	
Delay (s)	7.6	10.5		5.9	9.5	5.8		35.8			35.0	
Level of Service	A	B		A	A	A		D			C	
Approach Delay (s)		10.4			9.4			35.8			35.0	
Approach LOS		B			A			D			C	
Intersection Summary												
HCM 2000 Control Delay			11.6				HCM 2000 Level of Service			B		
HCM 2000 Volume to Capacity ratio			0.57									
Actuated Cycle Length (s)			90.0				Sum of lost time (s)			14.7		
Intersection Capacity Utilization			71.6%				ICU Level of Service			C		
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis 7: Westlake Avenue & Stephenson Avenue

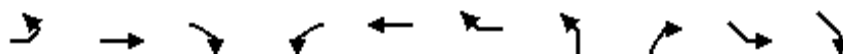
FB AM
Future Background AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control	Stop			Stop			Stop			Stop		
Traffic Volume (vph)	0	15	0	5	15	25	0	15	5	15	10	5
Future Volume (vph)	0	15	0	5	15	25	0	15	5	15	10	5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	16	0	5	16	27	0	16	5	16	11	5
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	16	48	21	32								
Volume Left (vph)	0	5	0	16								
Volume Right (vph)	0	27	5	5								
Hadj (s)	0.00	-0.22	-0.01	0.14								
Departure Headway (s)	4.1	3.8	4.1	4.2								
Degree Utilization, x	0.02	0.05	0.02	0.04								
Capacity (veh/h)	867	925	862	840								
Control Delay (s)	7.1	7.0	7.1	7.4								
Approach Delay (s)	7.1	7.0	7.1	7.4								
Approach LOS	A	A	A	A								
Intersection Summary												
Delay				7.2								
Level of Service				A								
Intersection Capacity Utilization				30.5%	ICU Level of Service	A						
Analysis Period (min)				15								

HCM Unsignalized Intersection Capacity Analysis

8: Morton Road & Danforth Avenue & Oak Park Avenue

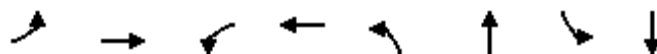
FB AM
Future Background AM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBR	SEL	SER
Lane Configurations		↔			↔		↔		↔	
Traffic Volume (veh/h)	0	520	10	15	780	15	5	15	0	0
Future Volume (Veh/h)	0	520	10	15	780	15	5	15	0	0
Sign Control		Free			Free		Stop		Stop	
Grade		0%			0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	565	11	16	848	16	5	16	0	0
Pedestrians							90		150	
Lane Width (m)							3.6		3.6	
Walking Speed (m/s)							1.2		1.2	
Percent Blockage							8		12	
Right turn flare (veh)										
Median type		None			None					
Median storage (veh)										
Upstream signal (m)		143			163					
pX, platoon unblocked	0.73			0.84			0.81	0.84	0.81	0.81
vC, conflicting volume	1014			666			1706	660	1627	1704
vC1, stage 1 conf vol										
vC2, stage 2 conf vol										
vCu, unblocked vol	835			506			1308	499	1210	1305
tC, single (s)	4.1			4.2			6.5	6.3	7.1	6.5
tC, 2 stage (s)										
tF (s)	2.2			2.3			4.0	3.4	3.5	4.0
p0 queue free %	100			98			95	96	100	100
cM capacity (veh/h)	516			802			104	436	88	103
Direction, Lane #	EB 1	WB 1	NB 1	SE 1						
Volume Total	576	880	21	0						
Volume Left	0	16	0	0						
Volume Right	11	16	16	0						
cSH	516	802	247	1700						
Volume to Capacity	0.00	0.02	0.08	0.00						
Queue Length 95th (m)	0.0	0.5	2.2	0.0						
Control Delay (s)	0.0	0.6	20.9	0.0						
Lane LOS		A	C	A						
Approach Delay (s)	0.0	0.6	20.9	0.0						
Approach LOS			C	A						
Intersection Summary										
Average Delay			0.6							
Intersection Capacity Utilization			64.1%		ICU Level of Service				C	
Analysis Period (min)			15							

Timings 9: Patricia Drive/Gledhill Avenue & Danforth Avenue

FB AM
Future Background AM



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations								
Traffic Volume (vph)	30	485	10	795	60	20	30	10
Future Volume (vph)	30	485	10	795	60	20	30	10
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases		2		6		4		8
Permitted Phases	2		6		4		8	
Detector Phase	2	2	6	6	4	4	8	8
Switch Phase								
Minimum Initial (s)	19.0	19.0	19.0	19.0	23.0	23.0	23.0	23.0
Minimum Split (s)	35.1	35.1	35.1	35.1	35.1	35.1	35.1	35.1
Total Split (s)	54.0	54.0	54.0	54.0	36.0	36.0	36.0	36.0
Total Split (%)	60.0%	60.0%	60.0%	60.0%	40.0%	40.0%	40.0%	40.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.9	2.9	2.9	2.9	4.1	4.1	4.1	4.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	5.9	5.9	5.9	5.9		7.1		7.1
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effect Green (s)	61.2	61.2	61.2	61.2		23.0		23.0
Actuated g/C Ratio	0.68	0.68	0.68	0.68		0.26		0.26
v/c Ratio	0.11	0.47	0.03	0.39		0.33		0.25
Control Delay	9.0	11.1	3.1	4.6		27.4		18.0
Queue Delay	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	9.0	11.1	3.1	4.6		27.4		18.0
LOS	A	B	A	A		C		B
Approach Delay		10.9		4.6		27.4		18.0
Approach LOS		B		A		C		B

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 63 (70%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 75

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.47

Intersection Signal Delay: 8.8

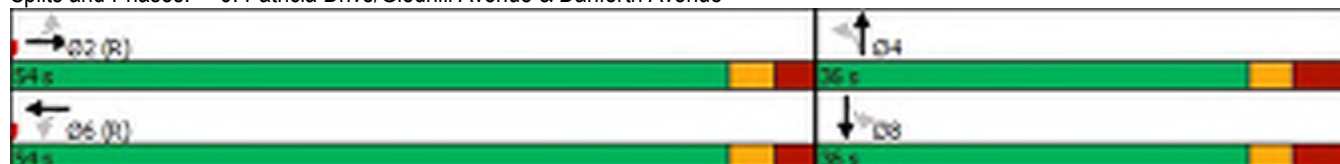
Intersection LOS: A

Intersection Capacity Utilization 61.1%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 9: Patricia Drive/Gledhill Avenue & Danforth Avenue





Lane Group	EBL	EBT	WBL	WBT	NBT	SBT
Lane Group Flow (vph)	33	561	11	916	101	83
v/c Ratio	0.11	0.47	0.03	0.39	0.33	0.25
Control Delay	9.0	11.1	3.1	4.6	27.4	18.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	9.0	11.1	3.1	4.6	27.4	18.0
Queue Length 50th (m)	2.4	54.6	0.1	8.0	13.1	6.2
Queue Length 95th (m)	6.9	82.4	m0.5	13.7	27.7	18.4
Internal Link Dist (m)		219.0		119.1	113.2	112.4
Turn Bay Length (m)	30.0		20.0			
Base Capacity (vph)	306	1191	429	2336	384	410
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.47	0.03	0.39	0.26	0.20

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

9: Patricia Drive/Gledhill Avenue & Danforth Avenue

FB AM
Future Background AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	30	485	20	10	795	30	60	20	15	30	10	35
Future Volume (vph)	30	485	20	10	795	30	60	20	15	30	10	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	5.9	5.9		5.9	5.9			7.1			7.1	
Lane Util. Factor	1.00	1.00		1.00	0.95			1.00			1.00	
Frpb, ped/bikes	1.00	0.99		1.00	0.99			0.98			0.91	
Flpb, ped/bikes	0.90	1.00		0.94	1.00			0.90			0.97	
Frt	1.00	0.99		1.00	0.99			0.98			0.94	
Flt Protected	0.95	1.00		0.95	1.00			0.97			0.98	
Satd. Flow (prot)	1502	1749		1529	3433			1489			1398	
Flt Permitted	0.28	1.00		0.39	1.00			0.76			0.84	
Satd. Flow (perm)	446	1749		627	3433			1173			1197	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.97	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	33	539	22	11	883	33	62	22	17	33	11	39
RTOR Reduction (vph)	0	1	0	0	2	0	0	10	0	0	31	0
Lane Group Flow (vph)	33	560	0	11	914	0	0	91	0	0	52	0
Confl. Peds. (#/hr)	214		81	81		214	192		56	56		192
Confl. Bikes (#/hr)			2			3						
Heavy Vehicles (%)	7%	7%	14%	10%	3%	4%	3%	11%	15%	6%	22%	10%
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	58.6	58.6		58.6	58.6			18.4			18.4	
Effective Green, g (s)	58.6	58.6		58.6	58.6			18.4			18.4	
Actuated g/C Ratio	0.65	0.65		0.65	0.65			0.20			0.20	
Clearance Time (s)	5.9	5.9		5.9	5.9			7.1			7.1	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0			3.0	
Lane Grp Cap (vph)	290	1138		408	2235			239			244	
v/s Ratio Prot		c0.32			0.27							
v/s Ratio Perm	0.07			0.02				c0.08			0.04	
v/c Ratio	0.11	0.49		0.03	0.41			0.38			0.21	
Uniform Delay, d1	5.9	8.1		5.6	7.5			30.9			29.8	
Progression Factor	1.00	1.00		0.40	0.48			1.00			1.00	
Incremental Delay, d2	0.8	1.5		0.1	0.5			1.0			0.4	
Delay (s)	6.7	9.6		2.3	4.1			31.9			30.2	
Level of Service	A	A		A	A			C			C	
Approach Delay (s)		9.4			4.1			31.9			30.2	
Approach LOS		A			A			C			C	
Intersection Summary												
HCM 2000 Control Delay			8.9			HCM 2000 Level of Service				A		
HCM 2000 Volume to Capacity ratio			0.47									
Actuated Cycle Length (s)			90.0			Sum of lost time (s)			13.0			
Intersection Capacity Utilization			61.1%			ICU Level of Service			B			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis

FB AM

10:

Future Background AM

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	5	0	0	45	35	5
Future Volume (Veh/h)	5	0	0	45	35	5
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	0	0	49	38	5
Pedestrians	1			1	17	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	0			0	1	
Right turn flare (veh)						
Median type				None	None	
Median storage veh						
Upstream signal (m)					25	
pX, platoon unblocked						
vC, conflicting volume	108	42	44			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	108	42	44			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	100	100			
cM capacity (veh/h)	881	1032	1576			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	5	49	43			
Volume Left	5	0	0			
Volume Right	0	0	5			
cSH	881	1576	1700			
Volume to Capacity	0.01	0.00	0.03			
Queue Length 95th (m)	0.1	0.0	0.0			
Control Delay (s)	9.1	0.0	0.0			
Lane LOS	A					
Approach Delay (s)	9.1	0.0	0.0			
Approach LOS	A					
Intersection Summary						
Average Delay		0.5				
Intersection Capacity Utilization		14.0%		ICU Level of Service		A
Analysis Period (min)		15				

Timings 1: Main Street & Danforth Avenue

FB AM
Future Background AM

									
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations									
Traffic Volume (vph)	20	410	245	710	80	80	265	45	225
Future Volume (vph)	20	410	245	710	80	80	265	45	225
Turn Type	Perm	NA	pm+pt	NA	Perm	Perm	NA	Perm	NA
Protected Phases		4	3	8			2		6
Permitted Phases	4		8		8	2		6	
Detector Phase	4	4	3	8	8	2	2	6	6
Switch Phase									
Minimum Initial (s)	30.0	30.0	6.0	30.0	30.0	20.0	20.0	20.0	20.0
Minimum Split (s)	36.7	36.7	13.4	36.7	36.7	31.7	31.7	31.7	31.7
Total Split (s)	38.0	38.0	18.0	56.0	56.0	34.0	34.0	34.0	34.0
Total Split (%)	42.2%	42.2%	20.0%	62.2%	62.2%	37.8%	37.8%	37.8%	37.8%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.7	3.7	4.4	3.7	3.7	3.7	3.7	3.7	3.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	6.7	6.7	7.4	6.7	6.7		6.7		6.7
Lead/Lag	Lag	Lag	Lead						
Lead-Lag Optimize?	Yes	Yes	Yes						
Recall Mode	Min	Min	Min	None	None	C-Min	C-Min	C-Min	C-Min
Act Effect Green (s)	31.2	31.2	49.2	49.9	49.9		26.7		26.7
Actuated g/C Ratio	0.35	0.35	0.55	0.55	0.55		0.30		0.30
v/c Ratio	0.10	0.87	0.75	0.72	0.14		0.72		0.43
Control Delay	19.1	43.7	28.6	20.0	2.9		29.9		27.4
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	19.1	43.7	28.6	20.0	2.9		29.9		27.4
LOS	B	D	C	B	A		C		C
Approach Delay		42.8		20.7			29.9		27.4
Approach LOS		D		C			C		C

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 89 (99%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.87

Intersection Signal Delay: 28.4

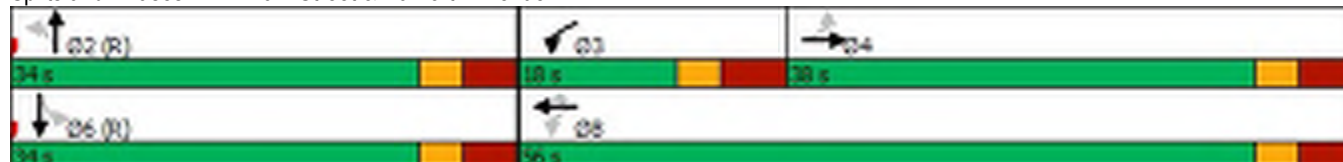
Intersection LOS: C

Intersection Capacity Utilization 126.4%

ICU Level of Service H

Analysis Period (min) 15

Splits and Phases: 1: Main Street & Danforth Avenue

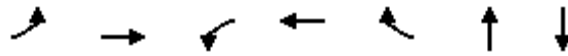


Queues

FB AM

1: Main Street & Danforth Avenue

Future Background AM



Lane Group	EBL	EBT	WBL	WBT	WBR	NBT	SBT
Lane Group Flow (vph)	21	521	253	732	82	525	299
v/c Ratio	0.10	0.87	0.75	0.72	0.14	0.72	0.43
Control Delay	19.1	43.7	28.6	20.0	2.9	29.9	27.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	19.1	43.7	28.6	20.0	2.9	29.9	27.4
Queue Length 50th (m)	2.8	75.1	20.8	83.7	0.0	39.3	23.5
Queue Length 95th (m)	m5.3	#150.7	#57.9	140.6	6.3	56.0	34.3
Internal Link Dist (m)		68.6		134.2		339.1	295.4
Turn Bay Length (m)	30.0						
Base Capacity (vph)	204	605	339	1021	611	753	715
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.10	0.86	0.75	0.72	0.13	0.70	0.42

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

FB AM

1: Main Street & Danforth Avenue

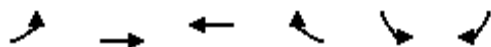
Future Background AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	20	410	95	245	710	80	80	265	165	45	225	20
Future Volume (vph)	20	410	95	245	710	80	80	265	165	45	225	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	6.7	6.7		7.4	6.7	6.7		6.7			6.7	
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00		0.95			0.95	
Frpb, ped/bikes	1.00	0.97		1.00	1.00	0.82		0.91			0.95	
Flpb, ped/bikes	0.94	1.00		0.99	1.00	1.00		0.93			0.98	
Frt	1.00	0.97		1.00	1.00	0.85		0.95			0.99	
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.99			0.99	
Satd. Flow (prot)	1528	1696		1690	1827	1028		2695			2930	
Flt Permitted	0.36	1.00		0.17	1.00	1.00		0.83			0.78	
Satd. Flow (perm)	581	1696		303	1827	1028		2241			2302	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	21	423	98	253	732	82	82	273	170	46	232	21
RTOR Reduction (vph)	0	9	0	0	0	37	0	65	0	0	6	0
Lane Group Flow (vph)	21	512	0	253	732	45	0	460	0	0	293	0
Confl. Peds. (#/hr)	185		173	173		185	768		139	139		768
Confl. Bikes (#/hr)			6			2						6
Heavy Vehicles (%)	10%	6%	3%	5%	4%	28%	3%	8%	5%	40%	7%	4%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	6	6	0	6	6
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4			8		8	2			6		
Actuated Green, G (s)	31.2	31.2		49.9	49.9	49.9		26.7			26.7	
Effective Green, g (s)	31.2	31.2		49.9	49.9	49.9		26.7			26.7	
Actuated g/C Ratio	0.35	0.35		0.55	0.55	0.55		0.30			0.30	
Clearance Time (s)	6.7	6.7		7.4	6.7	6.7		6.7			6.7	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	201	587		342	1012	569		664			682	
v/s Ratio Prot		c0.30		0.09	c0.40							
v/s Ratio Perm	0.04			0.32		0.04		c0.21			0.13	
v/c Ratio	0.10	0.87		0.74	0.72	0.08		0.69			0.43	
Uniform Delay, d1	19.9	27.5		15.3	14.9	9.3		28.0			25.5	
Progression Factor	0.89	1.00		1.00	1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.2	12.8		8.1	2.6	0.1		5.9			2.0	
Delay (s)	17.9	40.3		23.5	17.5	9.4		33.9			27.5	
Level of Service	B	D		C	B	A		C			C	
Approach Delay (s)		39.5			18.3			33.9			27.5	
Approach LOS		D			B			C			C	
Intersection Summary												
HCM 2000 Control Delay			27.5									
HCM 2000 Volume to Capacity ratio			0.83									
Actuated Cycle Length (s)			90.0									
Intersection Capacity Utilization			126.4%									
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis

2: Danforth Avenue & Chisholm Avenue

FB AM
Future Background AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	15	515	770	40	10	30
Future Volume (Veh/h)	15	515	770	40	10	30
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	16	542	811	42	11	32
Pedestrians		6	2		125	
Lane Width (m)		3.6	3.5		3.6	
Walking Speed (m/s)		1.2	1.2		1.2	
Percent Blockage		1	0		10	
Right turn flare (veh)						
Median type		None	None			
Median storage veh						
Upstream signal (m)		172	93			
pX, platoon unblocked	0.69				0.75	0.69
vC, conflicting volume	978				1512	942
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	746				1153	694
tC, single (s)	4.1				6.5	6.3
tC, 2 stage (s)						
tF (s)	2.2				3.6	3.4
p0 queue free %	97				92	88
cM capacity (veh/h)	540				138	269
Direction, Lane #	EB 1	WB 1	WB 2	SB 1		
Volume Total	558	811	42	43		
Volume Left	16	0	0	11		
Volume Right	0	0	42	32		
cSH	540	1700	1700	217		
Volume to Capacity	0.03	0.48	0.02	0.20		
Queue Length 95th (m)	0.7	0.0	0.0	5.8		
Control Delay (s)	0.8	0.0	0.0	25.7		
Lane LOS	A			D		
Approach Delay (s)	0.8	0.0		25.7		
Approach LOS				D		
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utilization			52.3%	ICU Level of Service		A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

3: Danforth East Site Access & Danforth Avenue

FB AM
Future Background AM

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↰			↱	↰	↱
Traffic Volume (veh/h)	510	15	20	780	10	20
Future Volume (Veh/h)	510	15	20	780	10	20
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	548	16	22	839	11	22
Pedestrians	7			9	149	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	1			1	12	
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (m)	136			128		
pX, platoon unblocked			0.87		0.76	0.87
vC, conflicting volume			713		1595	714
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			594		1233	595
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			97		91	94
cM capacity (veh/h)			755		126	383
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	564	861	33			
Volume Left	0	22	11			
Volume Right	16	0	22			
cSH	1700	755	228			
Volume to Capacity	0.33	0.03	0.14			
Queue Length 95th (m)	0.0	0.7	4.0			
Control Delay (s)	0.0	0.8	23.5			
Lane LOS		A	C			
Approach Delay (s)	0.0	0.8	23.5			
Approach LOS			C			
Intersection Summary						
Average Delay		1.0				
Intersection Capacity Utilization		69.7%	ICU Level of Service		C	
Analysis Period (min)		15				

HCM Unsignalized Intersection Capacity Analysis

4: Danforth West Site Access & Danforth Avenue

FB AM
Future Background AM

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (veh/h)	520	5	0	790	0	5
Future Volume (Veh/h)	520	5	0	790	0	5
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	559	5	0	849	0	5
Pedestrians	2					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (m)	49			215		
pX, platoon unblocked			0.87			0.87
vC, conflicting volume			564			1412
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			419			986
tC, single (s)			4.1			6.4
tC, 2 stage (s)						
tF (s)			2.2			3.5
p0 queue free %			100			100
cM capacity (veh/h)			997			209
						555
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	564	849	5			
Volume Left	0	0	0			
Volume Right	5	0	5			
cSH	1700	997	555			
Volume to Capacity	0.33	0.00	0.01			
Queue Length 95th (m)	0.0	0.0	0.2			
Control Delay (s)	0.0	0.0	11.5			
Lane LOS				B		
Approach Delay (s)	0.0	0.0	11.5			
Approach LOS				B		
Intersection Summary						
Average Delay				0.0		
Intersection Capacity Utilization				51.6%	ICU Level of Service	A
Analysis Period (min)				15		

HCM Unsignalized Intersection Capacity Analysis

5: Westlake Avenue & Westlake Site Access

FB AM
Future Background AM

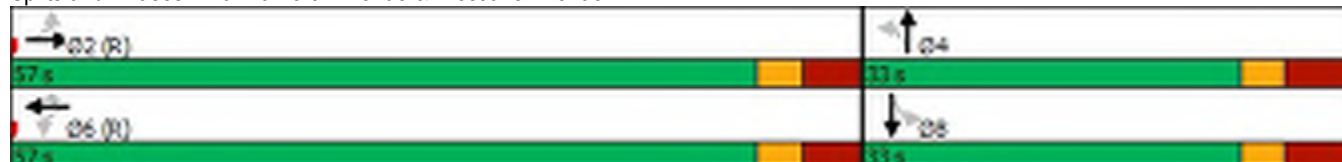
									
Movement	WBL	WBR	NBT	NBR	SBL	SBT			
Lane Configurations									
Traffic Volume (veh/h)	0	5	40	0	5	30			
Future Volume (Veh/h)	0	5	40	0	5	30			
Sign Control	Stop		Free			Free			
Grade	0%		0%			0%			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92			
Hourly flow rate (vph)	0	5	43	0	5	33			
Pedestrians	20		7			2			
Lane Width (m)	3.6		3.6			3.6			
Walking Speed (m/s)	1.2		1.2			1.2			
Percent Blockage	2		1			0			
Right turn flare (veh)									
Median type			None			None			
Median storage veh									
Upstream signal (m)						43			
pX, platoon unblocked									
vC, conflicting volume	113	65			63				
vC1, stage 1 conf vol									
vC2, stage 2 conf vol									
vCu, unblocked vol	113	65			63				
tC, single (s)	6.4	6.2			4.3				
tC, 2 stage (s)									
tF (s)	3.5	3.3			2.4				
p0 queue free %	100	99			100				
cM capacity (veh/h)	865	986			1409				
Direction, Lane #	WB 1	NB 1	SB 1						
Volume Total	5	43	38						
Volume Left	0	0	5						
Volume Right	5	0	0						
cSH	986	1700	1409						
Volume to Capacity	0.01	0.03	0.00						
Queue Length 95th (m)	0.1	0.0	0.1						
Control Delay (s)	8.7	0.0	1.0						
Lane LOS	A		A						
Approach Delay (s)	8.7	0.0	1.0						
Approach LOS	A								
Intersection Summary									
Average Delay		1.0							
Intersection Capacity Utilization		18.8%	ICU Level of Service	A					
Analysis Period (min)		15							

Timings 6: Danforth Avenue & Westlake Avenue

FB AM
Future Background AM

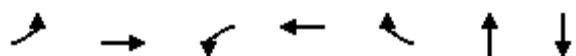
									
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations									
Traffic Volume (vph)	20	495	15	755	20	30	10	20	5
Future Volume (vph)	20	495	15	755	20	30	10	20	5
Turn Type	Perm	NA	Perm	NA	Perm	Perm	NA	Perm	NA
Protected Phases		2		6			4		8
Permitted Phases	2		6		6	4		8	
Detector Phase	2	2	6	6	6	4	4	8	8
Switch Phase									
Minimum Initial (s)	14.0	14.0	14.0	14.0	14.0	20.0	20.0	20.0	20.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	32.7	32.7	32.7	32.7
Total Split (s)	57.0	57.0	57.0	57.0	57.0	33.0	33.0	33.0	33.0
Total Split (%)	63.3%	63.3%	63.3%	63.3%	63.3%	36.7%	36.7%	36.7%	36.7%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	4.0	4.0	4.0	4.0	4.0	4.7	4.7	4.7	4.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0		7.7		7.7
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	C-Min	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	69.2	69.2	69.2	69.2	69.2		20.0		20.0
Actuated g/C Ratio	0.77	0.77	0.77	0.77	0.77		0.22		0.22
v/c Ratio	0.06	0.40	0.03	0.57	0.02		0.18		0.17
Control Delay	12.6	13.0	9.9	11.5	9.5		25.6		18.3
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	12.6	13.0	9.9	11.5	9.5		25.6		18.3
LOS	B	B	A	B	A		C		B
Approach Delay		12.9		11.4			25.6		18.3
Approach LOS		B		B			C		B
Intersection Summary									
Cycle Length: 90									
Actuated Cycle Length: 90									
Offset: 28 (31%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green									
Natural Cycle: 75									
Control Type: Actuated-Coordinated									
Maximum v/c Ratio: 0.57									
Intersection Signal Delay: 12.7					Intersection LOS: B				
Intersection Capacity Utilization 71.6%					ICU Level of Service C				
Analysis Period (min) 15									

Splits and Phases: 6: Danforth Avenue & Westlake Avenue



Queues
6: Danforth Avenue & Westlake Avenue

FB AM
Future Background AM



Lane Group	EBL	EBT	WBL	WBT	WBR	NBT	SBT
Lane Group Flow (vph)	21	548	16	803	21	54	53
v/c Ratio	0.06	0.40	0.03	0.57	0.02	0.18	0.17
Control Delay	12.6	13.0	9.9	11.5	9.5	25.6	18.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	12.6	13.0	9.9	11.5	9.5	25.6	18.3
Queue Length 50th (m)	2.1	68.4	1.3	87.4	1.7	6.3	3.8
Queue Length 95th (m)	m5.2	97.1	m2.0	100.5	m2.6	16.4	13.6
Internal Link Dist (m)		139.2		25.3		0.7	152.8
Turn Bay Length (m)	35.0		20.0		20.0		
Base Capacity (vph)	363	1358	496	1404	893	377	398
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.06	0.40	0.03	0.57	0.02	0.14	0.13

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

6: Danforth Avenue & Westlake Avenue

FB AM
Future Background AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	20	495	20	15	755	20	30	10	10	20	5	25
Future Volume (vph)	20	495	20	15	755	20	30	10	10	20	5	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	7.0	7.0		7.0	7.0	7.0		7.7			7.7	
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	0.99		1.00	1.00	0.77		0.98			0.95	
Flpb, ped/bikes	0.96	1.00		0.92	1.00	1.00		0.96			0.98	
Frt	1.00	0.99		1.00	1.00	0.85		0.97			0.93	
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.97			0.98	
Satd. Flow (prot)	1539	1766		1421	1827	1162		1623			1562	
Flt Permitted	0.28	1.00		0.43	1.00	1.00		0.79			0.85	
Satd. Flow (perm)	452	1766		636	1827	1162		1316			1349	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	21	527	21	16	803	21	32	11	11	21	5	27
RTOR Reduction (vph)	0	1	0	0	0	0	0	10	0	0	23	0
Lane Group Flow (vph)	21	547	0	16	803	21	0	44	0	0	30	0
Confl. Peds. (#/hr)	90		87	87		90	38		28	28		38
Confl. Bikes (#/hr)									3			1
Heavy Vehicles (%)	11%	6%	8%	15%	4%	6%	8%	0%	0%	5%	0%	4%
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6		6	4			8		
Actuated Green, G (s)	63.3	63.3		63.3	63.3	63.3		12.0			12.0	
Effective Green, g (s)	63.3	63.3		63.3	63.3	63.3		12.0			12.0	
Actuated g/C Ratio	0.70	0.70		0.70	0.70	0.70		0.13			0.13	
Clearance Time (s)	7.0	7.0		7.0	7.0	7.0		7.7			7.7	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	317	1242		447	1284	817		175			179	
v/s Ratio Prot		0.31			c0.44							
v/s Ratio Perm	0.05			0.03		0.02		c0.03			0.02	
v/c Ratio	0.07	0.44		0.04	0.63	0.03		0.25			0.17	
Uniform Delay, d1	4.2	5.7		4.1	7.1	4.0		35.0			34.6	
Progression Factor	1.75	1.65		1.42	1.10	1.43		1.00			1.00	
Incremental Delay, d2	0.4	1.0		0.1	1.8	0.0		0.8			0.4	
Delay (s)	7.6	10.5		5.9	9.5	5.8		35.8			35.0	
Level of Service	A	B		A	A	A		D			C	
Approach Delay (s)		10.4			9.4			35.8			35.0	
Approach LOS		B			A			D			C	
Intersection Summary												
HCM 2000 Control Delay			11.6				HCM 2000 Level of Service			B		
HCM 2000 Volume to Capacity ratio			0.57									
Actuated Cycle Length (s)			90.0				Sum of lost time (s)			14.7		
Intersection Capacity Utilization			71.6%				ICU Level of Service			C		
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis 7: Westlake Avenue & Stephenson Avenue

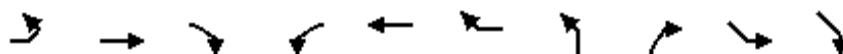
FB AM
Future Background AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control	Stop			Stop			Stop			Stop		
Traffic Volume (vph)	0	15	0	5	15	25	0	15	5	15	10	5
Future Volume (vph)	0	15	0	5	15	25	0	15	5	15	10	5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	16	0	5	16	27	0	16	5	16	11	5
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	16	48	21	32								
Volume Left (vph)	0	5	0	16								
Volume Right (vph)	0	27	5	5								
Hadj (s)	0.00	-0.22	-0.01	0.14								
Departure Headway (s)	4.1	3.8	4.1	4.2								
Degree Utilization, x	0.02	0.05	0.02	0.04								
Capacity (veh/h)	867	925	862	840								
Control Delay (s)	7.1	7.0	7.1	7.4								
Approach Delay (s)	7.1	7.0	7.1	7.4								
Approach LOS	A	A	A	A								
Intersection Summary												
Delay				7.2								
Level of Service				A								
Intersection Capacity Utilization				30.5%	ICU Level of Service				A			
Analysis Period (min)				15								

HCM Unsignalized Intersection Capacity Analysis

8: Morton Road & Danforth Avenue & Oak Park Avenue

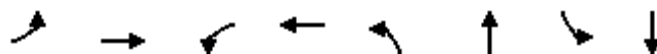
FB AM
Future Background AM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBR	SEL	SER
Lane Configurations		↔			↔		↔		↔	
Traffic Volume (veh/h)	0	520	10	15	780	15	5	15	0	0
Future Volume (Veh/h)	0	520	10	15	780	15	5	15	0	0
Sign Control		Free			Free		Stop		Stop	
Grade		0%			0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	565	11	16	848	16	5	16	0	0
Pedestrians							90		150	
Lane Width (m)							3.6		3.6	
Walking Speed (m/s)							1.2		1.2	
Percent Blockage							8		12	
Right turn flare (veh)										
Median type		None			None					
Median storage (veh)										
Upstream signal (m)		143			163					
pX, platoon unblocked	0.73			0.84			0.81	0.84	0.81	0.81
vC, conflicting volume	1014			666			1706	660	1627	1704
vC1, stage 1 conf vol										
vC2, stage 2 conf vol										
vCu, unblocked vol	835			506			1308	499	1210	1305
tC, single (s)	4.1			4.2			6.5	6.3	7.1	6.5
tC, 2 stage (s)										
tF (s)	2.2			2.3			4.0	3.4	3.5	4.0
p0 queue free %	100			98			95	96	100	100
cM capacity (veh/h)	516			802			104	436	88	103
Direction, Lane #	EB 1	WB 1	NB 1	SE 1						
Volume Total	576	880	21	0						
Volume Left	0	16	0	0						
Volume Right	11	16	16	0						
cSH	516	802	247	1700						
Volume to Capacity	0.00	0.02	0.08	0.00						
Queue Length 95th (m)	0.0	0.5	2.2	0.0						
Control Delay (s)	0.0	0.6	20.9	0.0						
Lane LOS		A	C	A						
Approach Delay (s)	0.0	0.6	20.9	0.0						
Approach LOS			C	A						
Intersection Summary										
Average Delay			0.6							
Intersection Capacity Utilization			64.1%		ICU Level of Service				C	
Analysis Period (min)			15							

Timings 9: Patricia Drive/Gledhill Avenue & Danforth Avenue

FB AM
Future Background AM



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations								
Traffic Volume (vph)	30	485	10	795	60	20	30	10
Future Volume (vph)	30	485	10	795	60	20	30	10
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases		2		6		4		8
Permitted Phases	2		6		4		8	
Detector Phase	2	2	6	6	4	4	8	8
Switch Phase								
Minimum Initial (s)	19.0	19.0	19.0	19.0	23.0	23.0	23.0	23.0
Minimum Split (s)	35.1	35.1	35.1	35.1	35.1	35.1	35.1	35.1
Total Split (s)	54.0	54.0	54.0	54.0	36.0	36.0	36.0	36.0
Total Split (%)	60.0%	60.0%	60.0%	60.0%	40.0%	40.0%	40.0%	40.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.9	2.9	2.9	2.9	4.1	4.1	4.1	4.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	5.9	5.9	5.9	5.9		7.1		7.1
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effect Green (s)	61.2	61.2	61.2	61.2		23.0		23.0
Actuated g/C Ratio	0.68	0.68	0.68	0.68		0.26		0.26
v/c Ratio	0.11	0.47	0.03	0.39		0.33		0.25
Control Delay	9.0	11.1	3.1	4.6		27.4		18.0
Queue Delay	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	9.0	11.1	3.1	4.6		27.4		18.0
LOS	A	B	A	A		C		B
Approach Delay		10.9		4.6		27.4		18.0
Approach LOS		B		A		C		B

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 63 (70%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 75

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.47

Intersection Signal Delay: 8.8

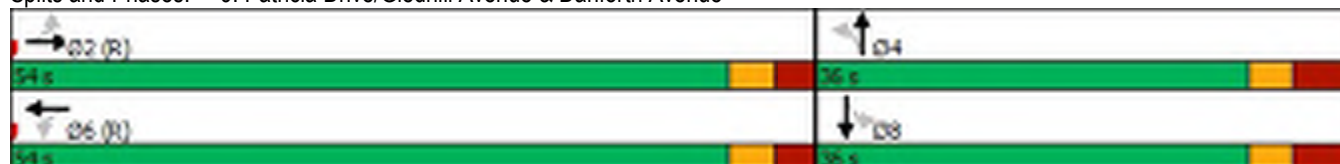
Intersection LOS: A

Intersection Capacity Utilization 61.1%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 9: Patricia Drive/Gledhill Avenue & Danforth Avenue





Lane Group	EBL	EBT	WBL	WBT	NBT	SBT
Lane Group Flow (vph)	33	561	11	916	101	83
v/c Ratio	0.11	0.47	0.03	0.39	0.33	0.25
Control Delay	9.0	11.1	3.1	4.6	27.4	18.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	9.0	11.1	3.1	4.6	27.4	18.0
Queue Length 50th (m)	2.4	54.6	0.1	8.0	13.1	6.2
Queue Length 95th (m)	6.9	82.4	m0.5	13.7	27.7	18.4
Internal Link Dist (m)		219.0		119.1	113.2	112.4
Turn Bay Length (m)	30.0		20.0			
Base Capacity (vph)	306	1191	429	2336	384	410
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.47	0.03	0.39	0.26	0.20

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

9: Patricia Drive/Gledhill Avenue & Danforth Avenue

FB AM
Future Background AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	30	485	20	10	795	30	60	20	15	30	10	35
Future Volume (vph)	30	485	20	10	795	30	60	20	15	30	10	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	5.9	5.9		5.9	5.9			7.1			7.1	
Lane Util. Factor	1.00	1.00		1.00	0.95			1.00			1.00	
Frpb, ped/bikes	1.00	0.99		1.00	0.99			0.98			0.91	
Flpb, ped/bikes	0.90	1.00		0.94	1.00			0.90			0.97	
Frt	1.00	0.99		1.00	0.99			0.98			0.94	
Flt Protected	0.95	1.00		0.95	1.00			0.97			0.98	
Satd. Flow (prot)	1502	1749		1529	3433			1489			1398	
Flt Permitted	0.28	1.00		0.39	1.00			0.76			0.84	
Satd. Flow (perm)	446	1749		627	3433			1173			1197	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.97	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	33	539	22	11	883	33	62	22	17	33	11	39
RTOR Reduction (vph)	0	1	0	0	2	0	0	10	0	0	31	0
Lane Group Flow (vph)	33	560	0	11	914	0	0	91	0	0	52	0
Confl. Peds. (#/hr)	214		81	81		214	192		56	56		192
Confl. Bikes (#/hr)			2			3						
Heavy Vehicles (%)	7%	7%	14%	10%	3%	4%	3%	11%	15%	6%	22%	10%
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	58.6	58.6		58.6	58.6			18.4			18.4	
Effective Green, g (s)	58.6	58.6		58.6	58.6			18.4			18.4	
Actuated g/C Ratio	0.65	0.65		0.65	0.65			0.20			0.20	
Clearance Time (s)	5.9	5.9		5.9	5.9			7.1			7.1	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0			3.0	
Lane Grp Cap (vph)	290	1138		408	2235			239			244	
v/s Ratio Prot		c0.32			0.27							
v/s Ratio Perm	0.07			0.02				c0.08			0.04	
v/c Ratio	0.11	0.49		0.03	0.41			0.38			0.21	
Uniform Delay, d1	5.9	8.1		5.6	7.5			30.9			29.8	
Progression Factor	1.00	1.00		0.40	0.48			1.00			1.00	
Incremental Delay, d2	0.8	1.5		0.1	0.5			1.0			0.4	
Delay (s)	6.7	9.6		2.3	4.1			31.9			30.2	
Level of Service	A	A		A	A			C			C	
Approach Delay (s)		9.4			4.1			31.9			30.2	
Approach LOS		A			A			C			C	
Intersection Summary												
HCM 2000 Control Delay			8.9			HCM 2000 Level of Service				A		
HCM 2000 Volume to Capacity ratio			0.47									
Actuated Cycle Length (s)			90.0			Sum of lost time (s)			13.0			
Intersection Capacity Utilization			61.1%			ICU Level of Service			B			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis

FB AM

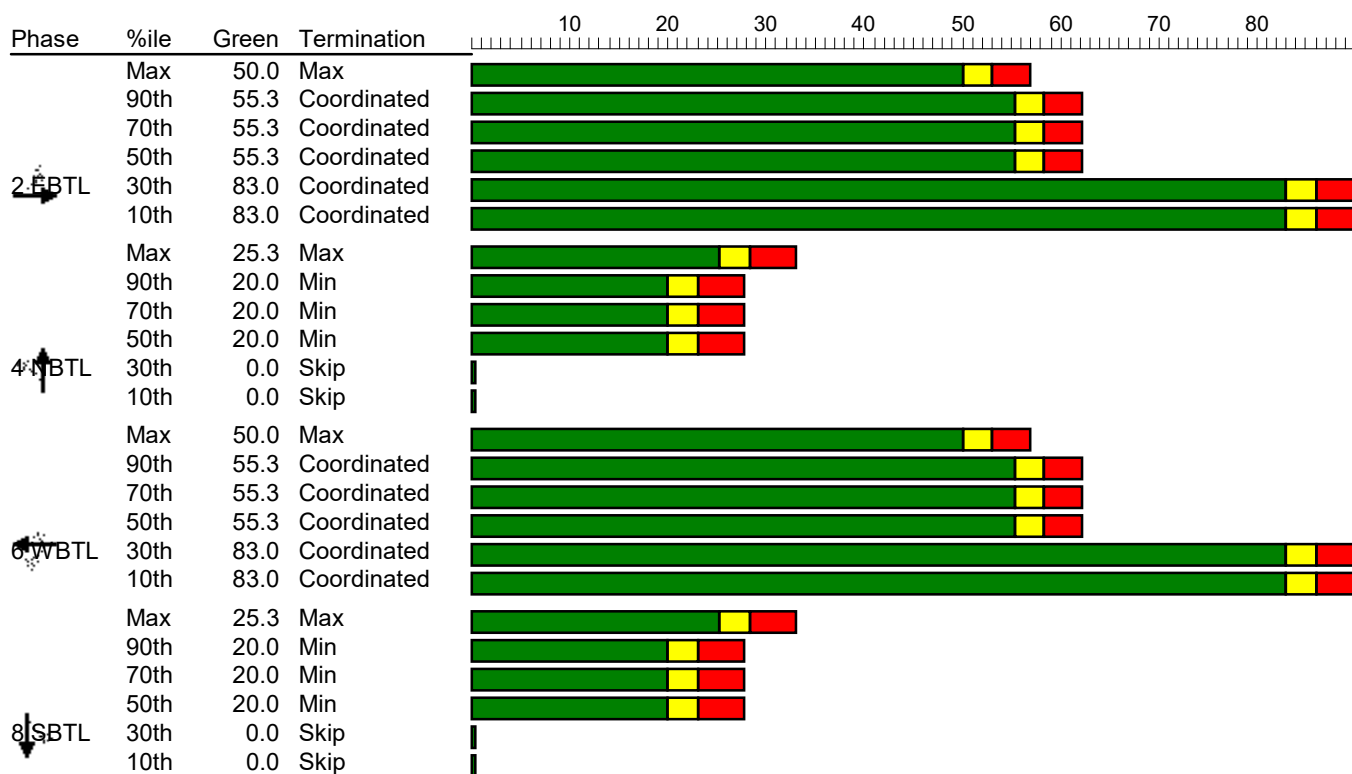
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Future Background AM

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	5	0	0	45	35	5
Future Volume (Veh/h)	5	0	0	45	35	5
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	0	0	49	38	5
Pedestrians	1			1	17	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	0			0	1	
Right turn flare (veh)						
Median type				None	None	
Median storage veh						
Upstream signal (m)					25	
pX, platoon unblocked						
vC, conflicting volume	108	42	44			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	108	42	44			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	100	100			
cM capacity (veh/h)	881	1032	1576			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	5	49	43			
Volume Left	5	0	0			
Volume Right	0	0	5			
cSH	881	1576	1700			
Volume to Capacity	0.01	0.00	0.03			
Queue Length 95th (m)	0.1	0.0	0.0			
Control Delay (s)	9.1	0.0	0.0			
Lane LOS	A					
Approach Delay (s)	9.1	0.0	0.0			
Approach LOS	A					
Intersection Summary						
Average Delay		0.5				
Intersection Capacity Utilization		14.0%		ICU Level of Service		A
Analysis Period (min)		15				

Actuated Signal, Actual Green Times
6: Danforth Avenue & Westlake Avenue

FB PM
Future Background PM



Cycle Length: 90

Timings

1: Main Street & Danforth Avenue

FT AM
Future Total AM

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	25	415	95	245	710	80	80	265	165	45	225	20
Future Volume (vph)	25	415	95	245	710	80	80	265	165	45	225	20
Satd. Flow (prot)	1623	1697	0	1700	1827	1248	0	2895	0	0	2977	0
Flt Permitted	0.363			0.165				0.825			0.779	
Satd. Flow (perm)	585	1697	0	282	1827	1028	0	2240	0	0	2299	0
Satd. Flow (RTOR)		14				82		92			9	
Lane Group Flow (vph)	26	526	0	253	732	82	0	525	0	0	299	0
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4			8		8	2			6		
Total Split (s)	38.0	38.0		18.0	56.0	56.0	34.0	34.0		34.0	34.0	
Total Lost Time (s)	6.7	6.7		7.4	6.7	6.7		6.7			6.7	
Act Effect Green (s)	31.2	31.2		49.3	50.0	50.0		26.6			26.6	
Actuated g/C Ratio	0.35	0.35		0.55	0.56	0.56		0.30			0.30	
v/c Ratio	0.13	0.88		0.76	0.72	0.14		0.72			0.44	
Control Delay	18.2	41.0		29.6	19.9	2.9		30.0			27.4	
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	
Total Delay	18.2	41.0		29.6	19.9	2.9		30.0			27.4	
LOS	B	D		C	B	A		C			C	
Approach Delay		40.0			20.9			30.0			27.4	
Approach LOS		D			C			C			C	

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 89 (99%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.88

Intersection Signal Delay: 28.0

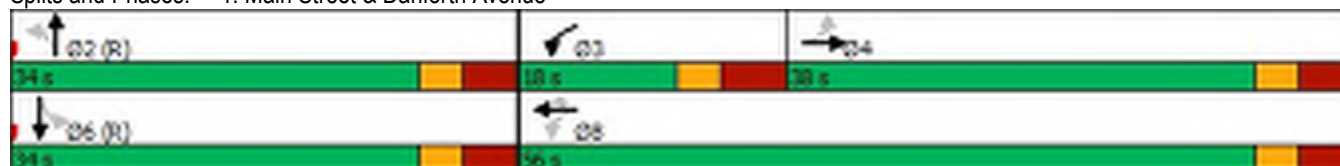
Intersection LOS: C

Intersection Capacity Utilization 126.4%

ICU Level of Service H

Analysis Period (min) 15

Splits and Phases: 1: Main Street & Danforth Avenue

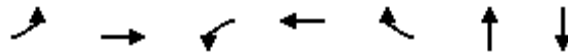


Queues

FT AM

1: Main Street & Danforth Avenue

Future Total AM



Lane Group	EBL	EBT	WBL	WBT	WBR	NBT	SBT
Lane Group Flow (vph)	26	526	253	732	82	525	299
v/c Ratio	0.13	0.88	0.76	0.72	0.14	0.72	0.44
Control Delay	18.2	41.0	29.6	19.9	2.9	30.0	27.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	18.2	41.0	29.6	19.9	2.9	30.0	27.4
Queue Length 50th (m)	3.4	80.5	20.8	83.4	0.0	39.3	23.5
Queue Length 95th (m)	m4.9	#153.3	#59.4	140.6	6.3	56.0	34.3
Internal Link Dist (m)		68.6		134.2		339.1	295.4
Turn Bay Length (m)	30.0						
Base Capacity (vph)	205	606	336	1022	611	753	714
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.13	0.87	0.75	0.72	0.13	0.70	0.42

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

1: Main Street & Danforth Avenue

FT AM

Future Total AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	25	415	95	245	710	80	80	265	165	45	225	20
Future Volume (vph)	25	415	95	245	710	80	80	265	165	45	225	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	6.7	6.7		7.4	6.7	6.7		6.7			6.7	
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00		0.95			0.95	
Frpb, ped/bikes	1.00	0.97		1.00	1.00	0.82		0.91			0.95	
Flpb, ped/bikes	0.94	1.00		0.99	1.00	1.00		0.93			0.98	
Frt	1.00	0.97		1.00	1.00	0.85		0.95			0.99	
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.99			0.99	
Satd. Flow (prot)	1528	1697		1691	1827	1028		2695			2930	
Flt Permitted	0.36	1.00		0.17	1.00	1.00		0.83			0.78	
Satd. Flow (perm)	585	1697		294	1827	1028		2242			2299	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	26	428	98	253	732	82	82	273	170	46	232	21
RTOR Reduction (vph)	0	9	0	0	0	36	0	65	0	0	6	0
Lane Group Flow (vph)	26	517	0	253	732	46	0	460	0	0	293	0
Confl. Peds. (#/hr)	185		173	173		185	768		139	139		768
Confl. Bikes (#/hr)			6			2						6
Heavy Vehicles (%)	10%	6%	3%	5%	4%	28%	3%	8%	5%	40%	7%	4%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	6	6	0	6	6
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4			8		8	2			6		
Actuated Green, G (s)	31.2	31.2		50.0	50.0	50.0		26.6			26.6	
Effective Green, g (s)	31.2	31.2		50.0	50.0	50.0		26.6			26.6	
Actuated g/C Ratio	0.35	0.35		0.56	0.56	0.56		0.30			0.30	
Clearance Time (s)	6.7	6.7		7.4	6.7	6.7		6.7			6.7	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	202	588		340	1015	571		662			679	
v/s Ratio Prot		c0.30		0.09	c0.40							
v/s Ratio Perm	0.04			0.32		0.04		c0.21			0.13	
v/c Ratio	0.13	0.88		0.74	0.72	0.08		0.70			0.43	
Uniform Delay, d1	20.1	27.6		15.4	14.8	9.3		28.1			25.6	
Progression Factor	0.83	0.90		1.00	1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.3	12.6		8.5	2.6	0.1		5.9			2.0	
Delay (s)	16.9	37.4		24.0	17.4	9.4		34.0			27.6	
Level of Service	B	D		C	B	A		C			C	
Approach Delay (s)		36.5			18.3			34.0			27.6	
Approach LOS		D			B			C			C	
Intersection Summary												
HCM 2000 Control Delay			26.9									
HCM 2000 Volume to Capacity ratio			0.83									
Actuated Cycle Length (s)			90.0									
Intersection Capacity Utilization			126.4%									
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis

2: Danforth Avenue & Chisholm Avenue

FT AM
Future Total AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	15	525	770	40	10	30
Future Volume (Veh/h)	15	525	770	40	10	30
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	16	553	811	42	11	32
Pedestrians		6	2		125	
Lane Width (m)		3.6	3.5		3.6	
Walking Speed (m/s)		1.2	1.2		1.2	
Percent Blockage		1	0		10	
Right turn flare (veh)						
Median type		None	None			
Median storage veh						
Upstream signal (m)		172	93			
pX, platoon unblocked	0.69				0.78	0.69
vC, conflicting volume	978				1523	942
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	747				1032	695
tC, single (s)	4.1				6.5	6.3
tC, 2 stage (s)						
tF (s)	2.2				3.6	3.4
p0 queue free %	97				94	88
cM capacity (veh/h)	541				169	269
Direction, Lane #	EB 1	WB 1	WB 2	SB 1		
Volume Total	569	811	42	43		
Volume Left	16	0	0	11		
Volume Right	0	0	42	32		
cSH	541	1700	1700	234		
Volume to Capacity	0.03	0.48	0.02	0.18		
Queue Length 95th (m)	0.7	0.0	0.0	5.3		
Control Delay (s)	0.8	0.0	0.0	23.8		
Lane LOS	A			C		
Approach Delay (s)	0.8	0.0		23.8		
Approach LOS				C		
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utilization			52.3%	ICU Level of Service		A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 5: Westlake Avenue & Westlake Site Access

FT AM
Future Total AM

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	10	90	40	0	60	30
Future Volume (Veh/h)	10	90	40	0	60	30
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	98	43	0	65	33
Pedestrians	20		7			2
Lane Width (m)	3.6		3.6			3.6
Walking Speed (m/s)	1.2		1.2			1.2
Percent Blockage	2		1			0
Right turn flare (veh)						
Median type			None			None
Median storage veh						
Upstream signal (m)						43
pX, platoon unblocked						
vC, conflicting volume	233	65			63	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	233	65			63	
tC, single (s)	6.4	6.2			4.3	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.4	
p0 queue free %	98	90			95	
cM capacity (veh/h)	708	986			1409	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	109	43	98			
Volume Left	11	0	65			
Volume Right	98	0	0			
cSH	949	1700	1409			
Volume to Capacity	0.11	0.03	0.05			
Queue Length 95th (m)	3.1	0.0	1.2			
Control Delay (s)	9.3	0.0	5.2			
Lane LOS	A		A			
Approach Delay (s)	9.3	0.0	5.2			
Approach LOS	A					
Intersection Summary						
Average Delay		6.1				
Intersection Capacity Utilization		25.0%		ICU Level of Service		A
Analysis Period (min)		15				

Timings 6: Danforth Avenue & Westlake Avenue

FT AM
Future Total AM

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	20	475	55	35	745	20	80	10	45	20	5	25
Future Volume (vph)	20	475	55	35	745	20	80	10	45	20	5	25
Satd. Flow (prot)	1608	1719	0	1552	1827	1507	0	1641	0	0	1593	0
Flt Permitted	0.231			0.380				0.788			0.842	
Satd. Flow (perm)	391	1719	0	580	1827	1162	0	1282	0	0	1346	0
Satd. Flow (RTOR)		11						28			27	
Lane Group Flow (vph)	21	564	0	37	793	21	0	144	0	0	53	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6		6	4			8		
Total Split (s)	57.0	57.0		57.0	57.0	57.0	33.0	33.0		33.0	33.0	
Total Lost Time (s)	7.0	7.0		7.0	7.0	7.0		7.7			7.7	
Act Effect Green (s)	55.2	55.2		55.2	55.2	55.2		20.1			20.1	
Actuated g/C Ratio	0.61	0.61		0.61	0.61	0.61		0.22			0.22	
v/c Ratio	0.09	0.53		0.10	0.71	0.03		0.47			0.17	
Control Delay	14.8	19.6		11.3	19.1	10.2		30.0			18.2	
Queue Delay	0.0	0.0		0.0	0.0	0.0		0.0			0.0	
Total Delay	14.8	19.6		11.3	19.1	10.2		30.0			18.2	
LOS	B	B		B	B	B		C			B	
Approach Delay		19.5			18.5			30.0			18.2	
Approach LOS		B			B			C			B	

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 28 (31%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.71

Intersection Signal Delay: 19.9

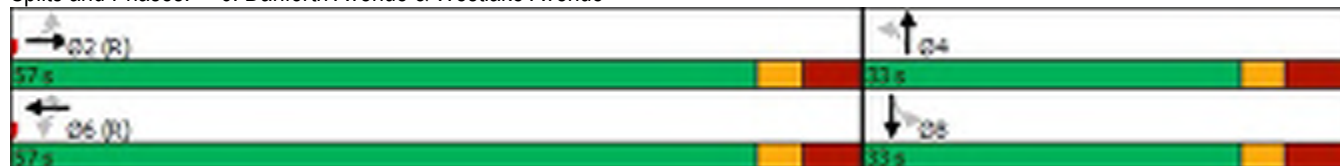
Intersection LOS: B

Intersection Capacity Utilization 71.1%

ICU Level of Service C

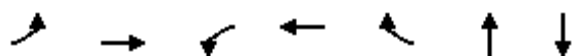
Analysis Period (min) 15

Splits and Phases: 6: Danforth Avenue & Westlake Avenue



Queues
6: Danforth Avenue & Westlake Avenue

FT AM
Future Total AM



Lane Group	EBL	EBT	WBL	WBT	WBR	NBT	SBT
Lane Group Flow (vph)	21	564	37	793	21	144	53
v/c Ratio	0.09	0.53	0.10	0.71	0.03	0.47	0.17
Control Delay	14.8	19.6	11.3	19.1	10.2	30.0	18.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	14.8	19.6	11.3	19.1	10.2	30.0	18.2
Queue Length 50th (m)	2.2	77.3	3.3	88.7	1.7	18.2	3.8
Queue Length 95th (m)	m5.5	107.0	m4.5	99.1	m2.5	36.9	13.5
Internal Link Dist (m)		139.2		25.3		0.7	152.8
Turn Bay Length (m)	35.0		20.0		20.0		
Base Capacity (vph)	240	1059	356	1121	712	380	397
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.53	0.10	0.71	0.03	0.38	0.13

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

6: Danforth Avenue & Westlake Avenue

FT AM
Future Total AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	20	475	55	35	745	20	80	10	45	20	5	25
Future Volume (vph)	20	475	55	35	745	20	80	10	45	20	5	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	7.0	7.0		7.0	7.0	7.0		7.7			7.7	
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	0.98		1.00	1.00	0.77		0.98			0.95	
Flpb, ped/bikes	1.00	1.00		0.93	1.00	1.00		0.96			0.98	
Frt	1.00	0.98		1.00	1.00	0.85		0.95			0.93	
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.97			0.98	
Satd. Flow (prot)	1608	1720		1440	1827	1162		1580			1568	
Flt Permitted	0.23	1.00		0.38	1.00	1.00		0.79			0.84	
Satd. Flow (perm)	390	1720		576	1827	1162		1282			1346	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	21	505	59	37	793	21	85	11	48	21	5	27
RTOR Reduction (vph)	0	4	0	0	0	0	0	22	0	0	21	0
Lane Group Flow (vph)	21	560	0	37	793	21	0	122	0	0	32	0
Confl. Peds. (#/hr)	90		87	87		90	38		28	28		38
Confl. Bikes (#/hr)									3			1
Heavy Vehicles (%)	11%	6%	8%	15%	4%	6%	8%	0%	0%	5%	0%	4%
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6		6	4			8		
Actuated Green, G (s)	55.2	55.2		55.2	55.2	55.2		20.1			20.1	
Effective Green, g (s)	55.2	55.2		55.2	55.2	55.2		20.1			20.1	
Actuated g/C Ratio	0.61	0.61		0.61	0.61	0.61		0.22			0.22	
Clearance Time (s)	7.0	7.0		7.0	7.0	7.0		7.7			7.7	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	239	1054		353	1120	712		286			300	
v/s Ratio Prot		0.33			c0.43							
v/s Ratio Perm	0.05			0.06		0.02		c0.10			0.02	
v/c Ratio	0.09	0.53		0.10	0.71	0.03		0.43			0.11	
Uniform Delay, d1	7.1	10.0		7.2	11.9	6.9		30.0			27.8	
Progression Factor	1.88	1.76		1.44	1.30	1.45		1.00			1.00	
Incremental Delay, d2	0.7	1.7		0.4	2.8	0.1		1.0			0.2	
Delay (s)	14.0	19.3		10.8	18.3	10.0		31.0			28.0	
Level of Service	B	B		B	B	B		C			C	
Approach Delay (s)		19.1			17.7			31.0			28.0	
Approach LOS		B			B			C			C	
Intersection Summary												
HCM 2000 Control Delay			19.7				HCM 2000 Level of Service			B		
HCM 2000 Volume to Capacity ratio			0.63									
Actuated Cycle Length (s)			90.0				Sum of lost time (s)		14.7			
Intersection Capacity Utilization			71.1%				ICU Level of Service		C			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis 7: Westlake Avenue & Stephenson Avenue

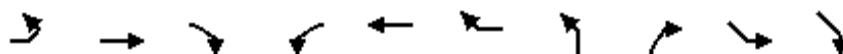
FT AM
Future Total AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control	Stop			Stop			Stop			Stop		
Traffic Volume (vph)	0	15	0	5	15	25	0	15	5	25	10	5
Future Volume (vph)	0	15	0	5	15	25	0	15	5	25	10	5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	16	0	5	16	27	0	16	5	27	11	5
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	16	48	21	43								
Volume Left (vph)	0	5	0	27								
Volume Right (vph)	0	27	5	5								
Hadj (s)	0.00	-0.22	-0.01	0.20								
Departure Headway (s)	4.1	3.8	4.1	4.3								
Degree Utilization, x	0.02	0.05	0.02	0.05								
Capacity (veh/h)	860	917	859	829								
Control Delay (s)	7.2	7.0	7.2	7.5								
Approach Delay (s)	7.2	7.0	7.2	7.5								
Approach LOS	A	A	A	A								
Intersection Summary												
Delay				7.2								
Level of Service				A								
Intersection Capacity Utilization				30.7%	ICU Level of Service	A						
Analysis Period (min)				15								

HCM Unsignalized Intersection Capacity Analysis

8: Morton Road & Danforth Avenue & Oak Park Avenue

FT AM
Future Total AM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBR	SEL	SER
Lane Configurations		↔			↔		↔		↔	
Traffic Volume (veh/h)	0	535	10	15	820	15	5	15	0	0
Future Volume (Veh/h)	0	535	10	15	820	15	5	15	0	0
Sign Control		Free			Free		Stop		Stop	
Grade		0%			0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	582	11	16	891	16	5	16	0	0
Pedestrians							90		150	
Lane Width (m)							3.6		3.6	
Walking Speed (m/s)							1.2		1.2	
Percent Blockage							8		12	
Right turn flare (veh)										
Median type		None			None					
Median storage (veh)										
Upstream signal (m)		143			163					
pX, platoon unblocked	0.67			0.83			0.75	0.83	0.75	0.75
vC, conflicting volume	1057			683			1766	678	1687	1764
vC1, stage 1 conf vol										
vC2, stage 2 conf vol										
vCu, unblocked vol	837			516			1347	509	1242	1344
tC, single (s)	4.1			4.2			6.5	6.3	7.1	6.5
tC, 2 stage (s)										
tF (s)	2.2			2.3			4.0	3.4	3.5	4.0
p0 queue free %	100			98			95	96	100	100
cM capacity (veh/h)	471			787			91	425	78	91
Direction, Lane #	EB 1	WB 1	NB 1	SE 1						
Volume Total	593	923	21	0						
Volume Left	0	16	0	0						
Volume Right	11	16	16	0						
cSH	471	787	227	1700						
Volume to Capacity	0.00	0.02	0.09	0.00						
Queue Length 95th (m)	0.0	0.5	2.4	0.0						
Control Delay (s)	0.0	0.6	22.5	0.0						
Lane LOS		A	C	A						
Approach Delay (s)	0.0	0.6	22.5	0.0						
Approach LOS			C	A						
Intersection Summary										
Average Delay			0.7							
Intersection Capacity Utilization			66.2%		ICU Level of Service				C	
Analysis Period (min)			15							

Timings 9: Patricia Drive/Gledhill Avenue & Danforth Avenue

FT AM
Future Total AM

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	30	500	20	10	835	30	60	20	15	30	10	35
Future Volume (vph)	30	500	20	10	835	30	60	20	15	30	10	35
Satd. Flow (prot)	1668	1749	0	1623	3437	0	0	1656	0	0	1443	0
Flt Permitted	0.266			0.379				0.764			0.840	
Satd. Flow (perm)	429	1749	0	616	3437	0	0	1172	0	0	1197	0
Satd. Flow (RTOR)		3			6			12			36	
Lane Group Flow (vph)	33	578	0	11	961	0	0	101	0	0	83	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Total Split (s)	54.0	54.0		54.0	54.0		36.0	36.0		36.0	36.0	
Total Lost Time (s)	5.9	5.9		5.9	5.9			7.1			7.1	
Act Effect Green (s)	61.2	61.2		61.2	61.2			23.0			23.0	
Actuated g/C Ratio	0.68	0.68		0.68	0.68			0.26			0.26	
v/c Ratio	0.11	0.49		0.03	0.41			0.33			0.25	
Control Delay	9.1	11.3		3.3	4.0			27.4			18.8	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	9.1	11.3		3.3	4.0			27.4			18.8	
LOS	A	B		A	A			C			B	
Approach Delay		11.2			3.9			27.4			18.8	
Approach LOS		B			A			C			B	

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 63 (70%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.49

Intersection Signal Delay: 8.5

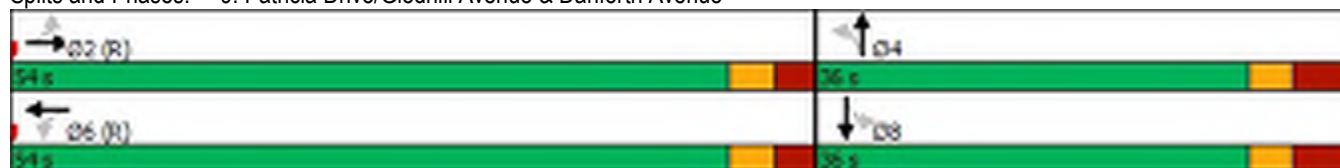
Intersection LOS: A

Intersection Capacity Utilization 61.9%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 9: Patricia Drive/Gledhill Avenue & Danforth Avenue



Queues

FT AM

9: Patricia Drive/Gledhill Avenue & Danforth Avenue

Future Total AM



Lane Group	EBL	EBT	WBL	WBT	NBT	SBT
Lane Group Flow (vph)	33	578	11	961	101	83
v/c Ratio	0.11	0.49	0.03	0.41	0.33	0.25
Control Delay	9.1	11.3	3.3	4.0	27.4	18.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	9.1	11.3	3.3	4.0	27.4	18.8
Queue Length 50th (m)	2.4	57.2	0.2	16.2	13.1	6.6
Queue Length 95th (m)	7.0	86.3	m0.6	17.3	27.7	18.9
Internal Link Dist (m)		219.0		119.1	113.2	112.4
Turn Bay Length (m)	30.0		20.0			
Base Capacity (vph)	291	1190	418	2339	384	408
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.49	0.03	0.41	0.26	0.20

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

9: Patricia Drive/Gledhill Avenue & Danforth Avenue

FT AM
Future Total AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	30	500	20	10	835	30	60	20	15	30	10	35
Future Volume (vph)	30	500	20	10	835	30	60	20	15	30	10	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	5.9	5.9		5.9	5.9			7.1			7.1	
Lane Util. Factor	1.00	1.00		1.00	0.95			1.00			1.00	
Frpb, ped/bikes	1.00	0.99		1.00	0.99			0.98			0.91	
Flpb, ped/bikes	0.91	1.00		0.94	1.00			0.90			0.97	
Frt	1.00	0.99		1.00	0.99			0.98			0.94	
Flt Protected	0.95	1.00		0.95	1.00			0.97			0.98	
Satd. Flow (prot)	1515	1750		1533	3436			1489			1398	
Flt Permitted	0.27	1.00		0.38	1.00			0.76			0.84	
Satd. Flow (perm)	424	1750		611	3436			1173			1197	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.97	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	33	556	22	11	928	33	62	22	17	33	11	39
RTOR Reduction (vph)	0	1	0	0	2	0	0	10	0	0	29	0
Lane Group Flow (vph)	33	577	0	11	959	0	0	91	0	0	54	0
Confl. Peds. (#/hr)	214		81	81		214	192		56	56		192
Confl. Bikes (#/hr)			2			3						
Heavy Vehicles (%)	7%	7%	14%	10%	3%	4%	3%	11%	15%	6%	22%	10%
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	58.6	58.6		58.6	58.6			18.4			18.4	
Effective Green, g (s)	58.6	58.6		58.6	58.6			18.4			18.4	
Actuated g/C Ratio	0.65	0.65		0.65	0.65			0.20			0.20	
Clearance Time (s)	5.9	5.9		5.9	5.9			7.1			7.1	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0			3.0	
Lane Grp Cap (vph)	276	1139		397	2237			239			244	
v/s Ratio Prot		c0.33			0.28							
v/s Ratio Perm	0.08			0.02				c0.08			0.05	
v/c Ratio	0.12	0.51		0.03	0.43			0.38			0.22	
Uniform Delay, d1	5.9	8.2		5.6	7.6			30.9			29.8	
Progression Factor	1.00	1.00		0.42	0.40			1.00			1.00	
Incremental Delay, d2	0.9	1.6		0.1	0.5			1.0			0.5	
Delay (s)	6.8	9.8		2.5	3.5			31.9			30.3	
Level of Service	A	A		A	A			C			C	
Approach Delay (s)		9.6			3.5			31.9			30.3	
Approach LOS		A			A			C			C	
Intersection Summary												
HCM 2000 Control Delay			8.5			HCM 2000 Level of Service				A		
HCM 2000 Volume to Capacity ratio			0.48									
Actuated Cycle Length (s)			90.0			Sum of lost time (s)			13.0			
Intersection Capacity Utilization			61.9%			ICU Level of Service			B			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis

10:

FT AM

Future Total AM

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	5	0	0	130	90	5
Future Volume (Veh/h)	5	0	0	130	90	5
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	0	0	141	98	5
Pedestrians	1			1	17	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	0			0	1	
Right turn flare (veh)						
Median type				None	None	
Median storage veh						
Upstream signal (m)					25	
pX, platoon unblocked						
vC, conflicting volume	260	102	104			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	260	102	104			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	100	100			
cM capacity (veh/h)	723	956	1499			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	5	141	103			
Volume Left	5	0	0			
Volume Right	0	0	5			
cSH	723	1499	1700			
Volume to Capacity	0.01	0.00	0.06			
Queue Length 95th (m)	0.2	0.0	0.0			
Control Delay (s)	10.0	0.0	0.0			
Lane LOS	B					
Approach Delay (s)	10.0	0.0	0.0			
Approach LOS	B					
Intersection Summary						
Average Delay		0.2				
Intersection Capacity Utilization		17.2%		ICU Level of Service		A
Analysis Period (min)		15				

Timings

1: Main Street & Danforth Avenue

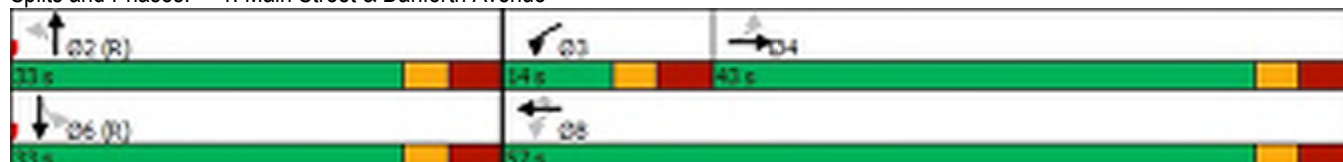
FT PM
Future Total PM

									
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations									
Traffic Volume (vph)	65	460	190	515	80	95	245	50	270
Future Volume (vph)	65	460	190	515	80	95	245	50	270
Turn Type	Perm	NA	pm+pt	NA	Perm	Perm	NA	Perm	NA
Protected Phases		4	3	8			2		6
Permitted Phases	4		8		8	2		6	
Detector Phase	4	4	3	8	8	2	2	6	6
Switch Phase									
Minimum Initial (s)	30.0	30.0	6.0	30.0	30.0	20.0	20.0	20.0	20.0
Minimum Split (s)	36.7	36.7	12.7	36.7	36.7	28.7	28.7	28.7	28.7
Total Split (s)	43.0	43.0	14.0	57.0	57.0	33.0	33.0	33.0	33.0
Total Split (%)	47.8%	47.8%	15.6%	63.3%	63.3%	36.7%	36.7%	36.7%	36.7%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	6.7	6.7	6.7	6.7	6.7		6.7		6.7
Lead/Lag	Lag	Lag	Lead						
Lead-Lag Optimize?	Yes	Yes	Yes						
Recall Mode	None	None	Min	None	None	C-Min	C-Min	C-Min	C-Min
Act Effect Green (s)	33.4	33.4	48.1	48.1	48.1		28.5		28.5
Actuated g/C Ratio	0.37	0.37	0.53	0.53	0.53		0.32		0.32
v/c Ratio	0.24	0.86	0.73	0.54	0.12		0.81		0.50
Control Delay	23.1	44.0	29.3	16.0	5.3		28.7		28.1
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	23.1	44.0	29.3	16.0	5.3		28.7		28.1
LOS	C	D	C	B	A		C		C
Approach Delay		41.7		18.1			28.7		28.1
Approach LOS		D		B			C		C

Intersection Summary

Cycle Length: 90	
Actuated Cycle Length: 90	
Offset: 70 (78%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green	
Natural Cycle: 80	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.86	
Intersection Signal Delay: 28.4	Intersection LOS: C
Intersection Capacity Utilization 113.9%	ICU Level of Service H
Analysis Period (min) 15	

Splits and Phases: 1: Main Street & Danforth Avenue

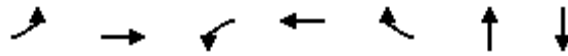


Queues

FT PM

1: Main Street & Danforth Avenue

Future Total PM



Lane Group	EBL	EBT	WBL	WBT	WBR	NBT	SBT
Lane Group Flow (vph)	68	557	198	536	83	620	354
v/c Ratio	0.24	0.86	0.73	0.54	0.12	0.81	0.50
Control Delay	23.1	44.0	29.3	16.0	5.3	28.7	28.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	23.1	44.0	29.3	16.0	5.3	28.7	28.1
Queue Length 50th (m)	11.5	98.9	17.9	60.2	2.9	36.4	26.8
Queue Length 95th (m)	m18.2	#144.2	#37.7	84.4	9.3	#70.8	42.4
Internal Link Dist (m)		68.6		134.2		339.1	295.4
Turn Bay Length (m)	30.0						
Base Capacity (vph)	310	705	270	1031	697	765	702
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.22	0.79	0.73	0.52	0.12	0.81	0.50

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

1: Main Street & Danforth Avenue

FT PM

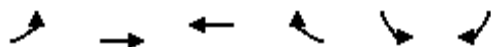
Future Total PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	65	460	75	190	515	80	95	245	255	50	270	20
Future Volume (vph)	65	460	75	190	515	80	95	245	255	50	270	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	6.7	6.7		6.7	6.7	6.7		6.7			6.7	
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00		0.95			0.95	
Frpb, ped/bikes	1.00	0.97		1.00	1.00	0.77		0.81			0.96	
Flpb, ped/bikes	0.89	1.00		0.99	1.00	1.00		0.93			0.98	
Frt	1.00	0.98		1.00	1.00	0.85		0.94			0.99	
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.99			0.99	
Satd. Flow (prot)	1578	1734		1655	1845	1211		2412			2981	
Flt Permitted	0.46	1.00		0.17	1.00	1.00		0.80			0.73	
Satd. Flow (perm)	771	1734		292	1845	1211		1950			2201	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	68	479	78	198	536	83	99	255	266	52	281	21
RTOR Reduction (vph)	0	7	0	0	0	22	0	147	0	0	5	0
Lane Group Flow (vph)	68	550	0	198	536	61	0	473	0	0	349	0
Confl. Peds. (#/hr)	251		260	260		251	689		212	212		689
Confl. Bikes (#/hr)			1			2			3			2
Heavy Vehicles (%)	1%	1%	20%	7%	3%	1%	2%	8%	0%	31%	7%	4%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	6	6	0	6	6
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4			8		8	2			6		
Actuated Green, G (s)	33.4	33.4		48.1	48.1	48.1		28.5			28.5	
Effective Green, g (s)	33.4	33.4		48.1	48.1	48.1		28.5			28.5	
Actuated g/C Ratio	0.37	0.37		0.53	0.53	0.53		0.32			0.32	
Clearance Time (s)	6.7	6.7		6.7	6.7	6.7		6.7			6.7	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	286	643		277	986	647		617			696	
v/s Ratio Prot		c0.32		0.06	c0.29							
v/s Ratio Perm	0.09			0.32		0.05		c0.24			0.16	
v/c Ratio	0.24	0.86		0.71	0.54	0.09		0.77			0.50	
Uniform Delay, d1	19.5	26.1		15.8	13.7	10.3		27.8			25.0	
Progression Factor	1.11	1.22		1.00	1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.4	9.6		8.5	0.6	0.1		8.8			2.6	
Delay (s)	22.1	41.5		24.3	14.4	10.3		36.6			27.6	
Level of Service	C	D		C	B	B		D			C	
Approach Delay (s)		39.4			16.4			36.6			27.6	
Approach LOS		D			B			D			C	
Intersection Summary												
HCM 2000 Control Delay			29.2									
HCM 2000 Volume to Capacity ratio			0.82									
Actuated Cycle Length (s)			90.0									
Intersection Capacity Utilization			113.9%									
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis

2: Danforth Avenue & Chisholm Avenue

FT PM
Future Total PM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	15	580	600	30	20	25
Future Volume (Veh/h)	15	580	600	30	20	25
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	630	652	33	22	27
Pedestrians		4	6		192	
Lane Width (m)		3.6	3.5		3.6	
Walking Speed (m/s)		1.2	1.2		1.2	
Percent Blockage		0	0		16	
Right turn flare (veh)						
Median type		None	None			
Median storage veh						
Upstream signal (m)		172	93			
pX, platoon unblocked	0.81				0.90	0.81
vC, conflicting volume	877				1512	848
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	728				1029	692
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	97				88	91
cM capacity (veh/h)	600				191	298
Direction, Lane #	EB 1	WB 1	WB 2	SB 1		
Volume Total	646	652	33	49		
Volume Left	16	0	0	22		
Volume Right	0	0	33	27		
cSH	600	1700	1700	238		
Volume to Capacity	0.03	0.38	0.02	0.21		
Queue Length 95th (m)	0.7	0.0	0.0	6.0		
Control Delay (s)	0.7	0.0	0.0	24.0		
Lane LOS	A			C		
Approach Delay (s)	0.7	0.0		24.0		
Approach LOS				C		
Intersection Summary						
Average Delay		1.2				
Intersection Capacity Utilization		53.8%		ICU Level of Service	A	
Analysis Period (min)		15				

HCM Unsignalized Intersection Capacity Analysis 5: Westlake Avenue & Westlake Site Access

FT PM
Future Total PM

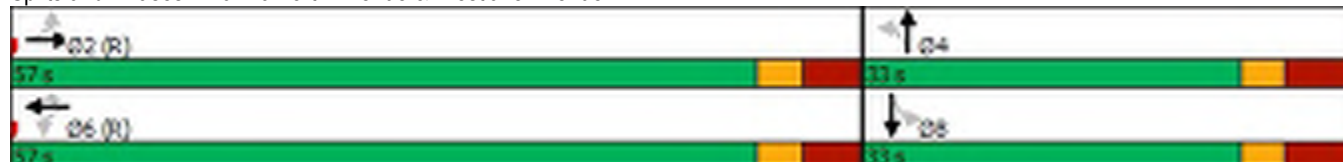
						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	5	85	40	5	85	30
Future Volume (Veh/h)	5	85	40	5	85	30
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	92	43	5	92	33
Pedestrians	34		6			6
Lane Width (m)	3.6		3.6			3.6
Walking Speed (m/s)	1.2		1.2			1.2
Percent Blockage	3		1			1
Right turn flare (veh)						
Median type			None			None
Median storage veh						
Upstream signal (m)						43
pX, platoon unblocked						
vC, conflicting volume	302	86			82	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	302	86			82	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	99	90			94	
cM capacity (veh/h)	629	946			1485	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	97	48	125			
Volume Left	5	0	92			
Volume Right	92	5	0			
cSH	922	1700	1485			
Volume to Capacity	0.11	0.03	0.06			
Queue Length 95th (m)	2.8	0.0	1.6			
Control Delay (s)	9.4	0.0	5.7			
Lane LOS	A		A			
Approach Delay (s)	9.4	0.0	5.7			
Approach LOS	A					
Intersection Summary						
Average Delay		6.0				
Intersection Capacity Utilization		27.1%		ICU Level of Service		A
Analysis Period (min)		15				

Timings 6: Danforth Avenue & Westlake Avenue

FT PM
Future Total PM

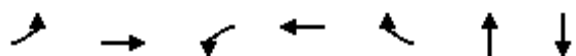
									
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations									
Traffic Volume (vph)	40	520	25	580	20	70	10	30	5
Future Volume (vph)	40	520	25	580	20	70	10	30	5
Turn Type	Perm	NA	Perm	NA	Perm	Perm	NA	Perm	NA
Protected Phases		2		6			4		8
Permitted Phases	2		6		6	4		8	
Detector Phase	2	2	6	6	6	4	4	8	8
Switch Phase									
Minimum Initial (s)	14.0	14.0	14.0	14.0	14.0	20.0	20.0	20.0	20.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	32.7	32.7	32.7	32.7
Total Split (s)	57.0	57.0	57.0	57.0	57.0	33.0	33.0	33.0	33.0
Total Split (%)	63.3%	63.3%	63.3%	63.3%	63.3%	36.7%	36.7%	36.7%	36.7%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	4.0	4.0	4.0	4.0	4.0	4.7	4.7	4.7	4.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	7.0	7.0	7.0	7.0	7.0		7.7		7.7
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	C-Min	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	55.3	55.3	55.3	55.3	55.3		20.0		20.0
Actuated g/C Ratio	0.61	0.61	0.61	0.61	0.61		0.22		0.22
v/c Ratio	0.11	0.59	0.07	0.52	0.03		0.41		0.19
Control Delay	5.2	13.6	13.4	19.8	12.2		27.1		23.9
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	5.2	13.6	13.4	19.8	12.2		27.1		23.9
LOS	A	B	B	B	B		C		C
Approach Delay		13.1		19.3			27.1		23.9
Approach LOS		B		B			C		C
Intersection Summary									
Cycle Length: 90									
Actuated Cycle Length: 90									
Offset: 85 (94%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green									
Natural Cycle: 65									
Control Type: Actuated-Coordinated									
Maximum v/c Ratio: 0.59									
Intersection Signal Delay: 17.4					Intersection LOS: B				
Intersection Capacity Utilization 66.2%					ICU Level of Service C				
Analysis Period (min) 15									

Splits and Phases: 6: Danforth Avenue & Westlake Avenue



Queues
6: Danforth Avenue & Westlake Avenue

FT PM
Future Total PM



Lane Group	EBL	EBT	WBL	WBT	WBR	NBT	SBT
Lane Group Flow (vph)	41	624	26	598	21	128	51
v/c Ratio	0.11	0.59	0.07	0.52	0.03	0.41	0.19
Control Delay	5.2	13.6	13.4	19.8	12.2	27.1	23.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	5.2	13.6	13.4	19.8	12.2	27.1	23.9
Queue Length 50th (m)	2.8	89.8	2.9	90.8	2.3	14.9	5.3
Queue Length 95th (m)	m4.0	132.2	m6.1	m118.7	m4.7	32.0	15.3
Internal Link Dist (m)		139.2		25.3		0.7	152.8
Turn Bay Length (m)	35.0		20.0		20.0		
Base Capacity (vph)	369	1060	360	1144	703	388	335
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.59	0.07	0.52	0.03	0.33	0.15

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

6: Danforth Avenue & Westlake Avenue

FT PM
Future Total PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	40	520	85	25	580	20	70	10	45	30	5	15
Future Volume (vph)	40	520	85	25	580	20	70	10	45	30	5	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	7.0	7.0		7.0	7.0	7.0		7.7			7.7	
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	0.95		1.00	1.00	0.72		0.97			0.96	
Flpb, ped/bikes	0.91	1.00		0.91	1.00	1.00		0.93			0.97	
Frt	1.00	0.98		1.00	1.00	0.85		0.95			0.96	
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.97			0.97	
Satd. Flow (prot)	1585	1716		1622	1863	1145		1590			1462	
Flt Permitted	0.36	1.00		0.34	1.00	1.00		0.80			0.77	
Satd. Flow (perm)	596	1716		580	1863	1145		1303			1154	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	41	536	88	26	598	21	72	10	46	31	5	15
RTOR Reduction (vph)	0	6	0	0	0	0	0	24	0	0	12	0
Lane Group Flow (vph)	41	618	0	26	598	21	0	104	0	0	39	0
Confl. Peds. (#/hr)	113		131	131		113	70		36	36		70
Confl. Bikes (#/hr)						2			3			2
Heavy Vehicles (%)	3%	3%	6%	0%	2%	0%	0%	0%	0%	20%	0%	0%
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6		6	4			8		
Actuated Green, G (s)	55.3	55.3		55.3	55.3	55.3		20.0			20.0	
Effective Green, g (s)	55.3	55.3		55.3	55.3	55.3		20.0			20.0	
Actuated g/C Ratio	0.61	0.61		0.61	0.61	0.61		0.22			0.22	
Clearance Time (s)	7.0	7.0		7.0	7.0	7.0		7.7			7.7	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	366	1054		356	1144	703		289			256	
v/s Ratio Prot		c0.36			0.32							
v/s Ratio Perm	0.07			0.04		0.02		c0.08			0.03	
v/c Ratio	0.11	0.59		0.07	0.52	0.03		0.36			0.15	
Uniform Delay, d1	7.2	10.5		7.0	9.9	6.8		29.6			28.2	
Progression Factor	0.62	1.08		1.79	1.80	1.75		1.00			1.00	
Incremental Delay, d2	0.5	2.1		0.3	1.5	0.1		0.8			0.3	
Delay (s)	5.0	13.4		12.9	19.2	12.0		30.4			28.5	
Level of Service	A	B		B	B	B		C			C	
Approach Delay (s)		12.9			18.7			30.4			28.5	
Approach LOS		B			B			C			C	
Intersection Summary												
HCM 2000 Control Delay			17.5				HCM 2000 Level of Service			B		
HCM 2000 Volume to Capacity ratio			0.53									
Actuated Cycle Length (s)			90.0				Sum of lost time (s)		14.7			
Intersection Capacity Utilization			66.2%				ICU Level of Service		C			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis 7: Westlake Avenue & Stephenson Avenue

FT PM
Future Total PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	5	30	0	10	25	20	5	20	15	30	0	5
Future Volume (vph)	5	30	0	10	25	20	5	20	15	30	0	5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	33	0	11	27	22	5	22	16	33	0	5
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	38	60	43	38								
Volume Left (vph)	5	11	5	33								
Volume Right (vph)	0	22	16	5								
Hadj (s)	0.03	-0.18	-0.20	0.09								
Departure Headway (s)	4.2	3.9	3.9	4.2								
Degree Utilization, x	0.04	0.07	0.05	0.04								
Capacity (veh/h)	842	892	878	824								
Control Delay (s)	7.3	7.2	7.1	7.4								
Approach Delay (s)	7.3	7.2	7.1	7.4								
Approach LOS	A	A	A	A								
Intersection Summary												
Delay				7.3								
Level of Service				A								
Intersection Capacity Utilization				30.1%	ICU Level of Service		A					
Analysis Period (min)				15								

HCM Unsignalized Intersection Capacity Analysis

8: Morton Road & Danforth Avenue & Oak Park Avenue

FT PM
Future Total PM

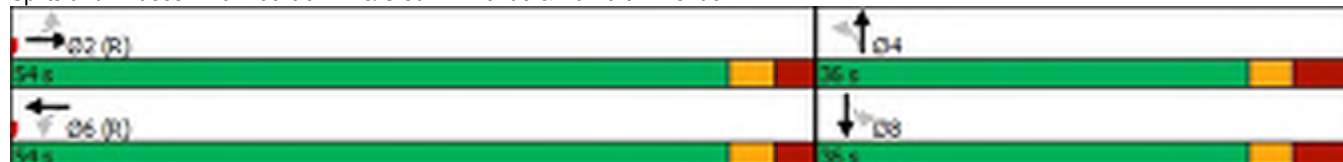
										
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBR	SEL	SER
Lane Configurations										
Traffic Volume (veh/h)	0	635	15	10	625	30	10	10	0	0
Future Volume (Veh/h)	0	635	15	10	625	30	10	10	0	0
Sign Control		Free			Free		Stop		Stop	
Grade		0%			0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	690	16	11	679	33	11	11	0	0
Pedestrians					9		122		107	
Lane Width (m)					3.6		3.6		3.6	
Walking Speed (m/s)					1.2		1.2		1.2	
Percent Blockage					1		10		9	
Right turn flare (veh)										
Median type		None			None					
Median storage (veh)										
Upstream signal (m)		143			163					
pX, platoon unblocked	0.80			0.78			0.88	0.78	0.88	0.88
vC, conflicting volume	819			828			1661	829	1548	1652
vC1, stage 1 conf vol										
vC2, stage 2 conf vol										
vCu, unblocked vol	651			641			1179	643	1050	1169
tC, single (s)	4.1			4.1			6.5	6.3	7.1	6.5
tC, 2 stage (s)										
tF (s)	2.2			2.2			4.0	3.4	3.5	4.0
p0 queue free %	100			98			92	97	100	100
cM capacity (veh/h)	690			663			136	320	126	137
Direction, Lane #	EB 1	WB 1	NB 1	SE 1						
Volume Total	706	723	22	0						
Volume Left	0	11	0	0						
Volume Right	16	33	11	0						
cSH	690	663	191	1700						
Volume to Capacity	0.00	0.02	0.12	0.00						
Queue Length 95th (m)	0.0	0.4	3.1	0.0						
Control Delay (s)	0.0	0.5	26.3	0.0						
Lane LOS		A	D	A						
Approach Delay (s)	0.0	0.5	26.3	0.0						
Approach LOS			D	A						
Intersection Summary										
Average Delay			0.6							
Intersection Capacity Utilization			55.6%		ICU Level of Service				B	
Analysis Period (min)			15							

Timings 9: Patricia Drive/Gledhill Avenue & Danforth Avenue

FT PM
Future Total PM

								
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations								
Traffic Volume (vph)	35	615	15	645	50	10	30	15
Future Volume (vph)	35	615	15	645	50	10	30	15
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases		2		6		4		8
Permitted Phases	2		6		4		8	
Detector Phase	2	2	6	6	4	4	8	8
Switch Phase								
Minimum Initial (s)	19.0	19.0	19.0	19.0	23.0	23.0	23.0	23.0
Minimum Split (s)	35.1	35.1	35.1	35.1	35.1	35.1	35.1	35.1
Total Split (s)	54.0	54.0	54.0	54.0	36.0	36.0	36.0	36.0
Total Split (%)	60.0%	60.0%	60.0%	60.0%	40.0%	40.0%	40.0%	40.0%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.9	2.9	2.9	2.9	4.1	4.1	4.1	4.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	5.9	5.9	5.9	5.9		7.1		7.1
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	61.2	61.2	61.2	61.2		23.0		23.0
Actuated g/C Ratio	0.68	0.68	0.68	0.68		0.26		0.26
v/c Ratio	0.09	0.55	0.04	0.29		0.21		0.16
Control Delay	8.4	12.3	13.5	15.0		26.8		23.8
Queue Delay	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	8.4	12.3	13.5	15.0		26.8		23.8
LOS	A	B	B	B		C		C
Approach Delay		12.2		14.9		26.8		23.8
Approach LOS		B		B		C		C
Intersection Summary								
Cycle Length: 90								
Actuated Cycle Length: 90								
Offset: 43 (48%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green								
Natural Cycle: 75								
Control Type: Actuated-Coordinated								
Maximum v/c Ratio: 0.55								
Intersection Signal Delay: 14.5					Intersection LOS: B			
Intersection Capacity Utilization 68.8%					ICU Level of Service C			
Analysis Period (min) 15								

Splits and Phases: 9: Patricia Drive/Gledhill Avenue & Danforth Avenue



Queues

FT PM

9: Patricia Drive/Gledhill Avenue & Danforth Avenue

Future Total PM



Lane Group	EBL	EBT	WBL	WBT	NBT	SBT
Lane Group Flow (vph)	36	683	16	688	67	57
v/c Ratio	0.09	0.55	0.04	0.29	0.21	0.16
Control Delay	8.4	12.3	13.5	15.0	26.8	23.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	8.4	12.3	13.5	15.0	26.8	23.8
Queue Length 50th (m)	2.6	72.2	2.3	56.2	8.9	6.6
Queue Length 95th (m)	6.8	108.0	m4.3	70.2	20.0	16.6
Internal Link Dist (m)		219.0		119.1	113.2	112.4
Turn Bay Length (m)	30.0		20.0			
Base Capacity (vph)	421	1238	381	2384	403	440
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.55	0.04	0.29	0.17	0.13

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

9: Patricia Drive/Gledhill Avenue & Danforth Avenue

FT PM
Future Total PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	35	615	40	15	645	15	50	10	5	30	15	10
Future Volume (vph)	35	615	40	15	645	15	50	10	5	30	15	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.6	3.6	3.5	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6
Total Lost time (s)	5.9	5.9		5.9	5.9			7.1			7.1	
Lane Util. Factor	1.00	1.00		1.00	0.95			1.00			1.00	
Frpb, ped/bikes	1.00	0.99		1.00	0.99			0.99			0.99	
Flpb, ped/bikes	0.89	1.00		0.94	1.00			0.96			0.98	
Frt	1.00	0.99		1.00	1.00			0.99			0.98	
Flt Protected	0.95	1.00		0.95	1.00			0.96			0.97	
Satd. Flow (prot)	1549	1819		1681	3504			1629			1618	
Flt Permitted	0.38	1.00		0.31	1.00			0.74			0.81	
Satd. Flow (perm)	614	1819		554	3504			1246			1352	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	36	641	42	16	672	16	52	10	5	31	16	10
RTOR Reduction (vph)	0	2	0	0	1	0	0	4	0	0	8	0
Lane Group Flow (vph)	36	681	0	16	687	0	0	63	0	0	49	0
Confl. Peds. (#/hr)	154		111	111		154	63		30	30		63
Confl. Bikes (#/hr)						3			1			
Heavy Vehicles (%)	3%	2%	3%	0%	2%	0%	2%	18%	20%	13%	0%	3%
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	58.6	58.6		58.6	58.6			18.4			18.4	
Effective Green, g (s)	58.6	58.6		58.6	58.6			18.4			18.4	
Actuated g/C Ratio	0.65	0.65		0.65	0.65			0.20			0.20	
Clearance Time (s)	5.9	5.9		5.9	5.9			7.1			7.1	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0			3.0	
Lane Grp Cap (vph)	399	1184		360	2281			254			276	
v/s Ratio Prot		c0.37			0.20							
v/s Ratio Perm	0.06			0.03				c0.05			0.04	
v/c Ratio	0.09	0.58		0.04	0.30			0.25			0.18	
Uniform Delay, d1	5.8	8.8		5.6	6.8			30.0			29.6	
Progression Factor	1.00	1.00		1.73	1.87			1.00			1.00	
Incremental Delay, d2	0.4	2.0		0.2	0.3			0.5			0.3	
Delay (s)	6.3	10.8		10.0	13.0			30.5			29.9	
Level of Service	A	B		A	B			C			C	
Approach Delay (s)		10.6			12.9			30.5			29.9	
Approach LOS		B			B			C			C	
Intersection Summary												
HCM 2000 Control Delay			13.2			HCM 2000 Level of Service				B		
HCM 2000 Volume to Capacity ratio			0.50									
Actuated Cycle Length (s)			90.0			Sum of lost time (s)			13.0			
Intersection Capacity Utilization			68.8%			ICU Level of Service			C			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis

10:

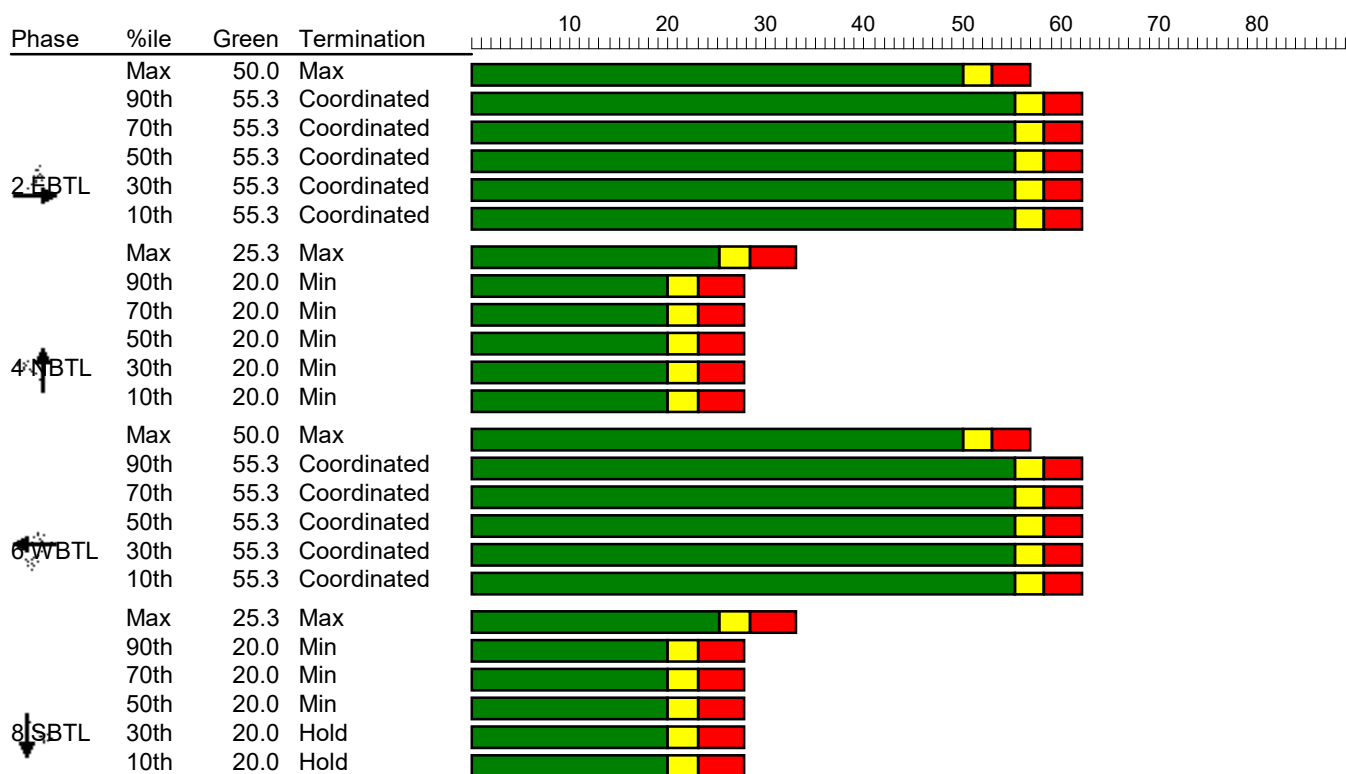
FT PM

Future Total PM

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	0	0	0	125	115	0
Future Volume (Veh/h)	0	0	0	125	115	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	136	125	0
Pedestrians	6			3	34	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	1			0	3	
Right turn flare (veh)						
Median type				None	None	
Median storage veh						
Upstream signal (m)					25	
pX, platoon unblocked						
vC, conflicting volume	301	134	131			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	301	134	131			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	672	913	1459			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	0	136	125			
Volume Left	0	0	0			
Volume Right	0	0	0			
cSH	1700	1459	1700			
Volume to Capacity	0.00	0.00	0.07			
Queue Length 95th (m)	0.0	0.0	0.0			
Control Delay (s)	0.0	0.0	0.0			
Lane LOS	A					
Approach Delay (s)	0.0	0.0	0.0			
Approach LOS	A					
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization			18.3%	ICU Level of Service		A
Analysis Period (min)			15			

Actuated Signal, Actual Green Times
6: Danforth Avenue & Westlake Avenue

FT PM
Future Total PM



Cycle Length: 90

